10/26/2020 7**:**02:09 PM

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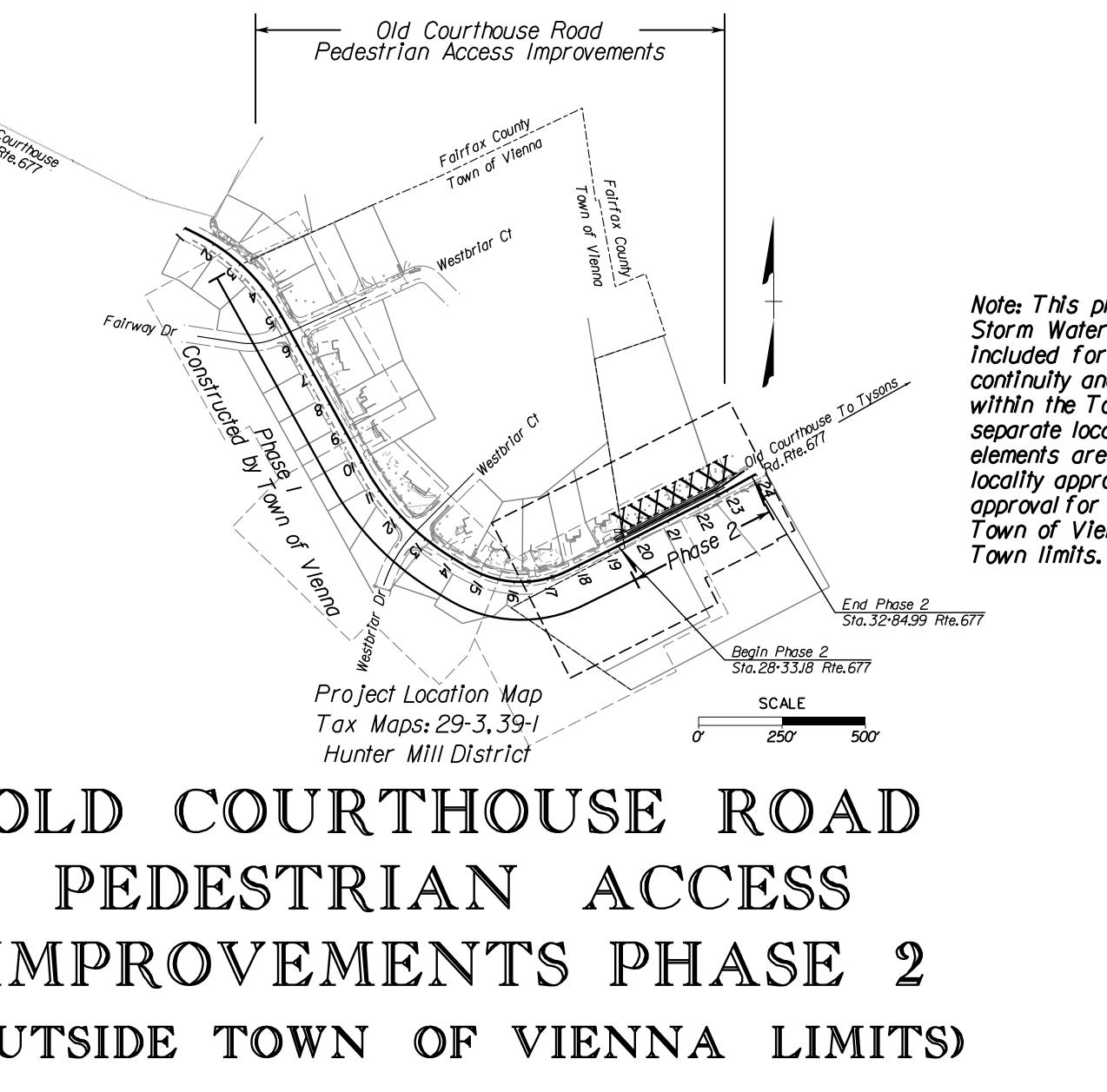
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	INDEX OF SHEETS
PROJECT MANAGER_ <i>Michael Gallagher_Town_of_Vienna_(703) 255-6389</i> SURVEYED BY <i>Rinker_Design_Assoc.P.C.(703) 368-7373_(2011)</i> DESIGN SUPERVISED BY _ <i>Adam_D.Welschenbach.P.E.,Rinker_Design_Assoc.,P.C.(703) 368-7373</i> . DESIGNED BY _ <i>Adam_D.Welschenbach.P.E.,Rinker_Design_Assoc.,P.C.(703) 368-7373_</i>	NDEX OF SHEETS Sheet Ma, I., S., Mithan S., Morri U.S.B. Sheet Ma, I.AIB, MORI U.S.B. Sheet Ma, I.AIB, MORI U.S.B. Sheet Ma, I.G., S., Standard Falfrax County Ultility General Notes. Sheet Ma, I.G., S., Standard Falfrax County Ultility General Notes. Sheet Ma, I.G., S., Standard Falfrax County Ultility General Notes. Sheet Ma, I.G., S., Standard Falfrax County Ultility General Notes. Sheet Ma, I.HII, M., WOT U.S.B. Sheet Ma, I.M., I.H., M. WOT U.S.B. Sheet Ma, I.M., I.L.B. Sheet Ma, I.M., T., WI-SSOC Proces J.Sta.25-25 to 33-00. Sheet Ma, I.M., T., WI-SSOC Proces J.Sta.25-25 to 33-00. Sheet Ma, I.M., T.L.B. Sheet Ma, Z.M.J. Sheet Ma, Z.M.J. Sheet Ma, Z.M.J. </td
	MAINTENANCE LEGEND: XXX-FAIRFAX COUNTY 348 LF-6' CONC. SIDEWALK 0 LF-TRAIL 0 EA-BMP FACILITIES DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION.
	THIS PROJECT IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE MOST RECENT REVISIONS OF THE DEPARTMENT'S: 2020 ROAD AND BRIDGE SPECIFICATIONS,
	2016 ROAD AND BRIDGE STANDARDS, 2009 MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD), 2011 VIRGINIA SUPPLEMENT TO THE MUTCD,
	2011 <u>VIRGINIA WORK AREA PROTECTION MANUAL</u> , AND AS AMENDED BY CONTRACT PROVISIONS AND THE COMPLETE ELECTRONIC .PDF VERSION OF THE PLAN ASSEMBLY.
	ALL CURVES ARE TO BE SUPERELEVATED, TRANSITIONED AND WIDENED IN ACCORDANCE WITH STANDARD <u>TC-5.11U</u> , EXCEPT WHERE OTHERWISE NOTED.

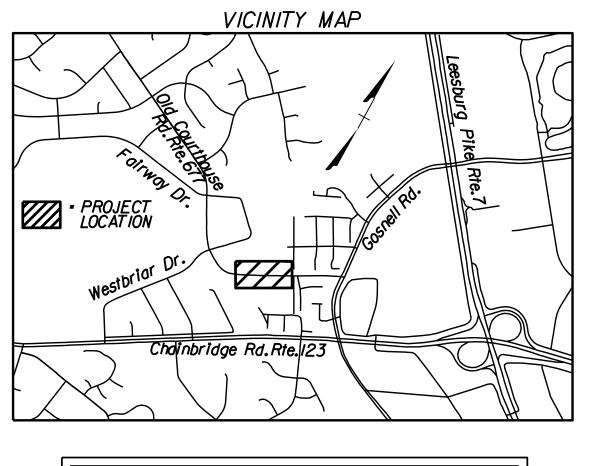
NOF VIENNA, VIR
5 12 1254 INCORPORATES

	OLD C
Fr:	Rte.724,Cr
То:	North City
AADT (2019)	7,200
DHV	1,095
D (%) (design hour)	83.5
T (%) (design hour)	0
DESIGN V (MPH)	25
POSTED V (MPH)	25

WN OF VIENNA, VIRGINIA RTMENT OF PUBLIC WORKS

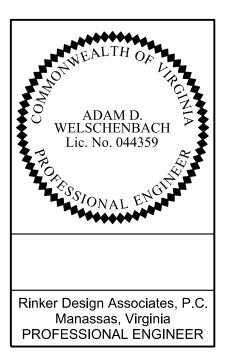


FUNCTIONAL CLASSIFICATION							
COURTHOUSE ROAD, ROUTE 677 (VIENNA ROUTE 6668)							
URBAN MINOR ARTERIAL (GS-6) - ROLLING							
Creek Crossing Rd ty Line for City of Vienna	East City Line for City of Vienna Rte.123,Chain Bridge Road	East City Line for City of Vienna West City Line for City of Vienna					
	10,000	8,300					
	1,230	1,129					
	84	82.6					
	0	0					
	25	25					
	25	25					





Note: This plan is for Phase 2 elements only. Storm Water Management/Hydraulics have been included for both Phase I and Phase 2 for continuity and clarity. ALL Phase lelements are within the Town of Vienna and subject to Town's separate locality approvals only. ALL Phase I elements are shown for information only and Town locality approval is provided seperately. Locality approval for Phase I is shown on Sheet 2K(9A). Town of Vienna maintains all roadways within



FINAL PLANS AUTHORIZED FOR CONSTRUC	CTION
DIRECTOR, TOWN OF VIENNA DEPARTMENT OF PUBLIC WORKS	DATE
	SHEET /

	GENERAL NOTES	
<u>EROSION AND SEDIMENT CONTROL</u> (THESE GENE	ERAL NOTES SHALL BE USED WHERE THEY ARE APPLICABLE TO THE PROJECT PLANS)	
I. EROSION CONTROL NARRATIVE	CONSTRUCTION NOTES (CONTINUED)	CONSTRUCTION NOTES (CONTINUED)
PRIOR TO ANY LAND DISTURBING OPERATIONS, THE EROSION CONTROLS, AS SPECIFIED BY THE ENGINEERING PLANS, SHALL BE INSTALLED. ALL MECHANICAL AND VEGETATIVE PRACTICES SHALL BE IN CONFORMANCE WITH THE REQUIREMENTS CONTAINED IN THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL AND THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK. SLOPE AREAS THAT CANNOT BE ADEQUATELY STABILIZED BY SEEDING, SHALL BE SODDED AND STAKED. AS DISTURBED AREAS, NOT TO BE CONSTRUCTED UPON, ARE FINAL GRADED, THEY SHALL BE PREPARED, LIME AND FERTILIZER APPLIED, SEEDED AND MULCHED. FOR THE AREAS OUTSIDE THE V.D.O.T. RIGHT-OF-WAY, THE SEED SHALL CONSIST OF A MIXTURE OF KENTUCKY 31 TALL FESCUE AND KENBLUE IN ACCORDANCE WITH DEPARTMENT OF	 13. THE CONTRACTOR SHALL RESTORE ALL DRIVEWAYS DISTURBED DURING CONSTRUCTION. RESTORATION SHALL CONSIST OF THE FOLLOWING: * GRAVEL DRIVEWAY - PROVIDE MINIMUM 6" COMPACTED 21A * ASPHALT DRIVEWAY - PROVIDE 6" COMPACTED 21A WITH MINIMUM 2" SM-9.5A OVERLAY 	 <u>STORM SEWER</u> A WATERTIGHT CONNECTION SHALL BE MADE AT ALL PIPES ENTERING DRAINAGE STRUCTURES. IN ADDITION, WATERTIGHT CONNECTIONS SHALL BE MADE BETWEEN SECTIONS OF PIPE. LENGTHS OF PIPE SHOWN ON THE DRAWINGS ARE MEASURED FROM CENTER OF STRUCTURE TO
PUBLIC WORKS AND ENVIRONMENTAL SERVICES SPECIFICATIONS OR COUNTY APPROVED ALTERNATE SEED FOR AREAS LOCATED WITHIN THE V.D.O.T. RIGHT-OF-WAY, THE SEED MIXTURE SHALL BE IN CONFORMANCE WITH CURRENT V.D.O.T.	* CONCRETE DRIVEWAY - PROVIDE A NEAT SAWCUT CONNECTION, MINIMUM 4" COMPACTED 21A AND 5" CLASS A3 CONCRETE WITH WWF 6X6 - W2.9 X W2.9	CENTER OF STRUCTURE.
REQUIREMENTS. SEDIMENT CONTROLS AND MECHANICAL DEVICES SHALL BE REMOVED FROM CONTRIBUTING AREAS AS THEY BECOME STABILIZED. THIS RESTORATION WORK WILL BE PERFORMED WITHIN 7 DAYS AFTER FINAL GRADING. ALL TEMPORARY SEDIMENT CONTROLS AND MECHANICAL DEVICES SHALL BE REMOVED FROM CONTRIBUTING AREAS AS THEY BECOME STABILIZED. FOR ADDITIONAL DETAILS, REFER TO THE CURRENT EDITION OF THE VIRGINIA FROSION	14. WHERE A PROPOSED PIPE CROSSES OR PARALLELS A STREET, THE ASPHALT SHALL BE NEATLY SAWCUT TO FULL DEPTH. AFTER INSTALLATION OF THE PIPE, THE ROADWAY SHALL BE PATCHED IN ACCORDANCE WITH THE V.D.O.T. LAND USE PERMIT ISSUED FOR THE PROJECT.	 INVERT ELEVATIONS SHOWN ON THE DRAWINGS ARE TAKEN AT THE FACE OF THE STRUCTURE. SHAPE THE INVERTS OF ALL STRUCTURES ACCORDING TO V.D.O.T. STANDARD IS-1. STORM SEWER AND CULVERT PIPE SHALL BE REINFORCED CONCRETE PIPE TO CONFORM TO THE
and sediment control handbook and the fairfax county public facilities manual. II. <u>EROSION, SEDIMENTATION, AND LAND CONSERVATION NOTES</u>	15. HORIZONTAL LOCATION OF THE SERVICE CONNECTIONS AT SANITARY SEWER MAINS, IF INDICATED ON THE CONSTRUCTION PLANS, WERE TAKEN FROM TELEVISION INSPECTION REPORTS. ALL SERVICE CONNECTIONS WERE SHOWN TO BE IN THE UPPER TWO QUADRANTS OF THE MAINS. EXCEPT FOR INTERPOLATION OF THE INVERT AT THE SANITARY MAINS, THE ELEVATIONS AND VERTICAL	CURRENT A.A.S.H.T.O. DESIGNATION M170, UNLESS OTHERWISE DESIGNATED ON THE PLANS. CLASS III PIPE WILL BE REQUIRED WITHIN THE LIMITS OF V.D.O.T. RIGHT-OF-WAY, UNLESS OTHERWISE NOTED.
 MEASURES TO CONTROL EROSION AND SILTATION SHALL BE PROVIDED PURSUANT TO AND IN COMPLIANCE WITH CURRENT FEDERAL, STATE AND LOCAL REGULATIONS. THE INFORMATION CONTAINED IN THE CONSTRUCTION PLANS AND/OR THE APPROVAL OF THE PLANS SHALL IN NO WAY RELIEVE THE CONTRACTOR OR HIS AGENT OF ANY LEGAL RESPONSIBILITY WHICH MAY BE REQUIRED BY THE CODE OF VIRGINIA OR ANY ORDINANCE ENACTED BY THE COUNTY OF FAIRFAX. 	LOCATIONS OF THE SERVICE CONNECTIONS ARE NOT KNOWN. IN SEWER MAINS AS NOTED, TELEVISION INSPECTIONS DID NOT DETECT ANY SERVICE CONNECTIONS AND THE LOCATIONS SHOWN ON THE DRAWING ARE THE MOST PROBABLE POSITIONS FOR SERVICE CONNECTIONS.	6. MINOR FIELD ADJUSTMENTS IN THE ELEVATION AND ALIGNMENT OF THE STORM SEWER AND STRUCTURE MIGHT BE NECESSARY TO MEET EXISTING CONDITIONS AND PROPOSED FINAL GRADING. THE CONTRACTOR SHALL NOTIFY FAIRFAX COUNTY DEPARTMENT OF PUBLIC WORKS AND ENVIRONMENTAL SERVICES PRIOR TO MAKING ANY NECESSARY ADJUSTMENTS.
2. ALL AREAS, ON OR OFF-SITE, WHICH ARE DISTURBED BY THIS CONSTRUCTION AND WHICH ARE NOT PAVED OR BUILT UPON SHALL BE ADEQUATELY STABILIZED TO CONTROL EROSION AND SEDIMENTATION. ACCEPTABLE STABILIZATION SHALL CONSIST OF PERMANENT GRASS SEED MIXTURE INSTALLED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. ALL SLOPES 2:1 AND	16. THE PAVEMENT DESIGN IN THE CONSTRUCTION PLANS WAS PREPARED BASED ON AVAILABLE SUBSURFACE INFORMATION INCLUDING LABORATORY CALIFORNIA BEARING RATIO (C.B.R.) TESTS THAT WERE PERFORMED DURING THE DESIGN SUBSURFACE INVESTIGATION. WHEN THE CONTRACTOR REACHES THE SUBGRADE ELEVATION, ADDITIONAL CBR TEST LOCATIONS MAY BE DIRECTED BY THE ENGINEER FOR SELECTED LOCATIONS	 TOP OF STRUCTURES SHALL BE SET TO MATCH CURB AND GUTTER, SIDEWALK AND/OR DITCH CONSTRUCTION. A VIDEO INSPECTION OF ALL NEW OR MODIFIED STORM SEWERS IN VDOT RIGHT OF WAY WILL
GREATER SHALL BE SODDED AND STAKED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE ENGINEER. 3. ANY DISTURBED AREA NOT PAVED, PERMANENTLY SEEDED, SODDED, OR BUILT UPON BY 1 NOVEMBER OR DISTURBED AFTER THAT	BASED ON ACTUAL FIELD CONDITIONS OBSERVED. THE CBR SAMPLES AND TESTS SHALL BE PERFORMED BY A GEOTECHNICAL ENGINEERING FIRM UNDER CONTRACT WITH FAIRFAX COUNTY. THE RESULTS OF THE ENGINEER'S EVALUATION OF THE CBR LABORATORY TESTS SHALL BE OBTAINED IN WRITING PRIOR TO THE PLACEMENT OF ANY SUBBASE OR BASE MATERIAL IN THE AREA(S) UNDER CONSIDERATION. THE PROPOSED	BE REQUIRED NO MORE THAN 60 DAYS IN ADVANCE OF PROJECT ACCEPTANCE BY V.D.O.T. 9. ALL STORMSEWER PIPES WITHIN THE PROJECT LIMITS WILL BE FLUSHED.
DATE, IS TO BE SEEDED WITHIN 14 DAYS WITH OATS, ABRUZZI RYE OR APPROVED EQUIVALENT, AND MULCHED WITH HAY OR STRAW MULCH AT THE RATE OF 2 TONS PER ACRE. FOR ADDITIONAL DETAILS, REFER TO THE CURRENT EDITION OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.	PAVEMENT DESIGN FOR THE AREA(S) UNDER CONSIDERATION WILL EITHER BE CONFIRMED OR ADJUSTED BY THE ENGINEER BASED ON THE RESULT OF THE C.B.R. TEST RESULTS. THE CONTRACTOR SHALL COOPERATE WITH ENGINEER BY MODIFYING CONSTRUCTION ACTIVITIES AND/OR SCHEDULING IN ORDER TO PERMIT THE ADDITIONAL CBR TESTING. THE CONTRACTOR SHALL NOT BE ENTITLED TO ANY MONETARY DAMAGES WHATSOEVER FOR ANY DELAYS RESULTING	SIDEWALKS AND TRAILS 1. A MINIMUM OF 2 FOOT CLEARANCE IS REQUIRED BETWEEN THE WALKWAY EDGE AND ANY VERTICAL
4. NO AREA SHALL BE DENUDED AND NOT BE DISTURBED FOR A PERIOD LONGER THAN 14 DAYS IN ACCORDANCE WITH THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.	FROM THIS TESTING. THE CONTRACTOR'S SOLE RELIEF IS A TIME EXTENSION GRANTED IN ACCORDANCE WITH ARTICLE 8.3.	OBSTRUCTIONS SUCH AS TREES, UTILITY POLES, SIGNS, ETC., UNLESS OTHERWISE SPECIFICALLY NOTED ON THE PLANS AT EACH LOCATION.
5. TEMPORARY DIVERSIONS, SEEDED AND MULCHED OR STAKED STRAW BALE DIVERSIONS AND OTHER CONTROL MEASURES NECESSARY ARE TO BE PLACED AS INDICATED ON THE DRAWINGS PRIOR TO OR AS THE FIRST STEP IN EXCAVATION.	TRANSPORTATION MANAGEMENT PLAN	2. A 4 FOOT UTILITY STRIP IS REQUIRED BETWEEN THE EDGE OF THE WALKWAY AND THE BACK OF THE CURB AND GUTTER, UNLESS OTHERWISE NOTED ON THE PLAN.
6. WHEN IN ACCORDANCE WITH STATE AND FEDERAL JOB SAFETY REQUIREMENTS, ALL EXCAVATED MATERIAL IS TO BE PLACED ON THE UPHILL SIDE OF TRENCHES. NO MATERIAL IS TO BE PLACED IN STREAMS. ANY STOCKPILED MATERIAL WHICH WILL REMAIN IN PLACE LONGER THAN 14 DAYS IS TO BE SEEDED FOR TEMPORARY VEGETATION AND MULCHED. WHERE SPOIL IS PLACED ON THE DOWNHILL SIDE OF TRENCH, IT IS TO BE BACK-SLOPED TO DRAIN TOWARD THE TRENCH. WHEN NECESSARY TO DEWATER THE TRENCH.	 THE CONTRACTOR SHALL BE REQUIRED TO SUBMIT CHANGES TO APPROVED TRANSPORTATION MANAGEMENT PLANS FOR APPROVAL BY V.D.O.T. A MINIMUM OF 30 WORKING DAYS PRIOR TO BEGINNING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DAILY TRAFFIC CONTROL SUCH AS LANE CLOSURES, FLAGGERS 	3. A MINIMUM 10 FOOT SEPARATION IS REQUIRED BETWEEN THE EDGE OF THE SIDEWALK AND THE EDGE OF THE PAVEMENT FOR NON CURB AND GUTTER ROADS, UNLESS OTHERWISE NOTED ON THE PLANS.
THE PUMP DISCHARGE HOSE MUST OUTLET IN A STABILIZED AREA OR A SEDIMENT BASIN. 7. WHERE STREAM CROSSINGS ARE REQUIRED FOR EQUIPMENT, TEMPORARY CULVERTS SHALL BE PROVIDED.	DRIVEWAY ENTRANCES, ETC., TO PROPERLY MAINTAIN TRAFFIC THROUGHOUT THE PROJECT. 3. EXISTING SURFACE, BASE, AND SUBBASE MATERIAL WHICH WILL BE DEMOLISHED OR OBLITERATED DURING CONSTRUCTION AND WHICH IS DETERMINED SUITABLE BY THE ENGINEER, SHALL BE SALVAGED AND UTILIZED	4. ALL HANDRAIL MATERIAL AND INSTALLATION SHALL CONFORM TO THE CURRENT VDOT ROAD AND BRIDGE STANDARDS 601.05 <u>STANDARD HANDRAIL METHOD OF LOCATING AND ERECTING</u> . THE FOUNDATION DETAIL FOR HANDRAILS SHALL BE SPECIFIED IN THE CONSTRUCTION DRAWINGS.
8. DURING CONSTRUCTION, ALL STORM SEWER INLETS WILL BE PROTECTED BY SILT TRAPS, MAINTAINED AND MODIFIED AS REQUIRED BY CONSTRUCTION PROGRESS.	FOR TRAFFIC MAINTENANCE. 4. ALL REQUIRED CONSTRUCTION SIGNING, TEMPORARY PAVEMENT WIDENING, TEMPORARY LAND STABILIZATION,	5. ALL VEGETATIVE MATERIAL WITHIN 10 FEET OF VERTICAL CLEARANCE FROM THE PROPOSED TRAIL SHALL BE REMOVED PRIOR TO TRAIL CONSTRUCTION. THE HORIZONTAL CLEARING LIMITS SHALL BE IN ACCORDANCE WITH THE DETAIL ON THE DETAIL SHEET. AN EXCEPTION TO THIS
9. ALL DISTURBED AREAS ARE TO BE SEEDED AND MULCHED OR SODDED WITHIN 5 DAYS AFTER BACKFILL OF THE APPLICABLE TRENCH SECTION, IN ACCORDANCE WITH THE PROVISIONS CONTAINED IN THE PROJECT SPECIFICATIONS RELATING TO SEEDING AND SODDING. SPEED IS THE ESSENTIAL LAND CONSERVATION ELEMENT FOR A LINEAR PROJECT.	TRAFFIC BARRIERS, TEMPORARY PAVEMENT MARKINGS, ERADICATION, ETC., SHALL BE INCLUDED IN THE CONTRACTOR'S BID PRICE FOR TRAFFIC MAINTENANCE. 5. ALL TRAFFIC MAINTENANCE SHALL CONFORM WITH THE FOLLOWING AND THE LATEST REVISIONS THERETO: THE VIRGINIA	REQUIREMENT SHALL BE TO SAVE ANY TREES THAT ARE DESIGNATED ON THE CONSTRUCTION PLANS. 6. THE GRADES FOR PROPOSED SIDEWALK SHALL BE IN ACCORDANCE WITH CONSTRUCTION PLANS. ANY FIELD ADJUSTMENT OF THE PROPOSED SIDEWALK GRADES SHALL BE APPROVED BY THE
 FOR FURTHER REQUIREMENTS AND DETAILS OF TREE PRESERVATION, PLANTING, EROSION AND SEDIMENT CONTROL, SEE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL AND/OR THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK. ALL EROSION AND SEDIMENT CONTROLS SHALL BE REMOVED WITHIN 30 DAYS AFTER THE PROJECT IS STABILIZED. (MS-18) 	WORK AREA PROTECTION MANUAL, VA ROAD AND BRIDGE SPECIFICATIONS, VA ROAD AND BRIDGE STANDARDS AND THE FHWA MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES. 6. ALL EXISTING SIGNS SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION.	ENGINEER. THE FOLLOWING GRADE CRITERIA SHALL BE ADHERED TO: * MINIMUM: 1% EXCEPT IN SAGS WITH ADEQUATE DRAINAGE * MAXIMUM: ROADWAY GRADE OR 5%, WHICHEVER IS GREATER.
 AN EROSION AND SEDIMENT CONTROL CONTRACTOR CERTIFICATION (E.S.C.C.C.) IS REQUIRED FOR ALL LAND DISTURBING ACTIVITIES. THE CONTRACTOR SHALL PROPERLY INSTALL AND MAINTAIN EROSION AND SEDIMENT CONTROLS FOR THE LIFE OF THE PROJECT; AND ROUTINELY CHECK CONTROL DEVICES BEFORE, DURING AND AFTER STORM EVENTS. 	 CONTRACTOR SHALL MAINTAIN ACCESS TO ALL DRIVEWAY ENTRANCES DURING CONSTRUCTION. TEMPORARY CLOSURE AFFECTING EGRESS TO ADJACENT PROPERTIES SHALL BE COORDINATED WITH AFFECTED PARTIES. 	 ALL DRIVEWAYS SHALL BE GRADED AS INDICATED ON THE CONSTRUCTION PLANS TO CONFORM TO ALLOWABLE CROSS SLOPES FOR THE WALKWAY. THE CROSS SLOPES FOR WALKWAYS SHALL BE 1/4" PER FOOT OF WIDTH.
CONSTRUCTION NOTES	 PEDESTRIAN DETOUR SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION ACCORDING TO VIRGINIA WORK AREA PROTECTION AND MUTCD GUIDELINES. THE CONTRACTOR SHALL INFORM THE V.D.O.T. TRAFFIC OPERATIONS CENTER OF ANY LANE CLOSURES AND SUBSEQUENT OPENINGS 	8. THE MINIMUM ALLOWABLE TURNING RADIUS FOR WALKWAYS SHALL BE 20 FEET. ANY TURNING RADIUS LESS THAN 20 FEET, REQUIRED DUE TO FIELD ADJUSTMENTS, SHALL BE APPROVED BY THE ENGINEER.
 ALL CONSTRUCTION, INCLUDING ANY PROPOSED LANDSCAPING, SHALL CONFORM TO THE CURRENT EDITION OF THE FAIRFAX COUNTY PFM AND THE VIRGINIA DEPARTMENT OF TRANSPORTATION (V.D.O.T.) STANDARDS AND SPECIFICATIONS AND SHALL CONFORM TO THE CURRENT EDITION OF THE VIRGINIA UNIFORM STATEWIDE BUILDING CODE. 	ON A DAILY BASIS BY CALLING 703-877-3449. SANITARY SEWER	9. ALL MATERIALS AND CONSTRUCTION METHODS SHALL CONFORM TO THE REQUIREMENTS FOR WALKWAYS IN THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.
2. ALL CONSTRUCTION WITHIN THE V.D.O.T. RIGHT-OF-WAY SHALL CONFORM TO THE PROVISIONS CONTAINED IN THE V.D.O.T. LAND USE PERMIT ISSUED FOR THIS LOCATION. THE CONTRACTOR SHALL BE THOROUGHLY FAMILIARIZED WITH THE REQUIREMENTS OF THIS LAND USE PERMIT PRIOR TO THE START OF ANY CONSTRUCTION IN V.D.O.T. RIGHT-OF-WAY. THE SPECIAL PROVISIONS IN FORM MP-63 ARE A PART OF THE VDOT LAND USE PERMIT.	 ALL SANITARY SEWER IN THIS CONTRACT SHALL BE PVC SDR-35 UNLESS OTHERWISE NOTED. LENGTHS OF PIPE SHOWN ON THE DRAWINGS ARE MEASURED FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE. 	10. UNLESS MORE STRINGENT COMPACTION REQUIREMENTS ARE NOTED ON THE PLANS, THE SUBGRADE FOR ALL WALKWAYS SHALL BE COMPACTED TO A MINIMUM OF 95% OF THEORETICAL MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT WITHIN THE TOLERANCE SPECIFIED IN THE CURRENT
3. UNLESS MORE STRINGENT COMPACTION REQUIREMENTS ARE INDICATED ON THE PLANS OR IN THE SPECIFICATIONS, THE BACKFILL OF EMBANKMENT MATERIAL, THE INSTALLATION OF TRENCH BACKFILL AND THE RESTORATION OF DISTURBED AREAS SHALL BE COMPACTED IN ACCORDANCE WITH THE CURRENT EDITION OF THE V.D.O.T. ROAD AND BRIDGE SPECIFICATIONS. ALL COMPACTION SHALL BE AT ±2% OF THE OPTIMUM MOISTURE CONTENT.	3. IF PVC PIPE IS USED, ALL LATERAL SPUR CONNECTIONS SHALL BE MADE WITH MANUFACTURED PVC "TEE" OR "WYE" FITTINGS. NO SADDLES SHALL BE USED.	EDITION OF THE V.D.O.T. ROAD AND BRIDGE SPECIFICATIONS. 11. SHARED USE PATHS SHALL BE A MINIMUM OF 8.0 FEET FROM THE FACE OF CURB UNLESS OTHERWISE NOTED ON THE PLAN. TREE REMOVAL REPORTED TO THE PLAN.
4. ALL SUBGRADE, SUBBASE, BASE AND SHOULDER MATERIAL SHALL BE PLACED AND COMPACTED TO THE DENSITY SPECIFIED IN THE CURRENT EDITION OF THE V.D.O.T. ROAD AND BRIDGE SPECIFICATIONS. ALL COMPACTION SHALL BE AT ±2% OF THE OPTIMUM MOISTURE CONTENT.	4. LATERAL SPURS SHALL EXTEND 12" INSIDE PROPERTY LINES. IF THE UPPER ENDS OF THE LATERAL SPURS ARE GREATER THAN 6' BELOW THE GROUND SURFACE, THEN THE CONTRACTOR SHALL INSTALL TWO 45" BENDS AND A VERTICAL RISER (ALL 4" DIAMETER) TO WITHIN 4' OF THE GROUND SURFACE. THE INVERT SHOWN IN THE PROFILE IS AT THE BOTTOM OF THE LOWER 45"	TREE REMOVAL PROTECTION DURING CONSTRUCTION IN EASEMENTS DENOTES AN EXISTING TREE NOT TO BE REMOVED OR DAMAGED. ALL REMAINING TREES WITHIN THE EASEMENT LIMITS CAN BE REMOVED AS REQUIRED TO PERFORM THE CONSTRUCTION
5. THE CONTRACTOR SHALL PROVIDE ADEQUATE MEANS OF CLEANING TRUCKS AND/OR OTHER EQUIPMENT OF MUD PRIOR TO ENTERING THE V.D.O.T. RIGHT-OF-WAY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CLEAN ALL STREETS, ALLAY DUST AND TO TAKE WHATEVER MEASURES NECESSARY TO ENSURE THE ROAD(S) ARE MAINTAINED IN A CLEAN, MUD AND DUST FREE CONDITION AT ALL TIMES.	BEND. 5. AS-BUILT DRAWINGS FOR ALL NEW SANITARY SEWER INSTALLATIONS AND/OR ADJUSTMENTS TO EXISTING SANITARY SEWER FACILITIES SHALL BE PREPARED IN ACCORDANCE WITH SECTION 10-0104	THE CONSTRUCTION. IN ADDITION, THE CONTRACTOR MUST REMOVE ANY TREES ADVERSELY AFFECTED BY THE CONSTRUCTION TO THE EXTENT THAT IT IS LIKELY TO DIE IN THE OPINION OF THE COUNTY ARBORIST. HOWEVER, EVERY EFFORT WILL BE MADE TO AVOID THE REMOVAL OR DISTURBANCE OF REMAINING TREES.
 TEMPORARY OR PERMANENT PAVEMENT PATCHES ARE TO BE PLACED IN ALL ROADWAY CUTS WITH HOT MIX THE SAME DAY THE CUT IS MADE IN ACCORDANCE WITH V.D.O.T. REQUIREMENTS. SEE SPECIAL PROVISIONS FOR PAVEMENT OPEN CUTS, FORM LUP-OC NOVA FOR DETAILS. 	OF THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL. 6. WHEN AN EXISTING SANITARY SEWER LINE IS REPLACED, ALL EXISTING SANITARY SEWER LATERAL SPUR LINES SHALL BE RECONNECTED TO NEW SANITARY SEWER. THESE SANITARY SEWER LATERAL	DENOTES TREES TO HAVE TREE PROTECTION BARRICADE (SNOW FENCE) AT DRIP LINE AS PER ARTICLE 12 OF THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.
7. THE TOP ELEVATION OF EXISTING MANHOLES SHALL BE ADJUSTED TO MEET THE FINAL PAVEMENT ELEVATION AT THE TIME OF FINAL PAVING OPERATIONS. ALL MANHOLES ARE TO BE PROTECTED FROM THE TRAVELING PUBLIC. NO MANHOLE IS TO BE RAISED ABOVE THE TOP ELEVATION OF THE ROADWAY WITHOUT THE APPROPRIATE TRANSITION.	SPOR LINES SHALL BE RECOMMECTED TO NEW SAMITART SEWER. THESE SAMITART SEWER LATERAL SPUR LINE STREET TAPS SHALL BE PROVIDED STARTING AT A POINT 2-FEET TO 6-FEET FROM THE MAIN SEWER, OR IF NECESSARY, AT A LONGER DISTANCE TO ENSURE THAT THE EXISTING SAMITARY SEWER LATERAL PIPE IS IN GOOD CONDITION.	DENOTES AN EXISTING TREE TO BE REMOVED. THIS INCLUDES THE REMOVAL AND / OR GRINDING OF STUMPS TO A MINIMUM OF 12" BELOW FINISHED GRADE.
8. CONTRACTOR IS TO ADJUST MANHOLE, VALVE, AND METER BOX COVERS BEFORE PLACING FINAL SURFACE PAVEMENT.	7. THE CONTRACTOR SHALL OBTAIN A FAIRFAX COUNTY PLUMBING PERMIT PRIOR TO THE CONNECTION OF ANY SANITARY SEWER SPUR LINE TO THE MAIN SANITARY SEWER.	REVISED: APRIL. 2017 TAX MAP 29-3
9. THE CONTRACTOR SHALL VISIT THE SITE AND SHALL VERIFY EXISTING CONDITIONS PRIOR TO SUBMITTING A BID FOR THE CONSTRUCTION OF THE PROJECT.	OTWEALTH OF LIP	EMERGENCY POLICE - FIRE - RESCUE 911 TOWN OF VIENNA, VIRGINIA
10. WHERE EXISTING NATURAL DRAINAGE DITCHES OR STREAM BANKS ARE DISTURBED DURING CONSTRUCTION THE CONTRACTOR SHALL RESTORE THESE AREAS TO ORIGINAL ALIGNMENT, GRADE AND INVERT.	ADAM D.	DEPARTMENT OF PUBLIC WORKS 127 CENTER STREET S, VIENNA, VA. 22180
11. PROPOSED TOP OF CURB GRADES SHALL BE FIELD ADJUSTED AS REQUIRED TO CONFORM TO THE INTENT OF THE TYPICAL SECTION. A SMOOTH GRADE SHALL BE MAINTAINED FROM THE CENTERLINE TO THE PROPOSED EDGE OF PAVEMENT OR FACE OF CURB TO PRECLUDE THE FORMING OF FALSE GUTTERS AND/OR THE PONDING OF WATER ON THE ROADWAY. THE EXISTING PAVEMENT SHALL BE RECAPPED AND/OR REMOVED AND REPLACED AS REQUIRED TO ACCOMPLISH THIS REQUIREMENT. ALL CURB FORMS SHALL BE INSPECTED FOR HORIZONTAL AND VERTICAL ALIGNMENT BY FAIRFAX COUNTY OR THEIR AUTHORIZED REPRESENTATIVES PRIOR TO PLACING OF CONCRETE.	or All excavation work ANYWHERE in Virginia!	DEPARTMENT OF PUBLIC WORKS 703-255-6380 OLD COURTHOUSE ROAD DEPERDING
 12. THE FOLLOWING PROVISIONS SHALL APPLY TO THE USE OF SHEETING AND SHORING: (A) SHEETING AND SHORING OR OTHER APPROVED METHODS FOR TRENCH BRACING WILL BE REQUIRED ON THIS CONTRACT AS NEEDED TO MEET ALL SAFETY REQUIREMENTS. (B) UNLESS OTHERWISE DIRECTED BY THE ENGINEER, SHEETING AND SHORING WILL BE REMOVED FROM ALL TRENCHES PRIOR TO 	Always call 811 before you dig in Virginia!	PEDESTRIAN ACCESS IMPROVEMENTS STANDARD FAIRFAX COUNTY GENERAL NOTES HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA
(B) UNLESS OTHERWISE DIRECTED BY THE ENGINEER, SHEETING AND SHORING WILL BE REMOVED FROM ALL TRENCHES PRIOR TO BACKFILLING OPERATIONS. (C) UNLESS SPECIFICALLY IDENTIFIED IN THE CONTRACT DOCUMENTS, NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR SHEETING	Rinker Design Associates, P.C.	SCALE DESIGNED BY: Fairfax County SHEET DESCRIPTION BY ARREQUED DATE HORIZ- N/A DRAFTED BY: Fairfax County IC
AND SHORING.	PROFESSIONAL ENGINEER S	DESCRIPTION BY APPROVED DATE VERT N/A CHECKED BY: Fairfax County /C

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UTILITIES NOTES THE UTILITY INFORMATION SHOWN ON THESE PLANS IS TAKEN FROM INFORMATION PROVIDED BY AN UNDERGROUND UTILITY DESIGNATING AND LOCATING COMPANY AND IN SOME CASES, FROM INFORMATION RECEIVED FROM THE UTILITY COMPANIES. THE DEPARTMENT OF PUBLIC WORKS AND ENVIRONMENTAL SERVICES DOES NOT GUARANTEE THAT THE UTILITY INFORMATION SHOWN ON THE PLANS IS COMPLETE OR ACCURATE. THE CONTRACTOR MUST VERIFY THE UTILITY LOCATIONS PRIOR TO CONSTRUCTION. 2. ALL EXISTING UNDERGROUND UTILITIES SHALL BE MARKED IN THE FIELD BY MISS UTILITY PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SCHEDULING THE FIELD MARKING OF UTILITIES WITH MISS UTILITY. 3. ALL EXISTING UNDERGROUND UTILITIES SHALL BE PHYSICALLY LOCATED BY THE CONTRACTOR PRIOR TO THE BEGINNING OF ANY CONSTRUCTION IN THE VICINITY OF THESE UTILITIES. 4. THE CONTRACTOR SHALL CONFORM TO THE PROVISIONS AS SPECIFIED IN THE CURRENT VIRGINIA ADMINISTRATIVE CODE (VAC) SECTION <u>20 VAC 5 – 309–140, EXCAVATOR'S RESPONSIBILITIES TO</u> <u>AVOID DAMAGE, DISLOCATING OR DISTURBANCE OF UTILITY LINES</u>, AS FOLLOWS: "ANY PERSON EXCAVATING AROUND UNDERGROUND UTILITY LINES SHALL TAKE ALL REASONABLE STEPS TO PROTECT SUCH UTILITY LINES. THESE STEPS SHALL INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING: 1. THE EXCAVATOR SHALL PLAN THE EXCAVATION IN SUCH A MANNER TO AVOID DAMAGE TO, AND MINIMIZE INTERFERENCE WITH, UNDERGROUND UTILITY LINES IN AND NEAR THE CONSTRUCTION AREA. 2. THE EXCAVATOR SHALL EXPOSE THE UNDERGROUND UTILITY LINE TO ITS EXTREMITIES BY HAND DIGGING WITHIN THE EXCAVATION AREA WHEN EXCAVATION IS EXPECTED TO COME WITHIN TWO FEET OF THE MARKED LOCATION OF THE UNDERGROUND UTILITY LINE. 3. THE EXCAVATOR SHALL NOT UTILIZE MECHANIZED EQUIPMENT WITHIN TWO FEET OF THE EXTREMITIES OF ALL EXPOSED UTILITY LINES. 4. THE EXCAVATOR SHALL MAINTAIN A REASONABLE CLEARANCE, TO INCLUDE THE WIDTH OF THE UTILITY LINE, IF KNOWN, PLUS 24 INCHES, BETWEEN THE MARKED OR STAKED LOCATION OF AN UNDERGROUND UTILITY LINE AND THE CUTTING EDGE OR POINT OF ANY MECHANIZED EQUIPMENT, CONSIDERING THE KNOWN LIMIT OF CONTROL OF THE CUTTING EDGE OR POINT TO AVOID DAMAGE TO THE UTILITY LINE. 5. THE EXCAVATOR SHALL PROVIDE PROPER SUPPORT FOR UNDERGROUND UTILITY LINES DURING EXCAVATION ACTIVITIES, DURING BACKFILL OPERATIONS, THE EXCAVATOR SHALL USE THE SAME OR SIMILAR BACKFILL MATERIAL THAT WAS ORIGINALLY AROUND THE UTILITY LINE, ENSURE THERE IS PROPER COMPACTION AROUND THE UTILITY LINE, PROTECT ALL TRACER WIRES, AND PROTECT OR REPLACE WARNING TAPES." 5. DISCONNECTED, PRIOR TO CLEARING THE SITE OF TREES, BUILDINGS, FOUNDATIONS, ETC. WITHIN THE LIMITS OF CONSTRUCTION IN ACCORDANCE WITH THE REQUIREMENTS INDICATED ON THE CONSTRUCTION PLANS. CONTRACTORS SHALL NOTIFY OPERATORS WHO MAINTAIN UNDERGROUND UTILITY LINES IN THE AREA OF PROPOSED CONSTRUCTION, EXCAVATION OR BLASTING AT LEAST 2 WORKING DAYS, BUT NOT MORE THAN 10 WORKING DAYS PRIOR TO COMMENCEMENT OF EXCAVATION OR DEMOLITION IN ACCORDANCE WITH CHAPTER 63 OF FAIRFAX COUNTY CODE. NAMES AND TELEPHONE NUMBERS OF SELECT OPERATORS OF UNDERGROUND UTILITY LINES IN FAIRFAX COUNTY APPEAR ON THIS SHEET. THESE NUMBERS WILL ALSO BE USED TO SERVE EMERGENCY CONDITION NOTICE AS REQUIRED BY CHAPTER 63 OF THE FAIRFAX COUNTY CODE. THIS IS NOT A COMPLETE LIST OF ALL UNDERGROUND UTILITY OPERATORS IN FAIRFAX COUNTY.

GENERAL NOTES 2: UTILIT

(THESE GENERAL NOTES SHALL BE USED WHERE THEY ARE APPLICABLE TO THE PROJECT PLANS)

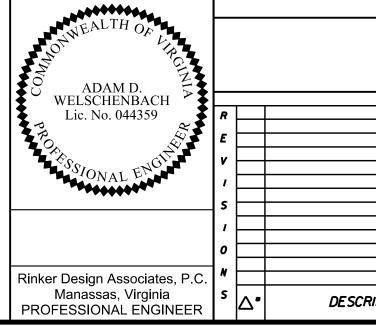
MISS UTILITY: CALL 1-800-552-7001 OR 811 VA811.COM PRIMARY UTILITY COMPANIES

PRIMARY UTILITY COMPANIES

AMERICAN TELEPHONE & TELEGRAPH CO. (AT&T) CITY OF FALLS CHURCH PUBLIC UTILITIES COLUMBIA GAS OF VIRGINIA COLUMBIA GAS TRANSMISSION CORP. COLONIAL PIPELINE CO. COX COMMUNICATIONS DOMINION TRANSMISSION, INC. DOMINION VIRGINIA POWER FAIRFAX CO. WASTEWATER COLLECTION DIVISION FAIRFAX WATER FAIRFAX COUNTY PUBLIC SAFETY MCI, WORLD WORLD COM. (METROPOLITAN FIBER SYSTEMS) NORTHERN VIRGINIA ELEC. CO-OP (NOVEC) TRANSCO GAS PIPELINE CO. TOWN OF HERNDON PUBLIC WORKS TOWN OF VIENNA WATER SERVICE SPRINT (GLOBAL ONE) UPPER OCCOQUAN SÉRVICE AUTHORITY (UOSA) SMART TRAFFIC SIGNAL SYSTEM PLANTATION PIPE LINE COMPANY VERIZON VIENNA WATER VIRGINIA AMERICAN WATER COMPANY WASHINGTON GAS WILLIAMS (TRANSCO) GAS PIPE LINE CORP.

Underground VDOT Owned/Maintained Utilities EMERGENCY 1-800-241-3624 1. Underground Virginia Department of Transportation (VDOT) Owned and/or Maintained (703-248-5044 24/7AFTER HOURS/EMERGENCIES 703-698-5613 utilities may be present within the project limits. These utilities may include power, 1-800-543-8911 - (703) 631-5363 (METRO) 1-800-835-7191 (24 HRS) communication or other utilities related to street/pedestrian lighting, Intelligent 1-800-926-2728 Traffic System (ITS) devices such as Variable Message Signs (VMS), traffic signals (703) - 378 - 0882and other related facilities. 1-888-264-8240 24/7 1-866-366-4357 2.Fairfax County has attempted to show any known underground VDOT owned or (703) 323-1211 maintained utilities on the plans based upon the best available information at the (703) 698-5600 OR 698-5613 (703)-691-2131 OR 911 time of design. i–800–624–9675 3. Miss Utility does not and will not mark the location of underground VDOT Owned (703) 852-6700 (703) 335-0500 and/or Maintained utilities within the project area. 1-800-440-8475 (24 HRS) 4. At least 48 business day hours in advance of any excavation, the Contractor shall (703) 435-6860 STATION 185 (703) 435-6846 (703) 255-6381 AFTER 5:00 PM., (703) 255-6385 be responsible for requesting that VDOT mark their underground utilities in the field. 1-800-521-0579 (24 HRS) All requests shall be submitted through the online ?Utility Marking System? by 703-830-2200 (703) - 383 - 2790registering at the following website: 1-800-510-5676 a. <u>http://www.vdotutilitymarking.virginia.gov/Account/Login.aspx?ReturnUrl=%2f</u> 1-800-837-4966 (703) - 255 - 63855. The 48 hour time limit does not begin until 7 AM the business day following receipt 1-800-452-6863 (703) 750-1000 (GAS LEAK (703) 750-4831) of the utility location request by VDOT. State holiday and weekends are not 1-800-440-8475 OR 703-368-3255 OFFICE considered to be business days. Underground VDOT utility location requests received after 4:30 PM shall be considered to be received by VDOT on the next business

day.



TRANSPORTATION PROJECTS/WORK WITHIN THE RIGHT-OF-WAY UTILITIES

6.No excavation shall commence until the underground VDOT utilities have been marked.

7. The Contractor shall be responsible for repair or replacement of underground VDOT owned/maintained utilities that are damaged due to construction operations at no cost to the County.

Utility Relocation Plans

1. Fairfax County has coordinated the proposed improvements with the owners of all known underground utilities in the project area.

2. Underground Utility Test Hole information, noting the horizontal and vertical location of known underground utilities that are in potential conflict with the project, may be included in the plan set or provided to the Contractor at the Pre-Construction Conference, or upon request following award of the construction contract. 3. Where underground utilities are in conflict with the project, Fairfax County often attempts to relocate conflicting utilities prior to the commencement of construction activities. However, as with all construction activities, extreme care shall be taken by the contractor to ensure utility locations are known prior to excavation. 4. Miss Utility may not have the most recent up to date information concerning underground utilities that may have been recently relocated prior to construction. 5. Fairfax County may provide available plans showing the approximate location of recently relocated underground utilities to the Contactor at the Pre-Construction Conference, or upon request following award of the Construction Contract. Where practicable, the utility relocation plans may be included in the Bid Documents. 6. Any known underground utilities that are in conflict with the project that are scheduled to be relocated during construction shall be as noted on the plans and/or special provisions. The Contractor shall be responsible for coordinating these relocations with the appropriate utility company.

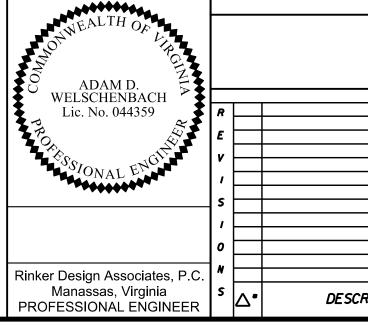
For All excavation work ANYWHERE in Virginia!

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Right of Way Data (For Fairfax County DOT)

Note: This sheet is for information only and for Fairfax County DOT tracking purposes only. This sheet is not to be used for actual Right of Way Acquisition. Please see prepared plats separately.

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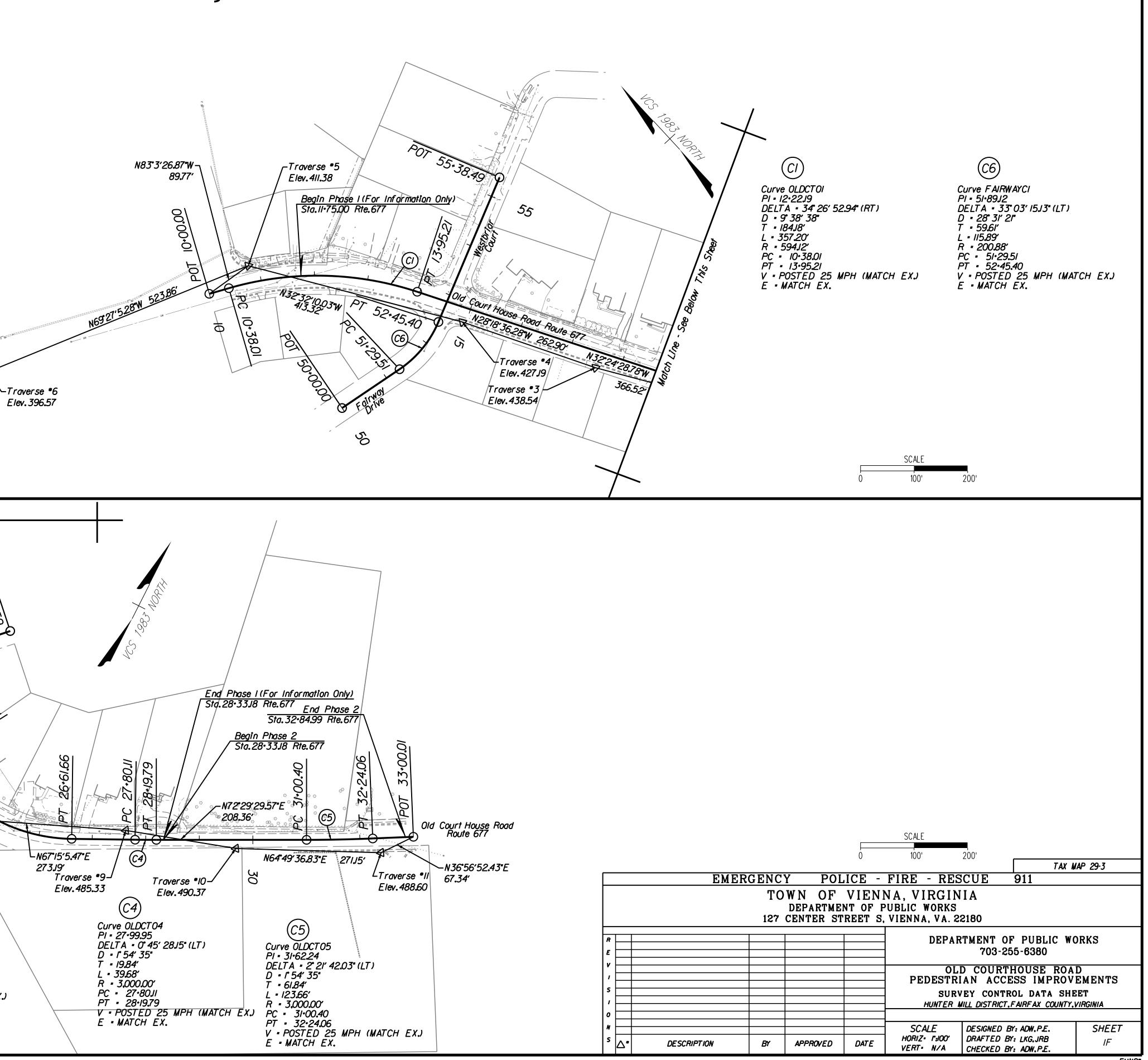


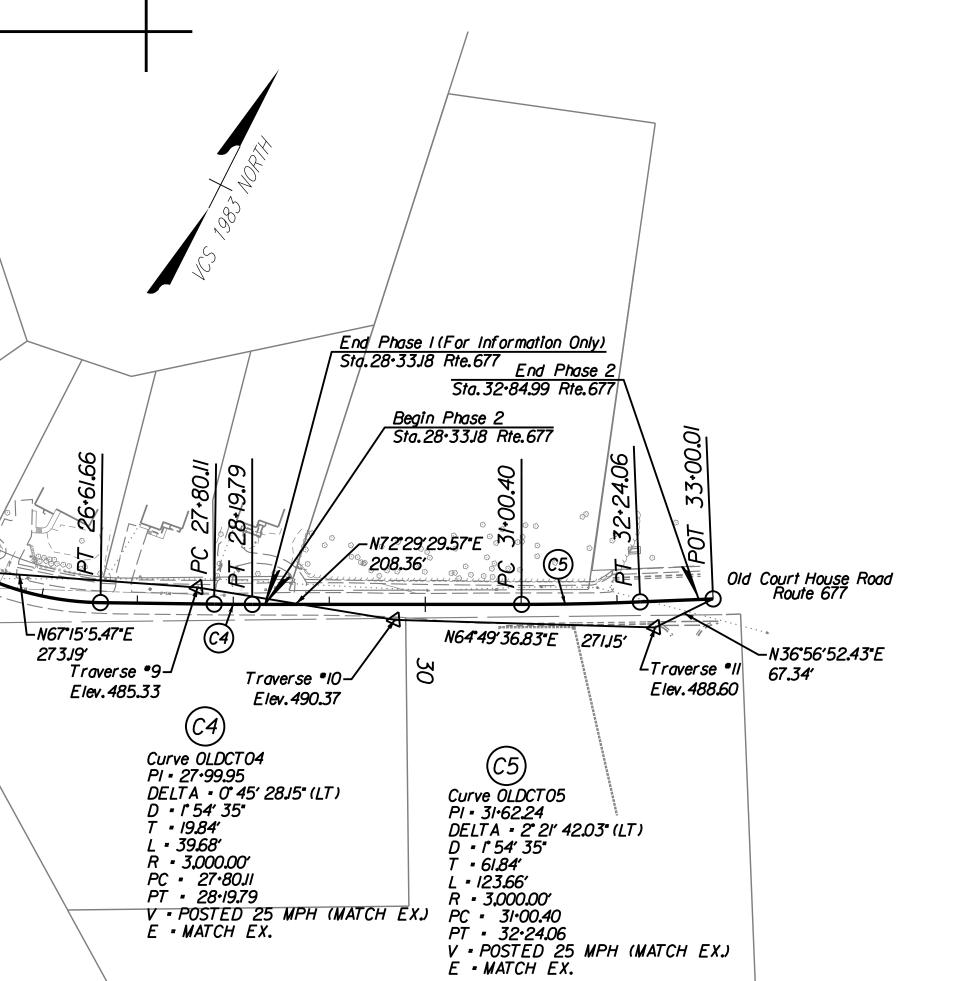
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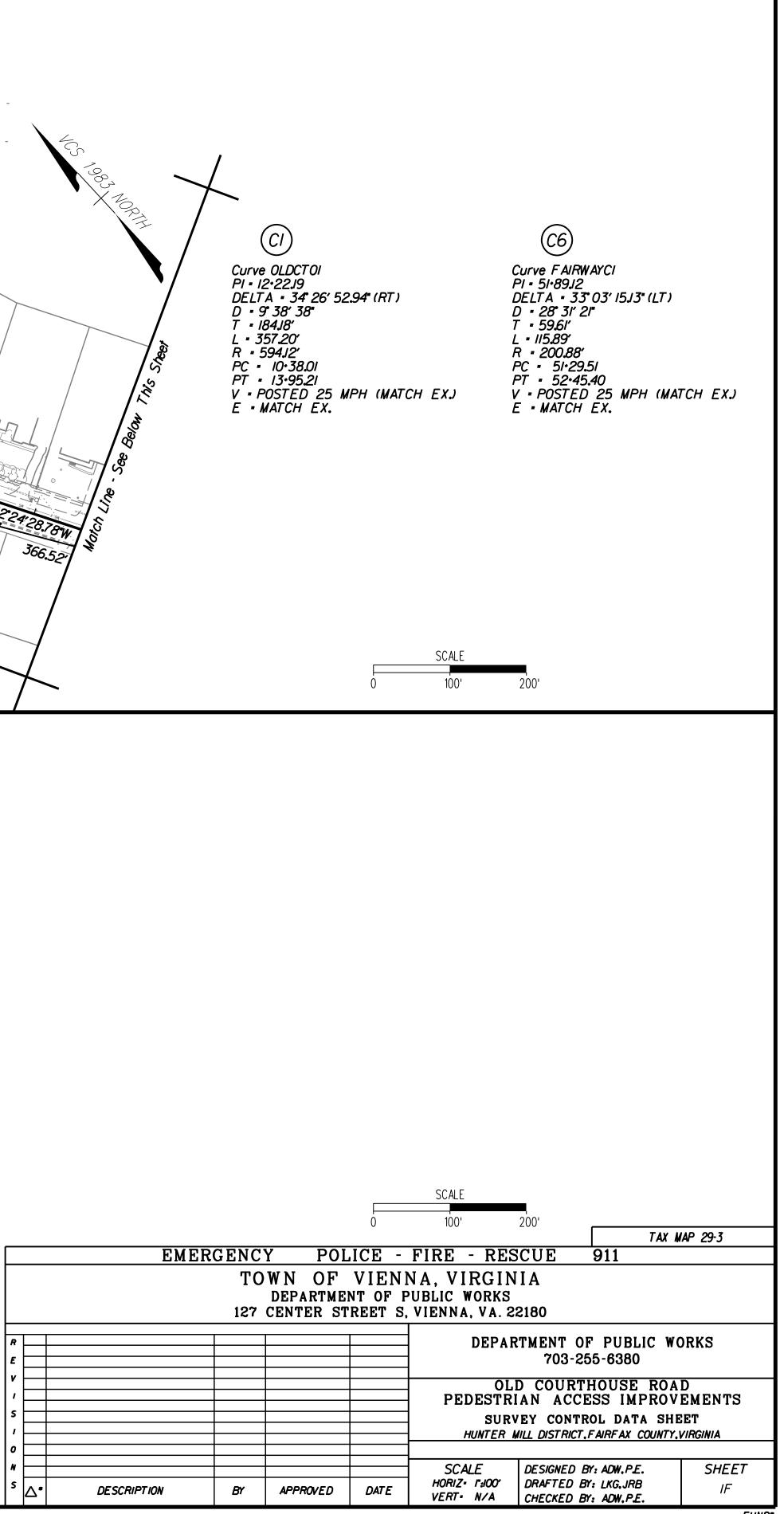
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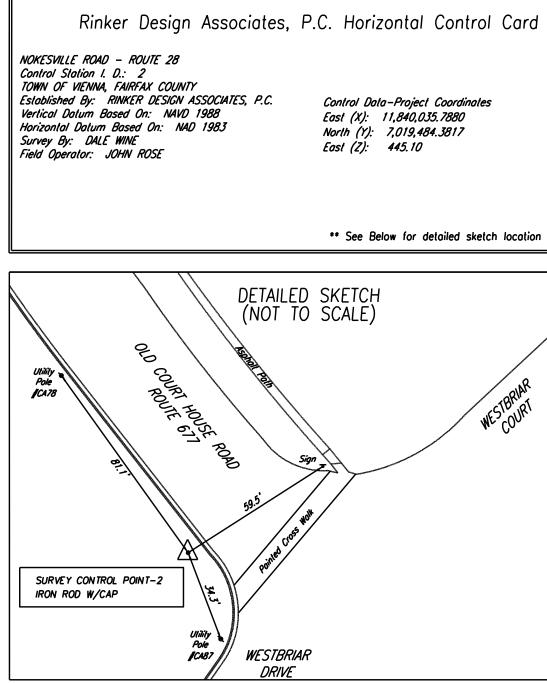
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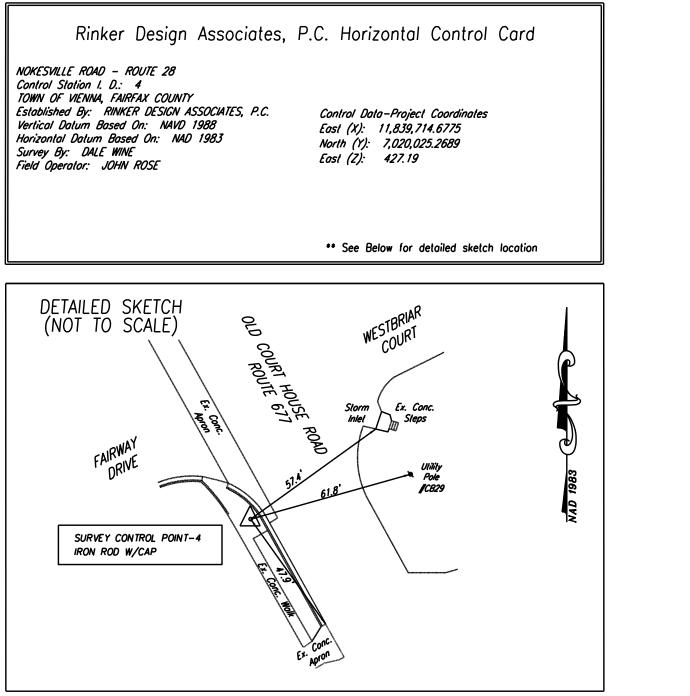
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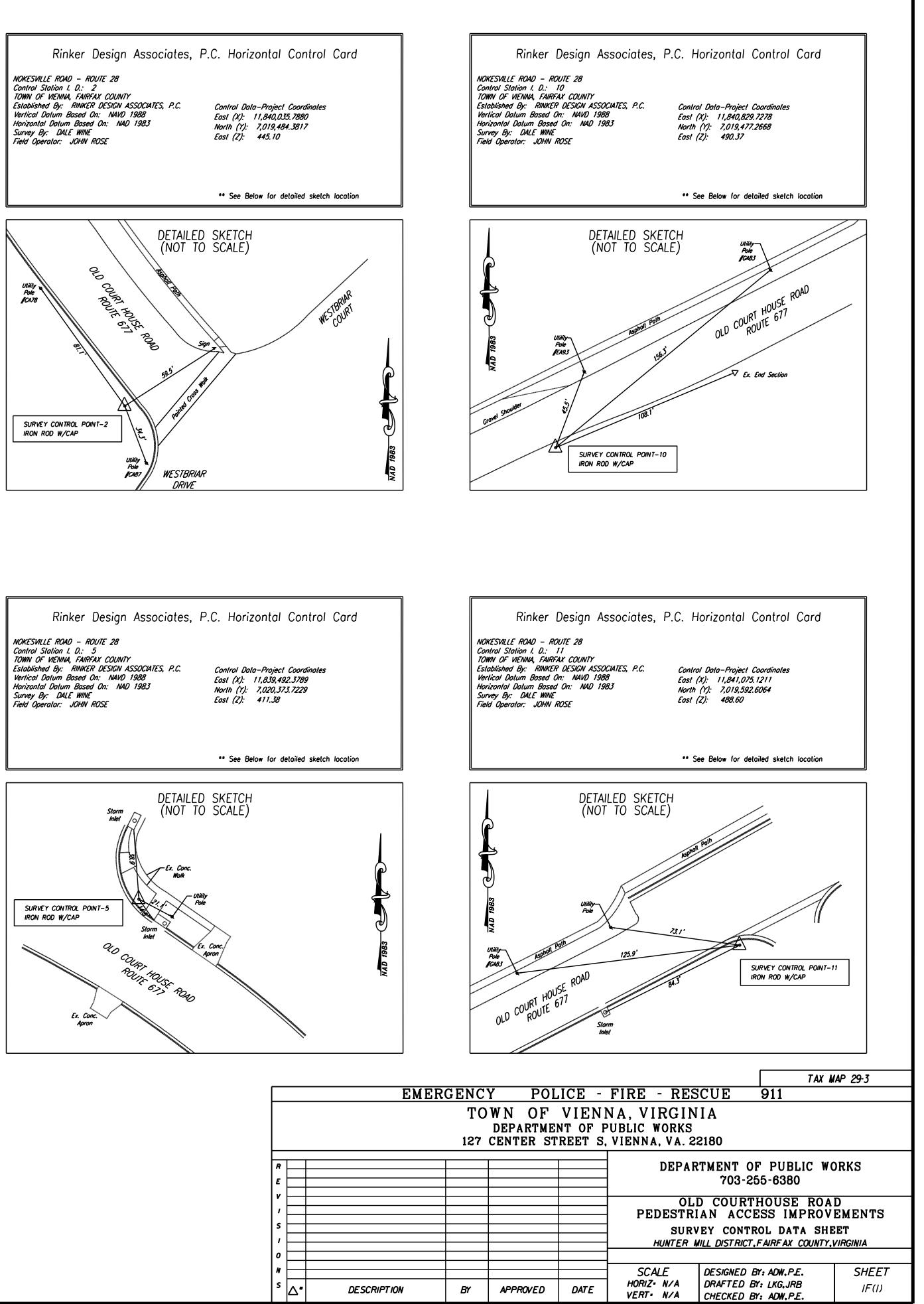


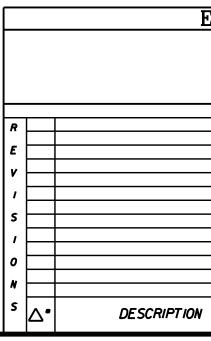










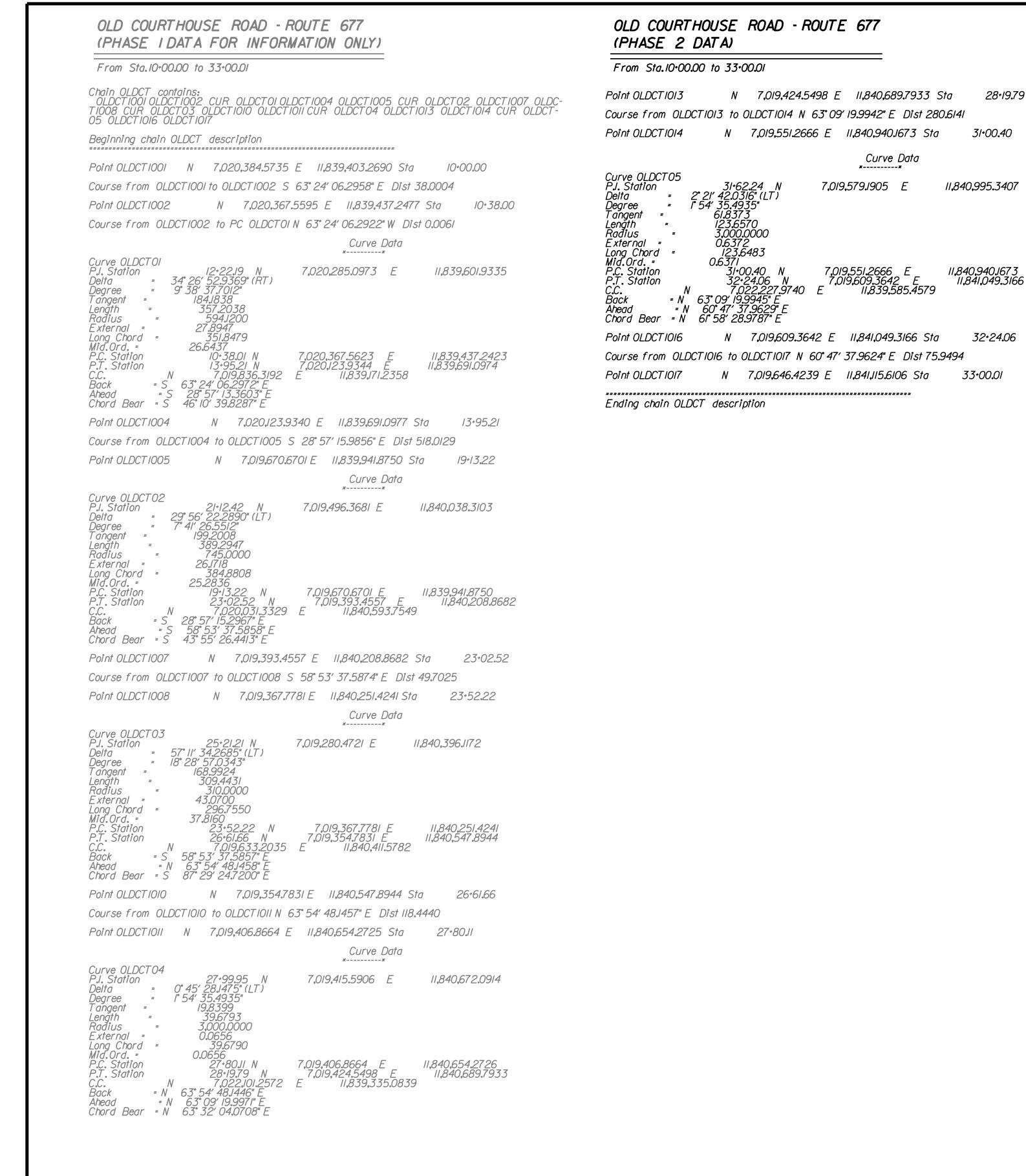


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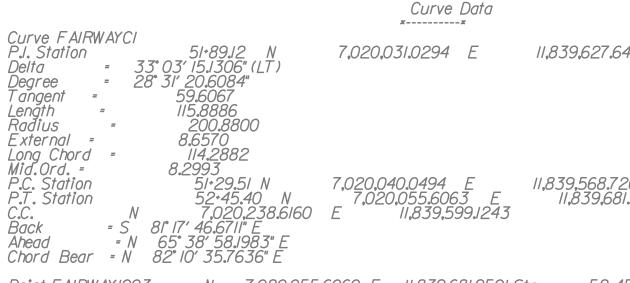
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FAIRWAY DRIVE (PHASE I DATA FOR INFORMATION ONLY)

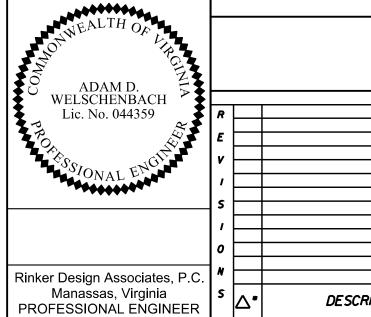
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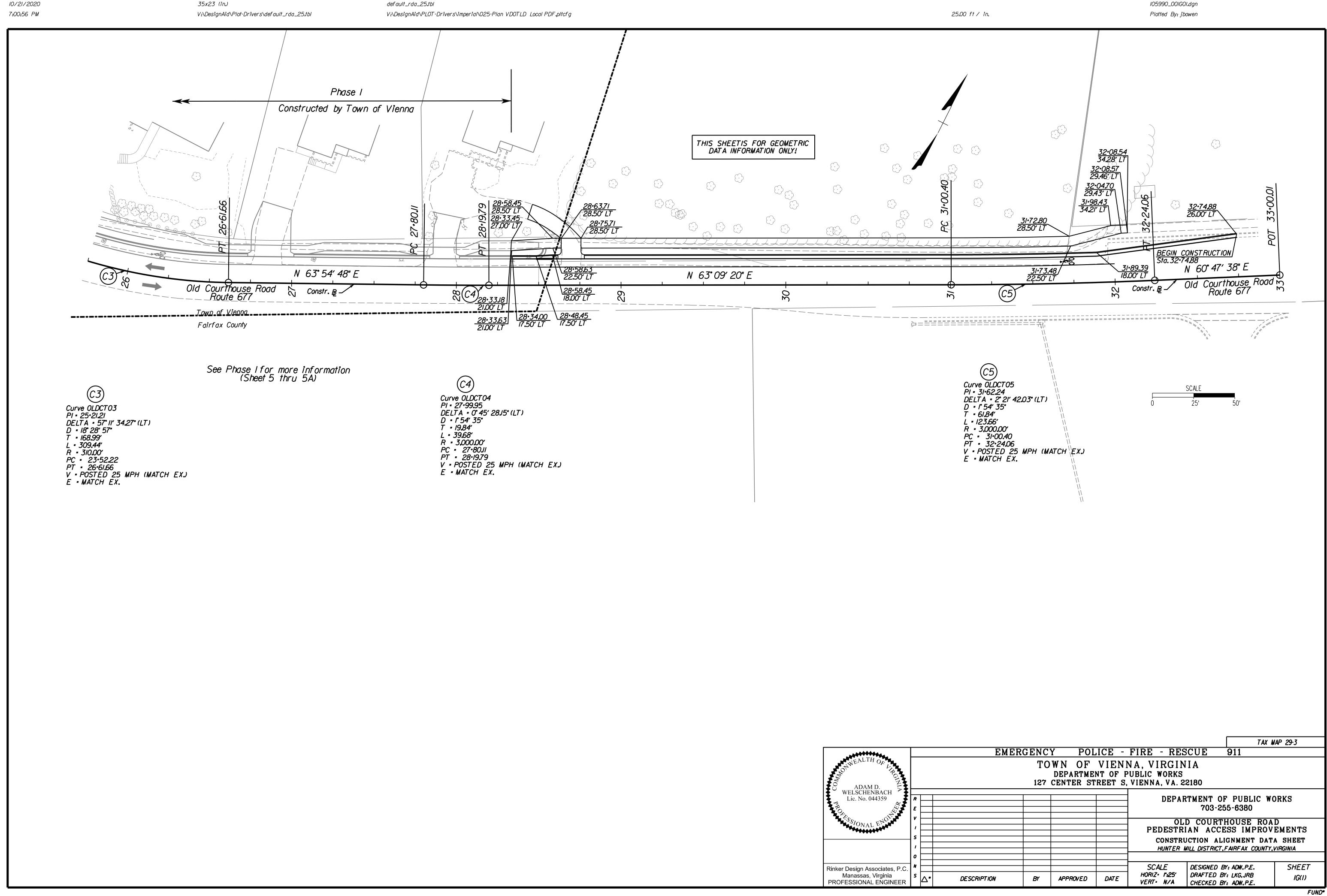
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Point FAIRWAY1003 7,020,055.6060 E II,839,681.9501 Sta 52+4 Course from FAIRWAY1003 to FAIRWAY1004 N 65° 38' 58.1997" E Dist 293.0897 Point FAIRWAY1004 N 7,020,176.4520 E 11,839,948.9665 Sta 55+*3* Ending chain FAIRWAYDR description



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	Temporary Traffic Control Plan
	General Notes:
1	TMP/SOC Type A Project Information
a	Identify the project's TMP Type: This project's TMP/SOC has been designed in conformance with a Type A TMP/SOC.
b	Identify the work zone location, length, and widths:
	The project location is as shown on Sheet I. The work zone areas have been delineated as shown on the TMP/SOC on Sheet IK Series.
	The work zone lengths and widths vary by location as shown on the TMP/SOC on Sheet IK Series.
C	Note the hours the Construction Area will be active: Construction Area shall be considered active when any impact to traffic occurs (Ist Cone in Road).
	Construction Area hours have the following limitations:
	LANE CLOSURES (NON MAJOR ARTERIAL)
	MONDAY TO THURSDAYFRIDAYSATURDAYSUNDAYDAY TIME9:00 AM to 3:30 PM9:00 AM to 2:00 PM* Not Allowed* Not Allowed
	NIGHT TIME * Not Allowed * Not Allowed * Not Allowed * Not Allowed
	* Night time and weekend work shall not be allowed unless approved by the Engineer.
	No lane closures will be allowed from noon on the day before a holiday until noon on the workday following the holi Holidays include all State and Federal holidays.
	Designation of Night Time Hours and Peak Hour Times:
·	Peak hours are 6:00am through 9:00am & 3:30pm through 6:30pm.Monday through Friday.
đ	The TMP/SOC, during construction, shall be in accordance with Sections 512,701,703,& 704 of the Virginia Departme of Transportation's <u>Road and Bridge Specifications</u> dated 2016, the <u>Virginia Work Area Protection Manual</u> , 2011 Editi Revised January 2015, and the current <u>Manual on Uniform Traffic Control Devices</u> (MUTCD), 2009 Edition, all revision therein.
e	Note any existing entrances, existing intersections, or existing pedestrian access points that will be affected by the Construction Area or by the traffic control devices:
	I) Existing Entrances:
	All existing private entrances ad jacent to the project shall remain open for the duration of construction.
	2) <u>Existing Intersections</u> : There are no existing unsignalized intersections within the project limits.
	3) Existing Pedestrian Access Points:
	Within the project limits, pedestrian access points are generally at the intersections and along both sides of roadways within project limits. The Contractor is to maintain adequate direction and guidance for pedestrian and bicyclists within the project site for the duration of construction.
	4) <u>Existing Bus Stops</u> : There are no bus stops within the project limits.
f	Identify the major types of travelers:
	The roadway carries diverse types of travelers, but the prevailing type of traveler is daily commuters.
g	The Contractor shall:
	Designate a person assigned to the project who will have the primary responsibility, with sufficient authority, fo implementing the TMP/SOC and other safety and mobility aspects of the permit work. This person shall be designated the "Project Safety Officer."
	Ensure that personnel assigned to the project are trained in traffic control to a level commensurate with their responsibilities in accordance with VDOT's work zone traffic control training guidelines.
	Inform the Fairfax County Construction Manager and VDOT of any work requiring lane shifts, lane closures, and/or phase changes a minimum of two weeks prior to implementing this activity.Contractor shall be respons for applying (including any cost) for any noise waivers through coordination with FCDOT to do night time work at no additional cost to the project.
	Perform reviews of the Construction Area to ensure compliance with contract documents at regularly scheduled intervals at the direction of the Fairfax County Construction Manager and/or VDOT.
	Maintain a copy of the temporary traffic control plan at the work site at all times.
	Coordinate with the Fairfax County Police Department, Fairfax County Fire/Rescue Department, and Virginia /S Police for any lane closures and any detours of any nature.
	Schedule all phases of construction in such a manner that water, sanitary sewer, cable, fiber cable/optic cable, any overhanging utilities, and any underground utilities services will not be interrupted.

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Temporary Traffic Control Plan Notes

- This TMP/SOC is intended as a guide. It is not to enumerate every detail which must be considered in the construction of each phase, but only to show the general handling of existing traffic. It shall be the responsibility of the Contractor to present a formal TMP/SOC with construction signage to the Engineer for approval prior to any construction activity that may affect the existing traffic.
- Contractor shall maintain at least one lane of traffic in each direction on Albemarle Drive, Edgehill Drive, and Monticello Road during construction of this project with a minimum clear travelway width in accordance with the guidelines of VDOT standard GS-10 unless otherwise approved by the Engineer. For street intersections, commercial connections, or private entrances, a minimum width no less than existing width shall be maintained at all times, unless approved by the Engineer.
- No Concrete Traffic Barrier Service shall be installed for construction of this project. 4
- The Contractor shall follow the geotechnical recommendations for the project. Materials designated as unsuitable materials as detailed in the geotechnical recommendations shall be disposed of offsite and are 5 not to be used for any part of construction.
- The Contractor shall ensure positive drainage for the duration of the project. The Contractor shall add any additional temporary measures necessary to facilitate proper, positive drainage for the duration of 6 construction.
- 7 All areas excavated below the existing pavement surface and within the clear zone at the conclusion of each workday, shall be backfilled to form an approximate 6: wedge against the existing pavement or newly constructed pavement surface for the safety and protection of vehicular traffic.
- 8 Each phase of construction shall be completed to the installation of intermediate course asphalt prior to the start of the next phase unless otherwise directed by the Engineer.
- Where Group 2 Channelizing Devices are used to separate the Construction Area and traffic, a minimum clear zone area as defined in the VWAPM shall be maintained. 9
- Group 2 Channelizing Devices shown on the plan are schematic. The Contractor shall follow the VWAPM 10 guidelines for device spacing along transitions and parallel to the travelway.
- The Contractor shall coordinate with Fairfax County for location(s) of the construction staging area(s). The 11 Contractor is responsible for obtaining all permits and/or easements as necessary.
- IMPLEMENTING THE TRANSPORTATION MANAGEMENT PLAN During the first day of the new work zone traffic pattern, the project's Manager and project's Construction Inspector shall inspect the work zone to ensure compliance with the TMP. On the third 12 day of Implementation of the TMP's new work zone traffic pattern, the Construction Inspector an on-site review of the work zone's performance in coordination with VDOT and recommend t Contractor any required changes to the TMP to enhance the work zone's safety and mobility. All changes shall be documented. An on-site review of the project's work zone traffic control by the Construction Inspector and the Contractor shall be conducted (with coordination from VDOT) with hours of any fatal incident/crash within the work zone.
- EVALUATION OF THE TRANSPORTATION MANAGEMENT PLAN A performance assessment of the TMP including area wide impacts on adjacent roadways shal performed by Fairfax County with coordination from VDOT Engineers during construction. As 13 circumstances dictate, a review of the overall effectiveness of the project's TMP shall be com during the Post Construction Meeting and included with the Post Construction Report. A copy of specific information on the effectiveness of the TMP will be forwarded to Fairfax County for VDOT coordination. A copy of the TMP Interim/Post Construction Report Form can be obtained Fairfax County with coordination with VDOT.
- PUBLIC COMMUNICATIONS PLAN The Contractor shall be responsible for:

- Notifying the Project Manager and Construction Inspector two weeks in advance of any a scheduled work plans and traffic delays.
- Notifying the Project Manager, Construction Inspector, and corresponding VDOT Engineer of any b unscheduled traffic delays.
- TRANSPORTATION OPERATIONS The Contractor shall be responsible for implementing and providing the following: 15
- Notify the Regional Transportation Operations Center (TOC) 7 days in advance, usually the Wednesday of the week prior, in order to place lane closure information on the 511 System and VA-Traffic.
- Post a list of local emergency response agencies inside the project's construction office/trailed
- Immediately report any traffic incidents that may occur in the work zone.
- Notify the project's Construction Inspector and corresponding VDOT Engineer of any incidents and expected traffic delays.
- Within 24 hours of any incidents within the construction work zone, a review of the traffic controls shall be completed and necessary adjustments made to reduce the frequency and severity of any future incidents.

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TMP/SOC General Notes

CONTACT NUMBERS

Town Dept. Public Works Director Town Construction Manager Town Construction Inspector

Emergency Call

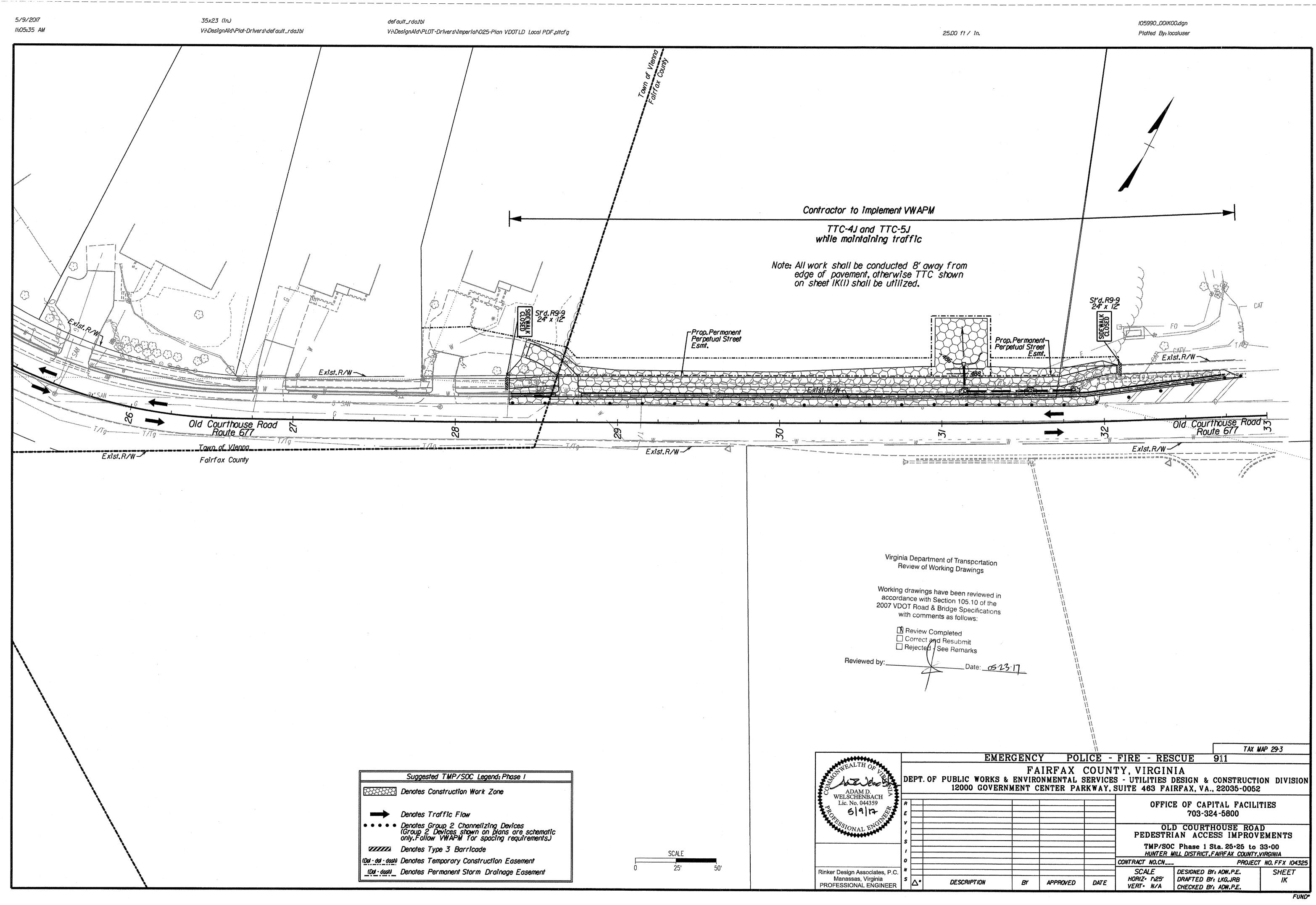
Non-Emergency Numbers: Fairfax County Police Fairfax County Fire & Rescue

Michael Gallagher, (703) 255-6380 Luis Blandon, (703) 255-5723 Luis Blandon, (703) 255-5723

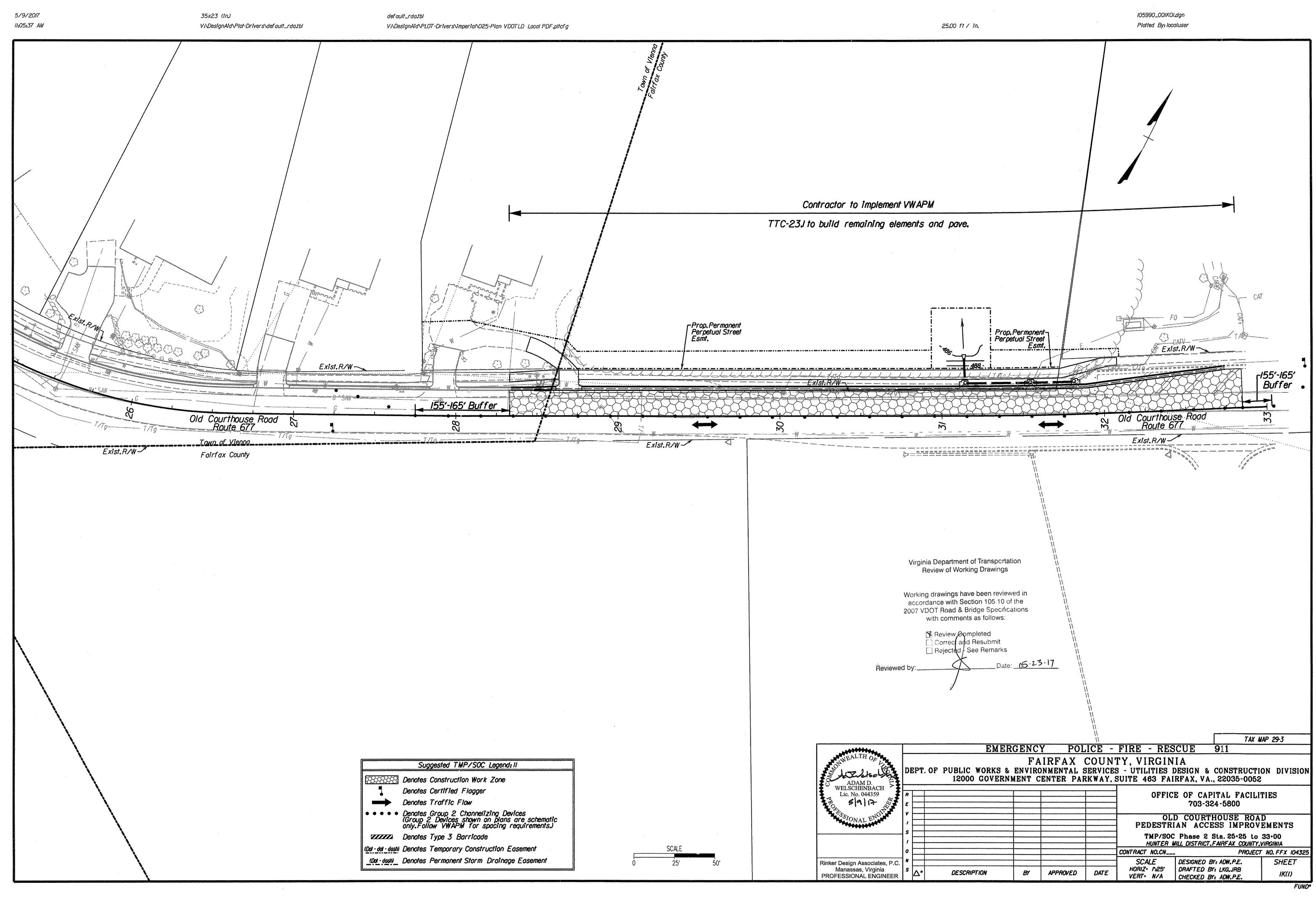
911

(703) 246-2253 (703) 246-2126

TMP/SOC Designer



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Erosion and Sediment Control Narrative

Project Description: This is a pedestrian access improvement project along Old Courthouse Road NE between Pine Valley Drive and Gosnell Road in the Town of Vienna, Virginia. The project proposes to add curb and gutter, sidewalk, curb ramps with pedestrian crossings at Westbriar Court. Additionally, all drainage will be collected and conveyed via a new proposed closed storm sewer system. The project is located in the Wolftrap Creek watershed management area which is within the greater Difficult Run watershed. The land disturbance area for this phase of the project is 0.22 ac.

Existing Site Conditions: The project site is along Old Courthouse Road NE between Pine Valley Drive and Gosnell Road. Vegetation within the project site consists of landscaped lawns and some large trees. Storm runoff is collected by roadside ditches and conveyed to four outfalls via existing closed storm sewer systems.

Adjacent Areas: Areas adjacent to the project are mostly residential or commercial in nature.

Off-site Areas: There will be minimal impacts to adjacent parcels associated with the construction of this project. All necessary right of way, right of entry agreements, easements, and provisions will be acquired prior to the start of construction. The Contractor shall be responsible for the locations of acceptable borrow and/or disposal sites, and these shall be in accordance with Town of Vienna requirements or as directed by the Town.

Soils: See soils map located on this sheet.

Critical Areas: There are no critical areas within the project site.

Erosion and Sediment Control Measures: Water quality and sediment/erosion control are of extreme importance. Care must be taken to avoid discharge of sediment into the existing storm water system. In order to best control impacts on this watershed, all vegetative and structural sediment control practices shall be constructed and maintained according to minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook. Strict compliance with this program and standards is required. We are therefore specifying a plan to minimize impacts on the ad jacent properties.

At the time of land disturbing activities within the Town right-of-way, the Contractor shall have a representative with Erosion and Sediment Control Contractor Certification (ESCCC) at the project site. The Town and Contractor are responsible for complying with applicable Local, State, and Federal Environmental Laws and Regulations, including acquiring clearances/authorizations from appropriate regulatory agencies.

Land Disturbing/Construction Sequence - Phase I I. The Contractor shall install the silt fence and inlet protection as shown on the Phase I Erosion & Sediment Control plan. 2. After the silt fence and inlet protection have been installed, the Contractor shall

obtain the site inspector's approval of these controls. 3. After the site inspector's approval of the initial controls, clear and grub the site as necessary.

Land Disturbing/Construction Sequence - Phase 2

I. Fine grade the site.

2. Install curb & gutter, sidewalk, and entrance base course and concrete pavement. 3. Install all permanent sod and fertilize all grassed areas.

4. Clean site of all trash and debris.

5. Have the inspector inspect all areas to determine if they are adequately stabilized.

Maintenance Program: The Contractor shall make a visual inspection of all mechanical controls and newly stabilized areas (i.e. seeded, mulched, or sodded areas) on a daily basis and after each rainfall event to insure that all controls are functioning properly. The following items will be checked in particular: inlet protection will be checked regularly for sediment buildup which will prevent drainage, and if the gravel is clogged by sediment, it shall be removed and cleaned or replaced; the silt fence barrier will be checked regularly for undermining or deterioration of the fabric, and sediment shall be removed when the level of sediment deposition reaches halfway to the top of the barrier; and the seeded areas will be checked regularly to ensure that a good stand is maintained, and areas shall be fertilized and reseeded as needed. Any damaged controls shall be repaired by the end of the work day, including reseeding and mulching if necessary. The Contractor may install additional measures should be or she deem if necessary at the inspector's approval. All erosion & sediment controls shall be removed within seven (7) days after the project is stabilized.

Structural Practices:

I. Silt Fence Barrier (3.05) - Silt fence barriers will be installed downslope of areas with minimal grades to filter sediment-laden runoff from sheet flow as indicated in the Erosion and Sediment control plans.

2. Storm Drain Inlet Protection (3.07) - All storm sewer inlets shall be protected during construction. Sediment-laden water shall be filtered before entering the storm sewer inlets.

3. Storm Drain Outlet Protection (3.18) - All storm sewer outlets shall be protected during construction.

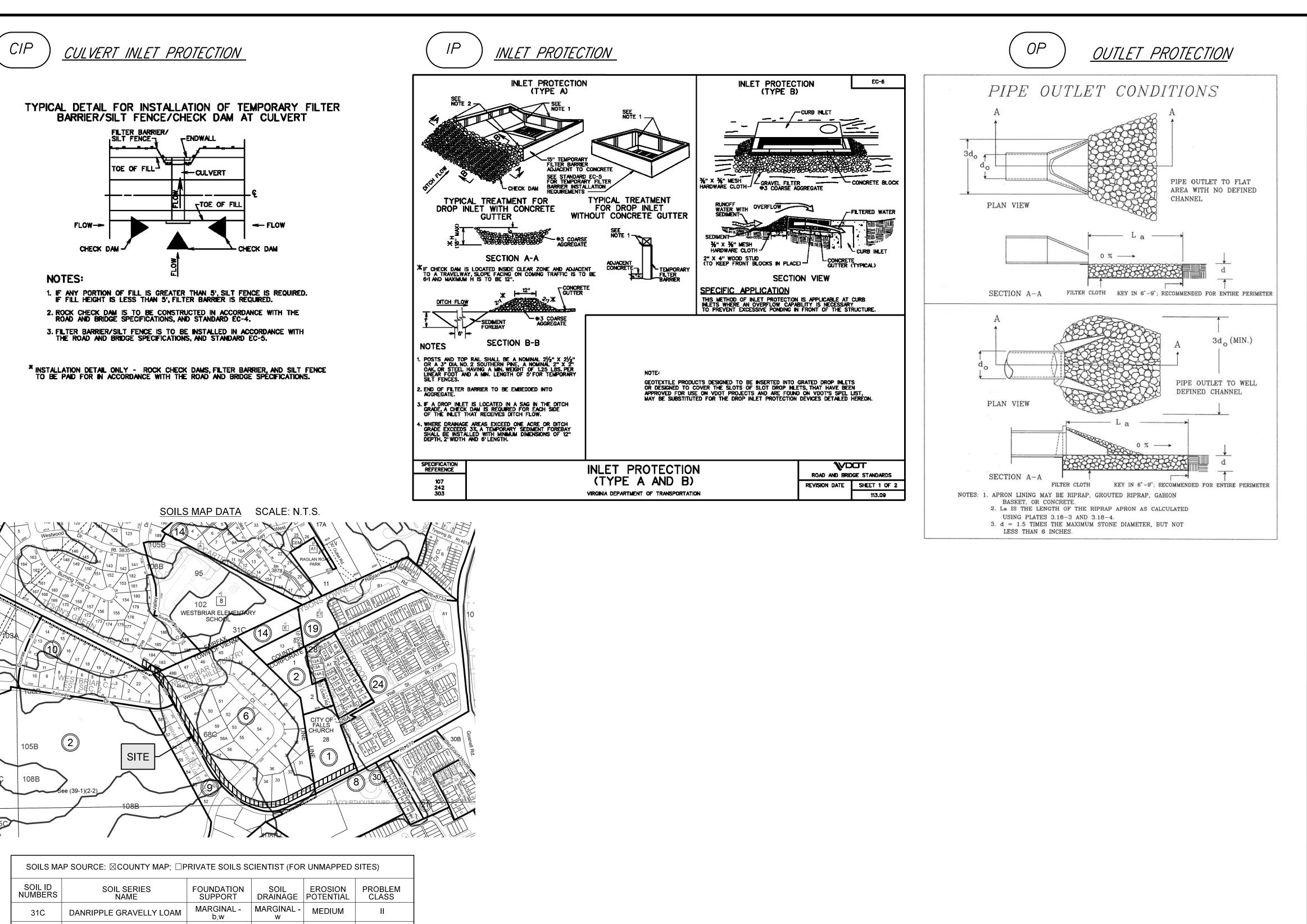
4. Temporary Seeding (3.31) - All denuded areas which will be left dormant for extended periods of time shall be seeded with fast germinating temporary vegetation immediately following grading. Selection of the seed mixture will depend on the time of year it is applied.

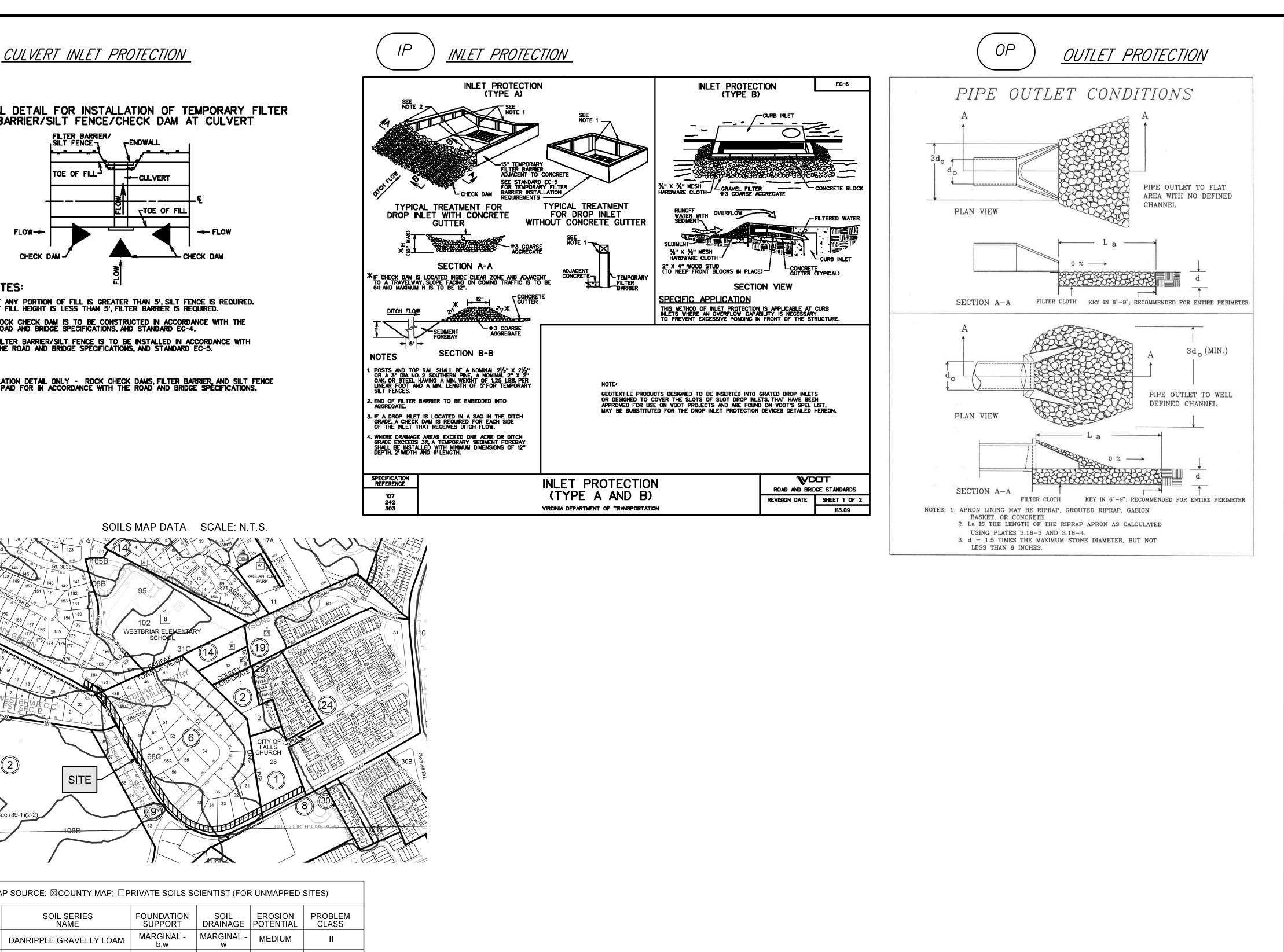
5. Permanent Seeding (3.32) - Perennial vegetative cover shall be established on disturbed areas by planting seed to reduce erosion and decrease sediment yield and to permanently stabilize disturbed areas. Selection of the seed mixture will depend on the time of year it is applied. The planting soil shall be applied in accordance with Std. 3.30.

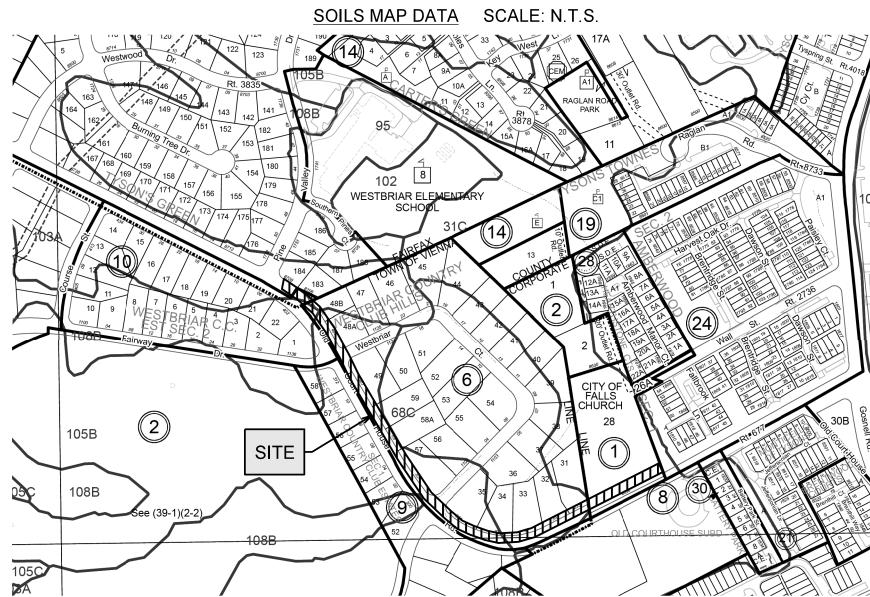
6. Permanent Stabilization - Permanent stabilization shall be done in accordance with the VESCH and all Town of Vienna seeding standards.

Stormwater Runoff Considerations: See sheet 2K series for Storm Computations and Outfall Analysis for this project.

Dust Control: Contractor shall be responsible to control dust throughout the entire construction phase by the application of water and/or approved adhesives per Std. 3.39 of the Virginia Erosion and Sediment Control Handbook.







SOILS MA	NP SOURCE: ⊠COUNTY MAP; □P	RIVATE SO
SOIL ID NUMBERS	SOIL SERIES NAME	FOUNDAT SUPPOF
31C	DANRIPPLE GRAVELLY LOAM	MARGINA b,w
68C	KINGSTOWNE DANRIPPLE COMPLEX	MARGINA w, b
103A	WHEATON CODORUS COMPLEX	POOR f, w, b
105B	WHEATON GLENELG COMPLEX	GOOD
108B	WHEATON SYMERDUCK COMPLEX	MARGINA w, b
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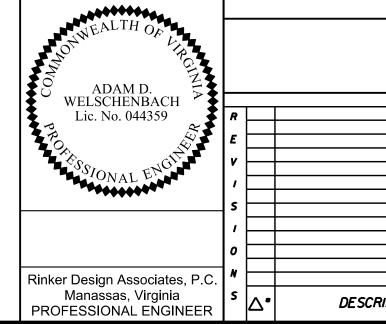
SOILS MAPPED OVER NATUALLY OCCURING BEDROCK. THESE SOILS OCCUR WITHIN A GEOLOGIC FORMATION KNOWN AS THE PINEY BRANCH COMPLEX, LOCALLY KNOWN AS GREENSTONE NATURALLY-OCCURRING ASBESTOS MINERALS, PREDOMINANTLY ACTINOLITE AND TREMOLITE, ARE KNOWN TO OCCUR IN THIS FORMATION. EXCAVATIONS IN BEDROCK OR EARTH MOVING ACTIVITIES WITHIN THIS FORMATION MAY EXPOSE THESE MINERALS TO THE ATMOSPHERE, ALLOWING THE FIBERS TO BECOME AIRBORNE.



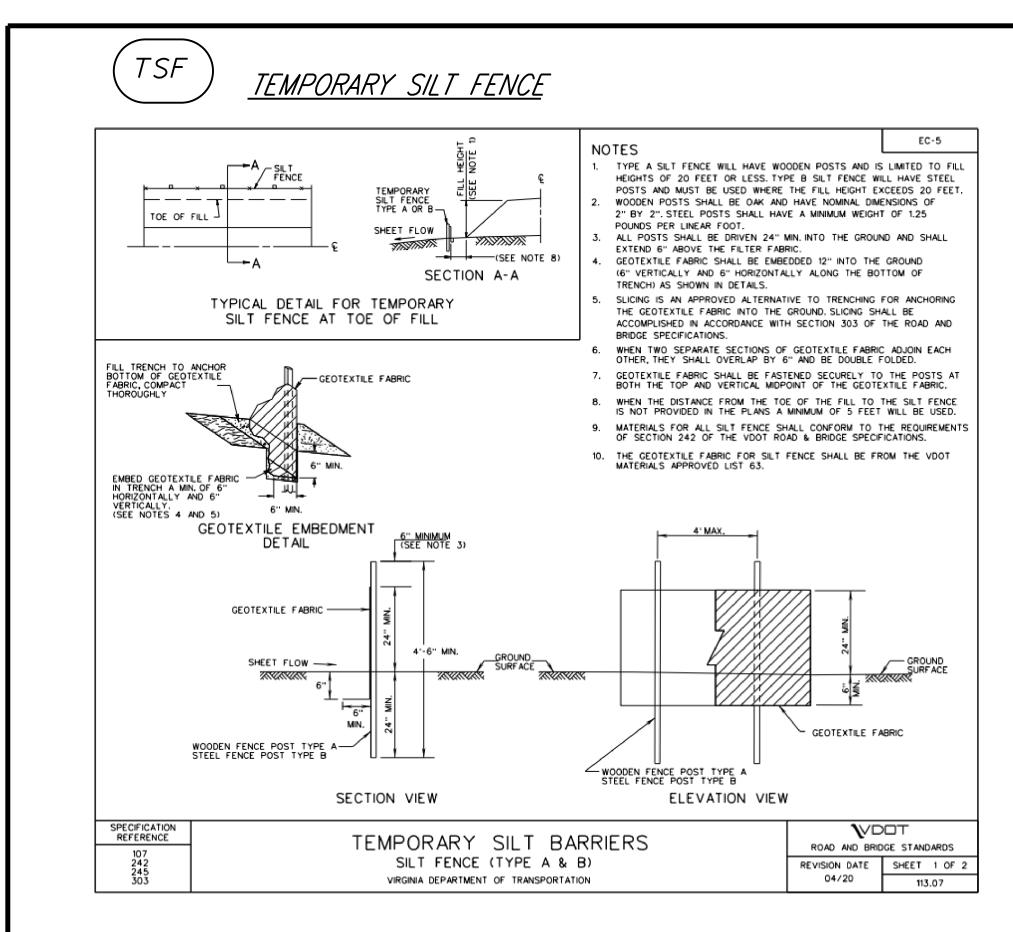
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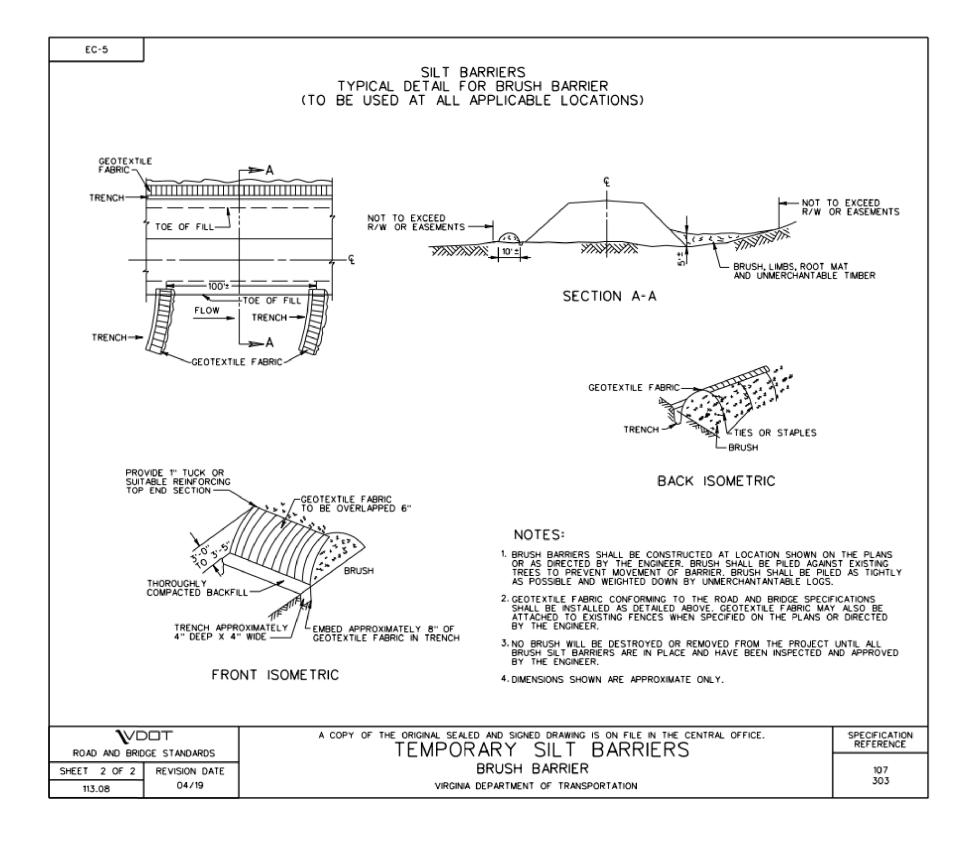
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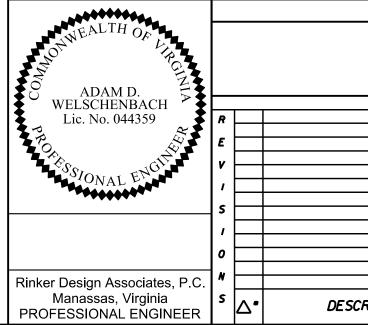
Y OCCURING ASBESTOS SOILS ARE LOCATED ON ON THE COUNTY WEBSITE. SPECIAL PRECAUTIONS RIGINATING FROM THESE SOILS ARE REQUIRED BY REGULATIONS ENFORCED BY THE VIRGINIA TRY AND SPECIAL GUIDANCE HAS BEEN ISSUED BY



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Erosion Control/Land Conservation Notes

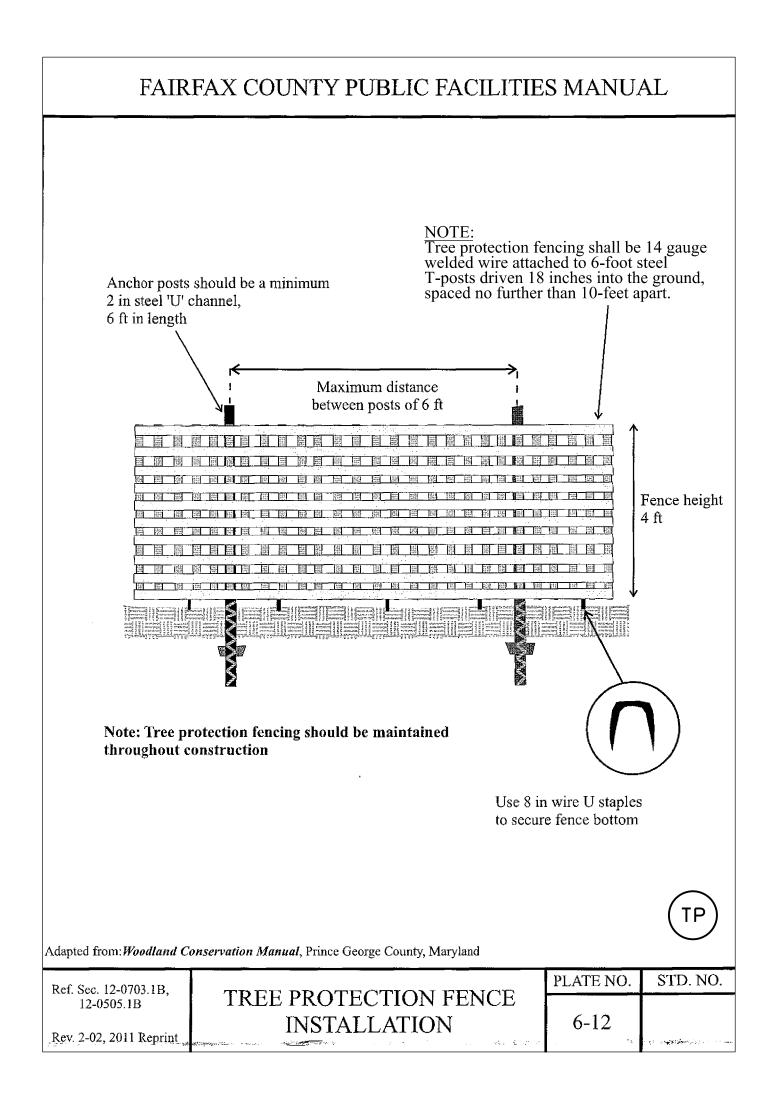
- I. The owner must notify the Department of Public Works at (703) 792-7070 at least 24 hours prior to the start of construction in accordance with applicable county ordinances and policies.
- 2. The owner grants the right-of-entry on to this property to the designated Fairfax County personnel for the purpose of inspecting and monitoring for compliance with Title 10.01, Chapter 5, Article 4 of the Code of Virginia, Erosion and Sediment Control Law and the Design and Construction Standards Manual Section 750.04 (c).
- 3. All erosion control measures shown on the approved plan must be in place and inspected and approved by the Department of Public Works prior to clearing, stripping of topsoil or grading.
- 4. A copy of the approved erosion and sediment control plan and permit shall be kept on the site at all times. 5. Contractor shall be responsible for additional erosion control measures may be required to
- prevent erosion and sedimentation as determined by Fairfax County inspector. 6. All disturbed areas are to drain to approved sediment control measures at all times during land disturbing activities and during site development until complete and adequate
- stabilization is achieved. 7.Water must be pumped into an approved filtering device during dewatering operations. 8. All erosion and sediment control practices must be constructed and maintained according to the minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook and the Virginia Regulations VR 625-02-00 Erosion and Sediment Control Regulations and to the Fairfax County Public Facilities Manual (PFM). The information contained in the construction plans and/or the approval of the plans shall in no way relieve the contractor or his agent of any legal responsibility which may be
- required by the Code of Virginia or any ordinance enacted by the County of Fairfax. 9. The contractor/contractor's representative will be responsible for the installation and maintenance of all erosion and sediment control practices at all times.
- IO. The contracotr/contractor's representative shall inspect all erosion and sediment control measures daily and after each significant rainfall. Any necessary repairs or cleanup to maintain the effectiveness of the erosion control devices must be made immediately after the inspection.
- II. Sediment trapping measures will be installed as a first step in grading and will be seeded and mulched immediately following installation.
- 12. All temporary erosion and sediment control measures are to be removed within 30 days after adequate site stabilization and after the temporary measures are no longer needed. as authorized by the Fairfax County inspectors.
- 13. If sediment is transported onto a paved road surface, the road will be cleaned thoroughly at the end of each day.sediment will be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing will be allowed only after sediment is removed in this manner.
- 14. Areas which are not to be disturbed (i.e. limits of disturbance) are to be clearly marked by flags, signs, etc. 15. Tree save areas shall be clearly marked in the field by tree protection measures as
- shown herein. 16. Orange safety fence must be installed around all silt traps and sediment basins (if
- applicable). 17. During construction of the project, soil stockpiles, and borrow areas shall be stabilized or protected with appropriate sediment control measures.
- 18. In accordance with state and federal job safety requirements, all excavated material is to be placed on the uphill side of trenches. no material is to be placed in stream. any stockpiled material which will remain in place longer than 14 days is to be seeded for temporary vegetation and mulched.where soil is placed on downhill side of trench, the pump discharge hose must outlet in a stabilized area or an approved filtering device auring aewatering operations.
- 19. For further requirements and details of tree preservation, planting, erosion and sediment control, see Fairfax County Public Facilities Manual and/or the Virginia Erosion and Sediment Control Handbook.
- 20. No more than 500 LF of trench may be opened at one time. Only that amount of trench that can be backfilled that same day shall be excavated within the workday. 21. Material used for backfilling trenches shall be properly compacted in order to minimize
- erosion and promote stabilization. 22. The grading/excavation contractor for the subject site is required to notify, in writing, the assigned site inspector regarding any excess material proposed to be hauled offsite prior to hauling. The notification must indicate the quantity of material to be moved off site, identification of the receiving site where the excess will be taken, and all information necessary to show that such receiving site has been properly permitted and has E&S controls installed.

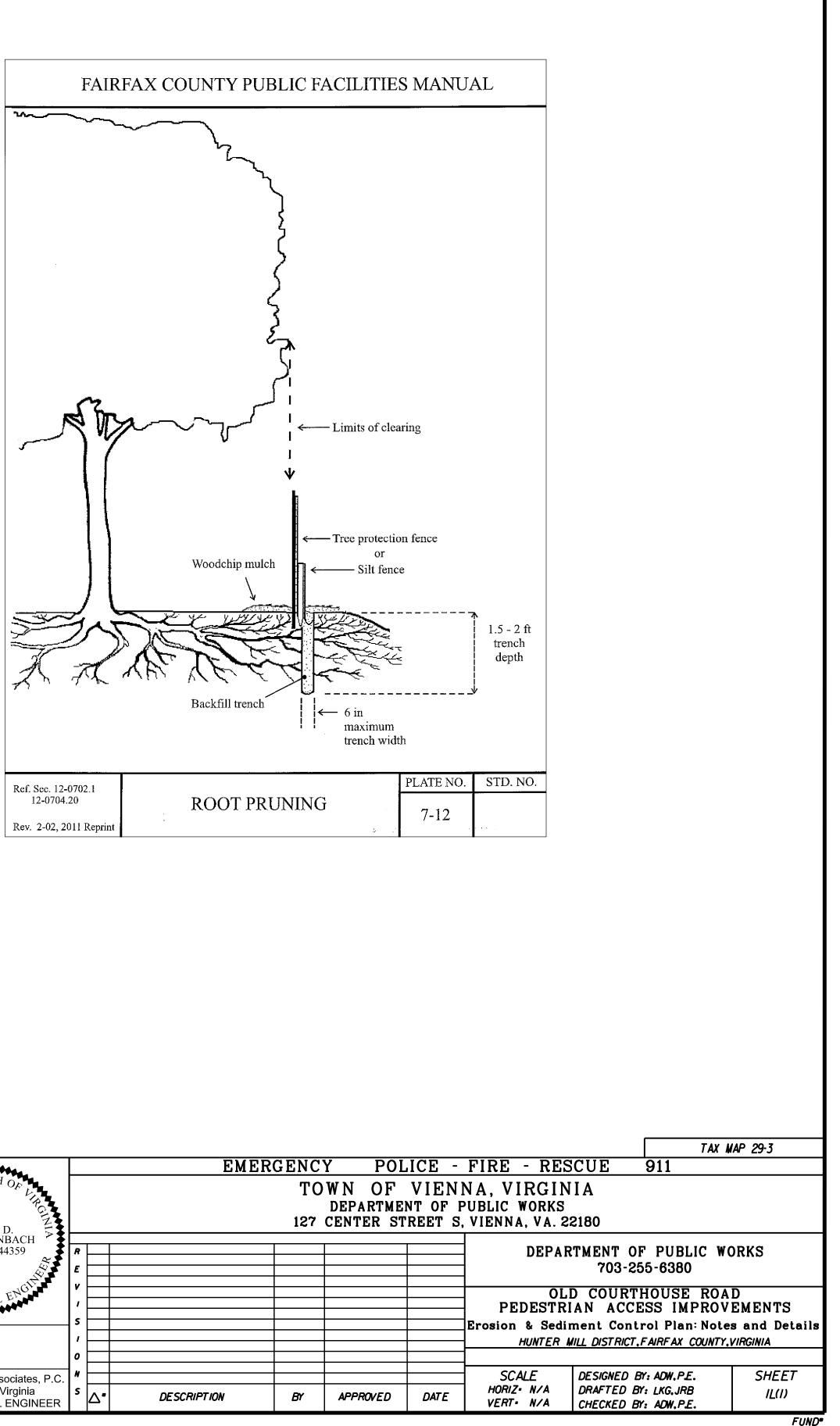
Management Strategies - Erosion & Sediment Control:

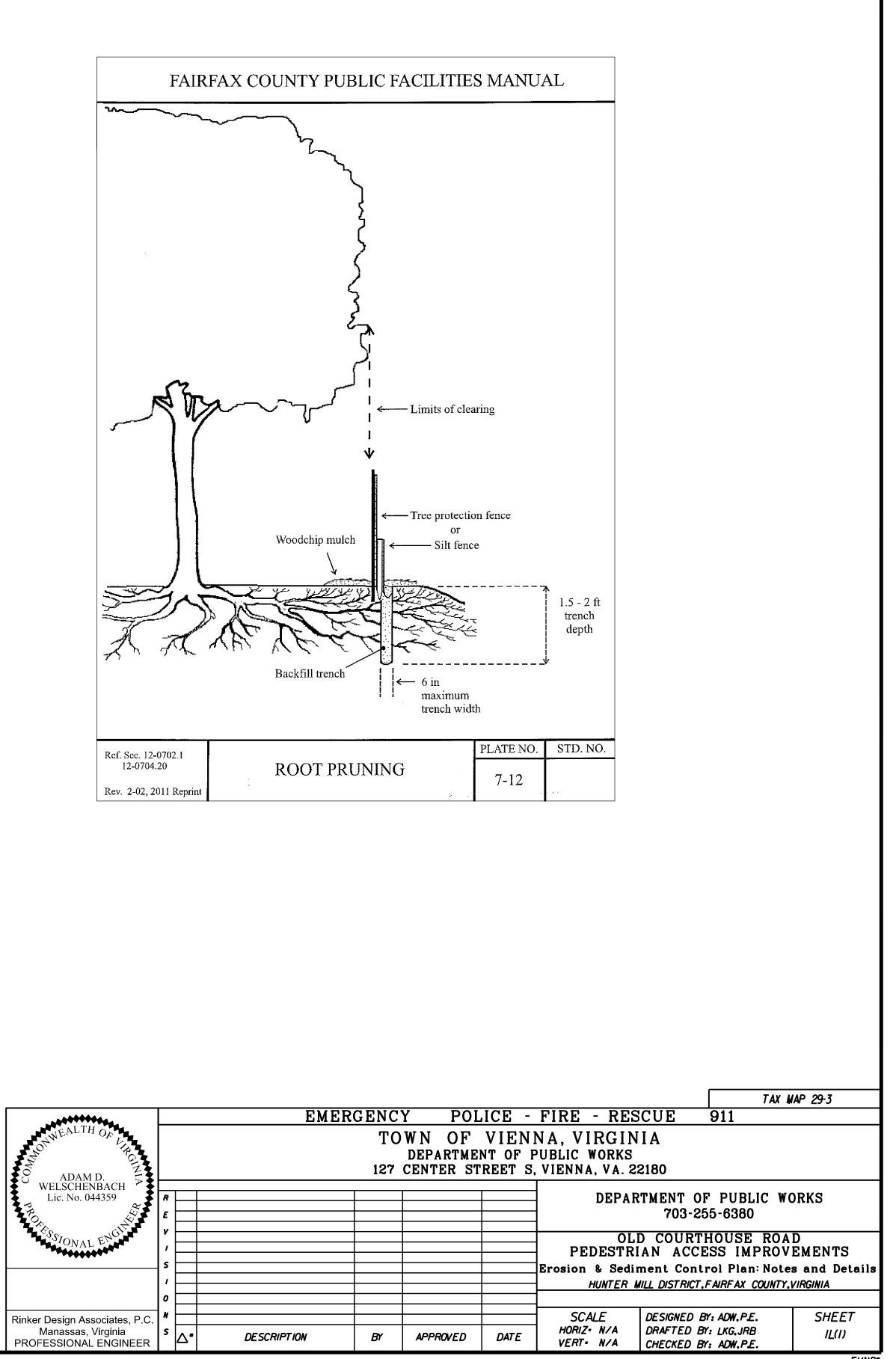
- Perimeter controls will be in place prior to major on-site earthwork activities. It is intended that perimeter controls be maintained throughout the earthwork phase and/or until upstream areas have been stabilized.stabilization shall consist of permanent seeding for grassed areas. It is the responsibility of the contractor to stockpile trench soil carefully within the construction easement until the county inspector approves the newly constructed sanitary sewer, if applicable. The erosion and sediment control program shall progress as follows:
- I.Flag limits of clearing & grading and hold pre-construction meeting.
- 2.The staging and any other access points shall be coordinated the county inspector. No dirt laden vehicles shall be permitted to leave the site and onto any road unless they are clean. Any mud or dirt deposited in the parking area shall be removed immediately. Soil stockpilling shall have appropriate E&S control measures to adequately handle soil laden runoff.
- 3. Install construction entrance in the locations shown on the plans, or in a location approved by the county inspector.
- 4. Provide minimum grubbing to allow E&S installation. 5. Install perimeter controls per the appropriate construction method. (The contractor may choose to install the perimeter controls all at once or linearly, whichever one is more appropriate if approved by the county inspector.)
- 6. Provide minimum grubbing to allow for temporary stage/storage areas. These areas chosen shall have sediment trapping measures as necessary and provided stabilization. These areas to be further investigated by the contractor and the county inspector during the pre-construction meeting, and shall be verified by a field investigation.
- 7. Construction of proposed trail and associated grading shall commence. Contractor to coordinate with the county inspector to determine whether seed or sod is appropriate in questionable areas.
- 8. For vegetative stabilization of all denuded areas see Erosion Control Measures and Structural Practices.
- 9. Once all areas are stabilized to the satisfaction of the county inspector the contractor shall remove perimeter controls. In accordance with Section 6-1503.3 of the Fairfax County Public Facilities Manual and once the site inspector has inspected the stabilized areas, mechanical sediment controls shall be removed. all denuded areas are to be permanently stabilized with permanent vegetation upon the approval of the Fairfax County inspector.

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Erosion & Sediment Control Notes & Details







4VAC50-30-40.<u>Minimum Standards</u>.(MS-I9)

A VESCP must be consistent with the following criteria.techniques and methods:

- I. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 14 days. Permanent stabilization shall be applied to areas that are to be left dormant for more than one year.
- 2. During construction of the project, soil stock piles and borrow areas shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as borrow areas and soil intentionally transported from the project site.
- 3.A permanent vegetative cover shall be established on denuded areas not otherwise permanently stabilized. Permanent vegetation shall not be considered established until a ground cover is achieved that is uniform, mature enough to survive and will inhibit erosion.
- 4. Sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment shall be constructed as a first step in any land-disturbing activity and shall be made functional before upslope land disturbance takes place.
- 5. Stabilization measures shall be applied to earthen structures such as dams, dikes and diversions immediately after installation.
- 6. Sediment traps and sediment basins shall be designed and constructed based upon the total drainage area to be served by the trap or basin. a. The minimum storage capacity of a sediment trap shall be 134 cubic
- yards per acre of drainage area and the trap shall only control drainage areas less than three acres.
- b. Surface runoff from disturbed areas that is comprised of flow from drainage areas greater than or equal to three acres shall be controlled by a sediment basin. The minimum storage capacity of a sediment basin shall be 134 cubic yards per acre of drainage area. The outfall system shall, at a minimum, maintain the structural integrity of the basin during a 25-year storm of 24-hour duration. Runoff coefficients used in runoff calculations shall correspond to a bare earth condition or those conditions expected to exist while the sediment basin is utilized.
- 7. Cut and fill slopes shall be designed and constructed in a manner that will minimize erosion. Slopes that are found to be eroding excessively within one year of permanent stabilization shall be provided with additional slope stabilizing measures until the problem is corrected.
- 8. Concentrated runoff shall not flow down cut or fill slopes unless contained within an adequate temporary or permanent channel, flume or slope drain structure.
- 9.Whenever water seeps from a slope face, adequate drainage or other protection shall be provided.
- IO. All storm sewer inlets that are made operable during construction shall be protected so that sediment-laden water cannot enter the conveyance system without first being filtered or otherwise treated to remove sediment.
- II. Before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel.
- 12. When work in a live watercourse is performed, precautions shall be taken to minimize encroachment.control sediment transport and stabilize the work area to the greatest extent possible during construction. Non-erodible material shall be used for the construction of causeways and cofferdams. Earthen fill may be used for these structures if armored by non-erodible cover materials.

13. When a live watercourse must be crossed by construction vehicles more than twice in any six-month period, a temporary vehicular stream crossing constructed of non-erodible material shall be provided.

14. All applicable federal, state and local chapters pertaining to working in or crossing live watercourses shall be met.

15. The bed and banks of a watercourse shall be stabilized immediately after work in the watercourse is completed.

16. Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria: a. No more than 500 linear feet of trench may be opened at one time.

b. Excavated material shall be placed on the uphill side of trenches.

- c. Effluent from dewatering operations shall be filtered or passed through an approved sediment trapping device, or both, and discharged in a manner that does not adversely affect flowing streams or off-site property.
- d.Material used for backfilling trenches shall be properly compacted in order to minimize erosion and promote stabilization. e. Restabilization shall be accomplished in accordance with this
- chapter. f. Applicable safety chapters shall be complied with.

Erosion & Sediment Control Notes & Details

- land-disturbing activities.
- further erosion and sedimentation.
- 19. Properties and waterways downstream from development sites shall be natural or man-made channels:
- the outfall of the pipe or pipe system shall be performed.
- manner:
- contributing drainage area of the project in question; or 2)
- of channel bed or banks.
- pipe or system.
- channels or pipes are not adequate, the applicant shall: channel the bed or banks; or
- storm is contained within the appurtenances;
- man-made channel; or
- downstream erosion.
- d. The applicant shall provide evidence of permission to make the improvements.
- project.
- the facility to the receiving channel.
- h. All on-site channels must be verified to be adequate.
- pipe or pipe system, or to a detention facility. all engineering calculations.

17. Where construction vehicle access routes intersect paved or public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a paved or public road surface, the road surface shall be cleaned thoroughly at the end of each day. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner. This provision shall apply to individual development lots as well as to larger

18. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization or after the temporary measures are no longer needed, unless otherwise authorized by the VESCP authority. Trapped sediment and the disturbed soil areas resulting from the disposition of temporary measures shall be permanently stabilized to prevent

protected from sediment deposition, erosion and damage due to increases in volume, velocity and peak flow rate of stormwater runoff for the stated frequency storm of 24-hour duration in accordance with the following standards and criteria. Stream restoration and relocation projects that incorporate natural channel design concepts are not man-made channels and shall be exempt from any flow rate capacity and velocity requirements for

a. Concentrated stormwater runoff leaving a development site shall be discharged directly into an adequate natural or man-made receiving channel, pipe or storm sewer system. For those sites where runoff is discharged into a pipe or pipe system, downstream stability analyses at b. Adequacy of all channels and pipes shall be verified in the following

I) The applicant shall demonstrate that the total drainage area to the point of analysis within the channel is one hundred times greater than the

a)Natural channels shall be analyzed by the use of a two-year storm to verify that stormwater will not overtop channel banks nor cause erosion

b) All previously constructed man-made channels shall be analyzed by the use of a ten-year storm to verify that stormwater will not overtop its banks and by the use of a two-year storm to demonstrate that stormwater will not cause erosion of channel bed or banks; and c) Pipes and storm sewer systems shall be analyzed by the use of a ten-year storm to verify that stormwater will be contained within the

c. If existing natural receiving channels or previously constructed man-made 1) Improve the channels to a condition where a ten-year storm will not overtop the banks and a two-year storm will not cause erosion to

2) Improve the pipe or pipe system to a condition where the ten-year

3) Develop a site design that will not cause the pre-development peak runoff rate from a two-year storm to increase when runoff outfalls into a natural channel or will not cause the pre-development peak runoff rate from a ten-year storm to increase when runoff outfalls into a

4) Provide a combination of channel improvement, stormwater detention or other measures which is satisfactory to the VESCP authority to prevent

e. All hydrologic analyses shall be based on the existing watershed characteristics and the ultimate development condition of the subject

f. If the applicant chooses an option that includes stormwater detention, he shall obtain approval from the VESCP of a plan for maintenance of the detention facilities. The plan shall set forth the maintenance requirements of the facility and the person responsible for performing the maintenance. g.Outfall from a detention facility shall be discharged to a receiving channel, and energy dissipators shall be placed at the outfall of all detention facilities as necessary to provide a stabilized transition from

i. Increased volumes of sheet flows that may cause erosion or sedimentation on ad jacent property shall be diverted to a stable outlet, adequate channel,

j. In applying these stormwater management criteria, individual lots or parcels in a residential, commercial or industrial development shall not be

considered to be separate development projects. Instead, the development, as a whole, shall be considered to be a single development project. Hydrologic parameters that reflect the ultimate development condition shall be used in

- k. All measures used to protect properties and waterways shall be employed in a manner which minimizes impacts on the physical, chemical and biological integrity of rivers, streams and other waters of the state.
- I. Any plan approved prior to July 1,2014, that provides for stormwater management that addresses any flow rate capacity and velocity requirements for natural or man-made channels shall satisfy the flow rate capacity and velocity requirements for natural or man-made channels if the practices are designed to (i) detain the water quality volume and to release it over 48 hours; (ii) detain and release over a 24-hour period the expected rainfall resulting from the one year, 24-hour storm; and (iii) reduce the allowable peak flow rate resulting from the 1.5,2, and 10-year, 24-hour storms to a level that is less than or equal to the peak flow rate from the site assuming it was in a good forested condition, achieved through multiplication of the forested peak flow rate by a reduction factor that is equal to the runoff volume from the site when it was in a accord forested condition divided by the runoff volume from the site in its proposed condition, and shall be exempt from any flow rate capacity and velocity requirements for natural or man-made channels as defined in any regulations promulgated pursuant to 10.1-562 or 10.1-570 of the Act.
- m.For plans approved on and after July 1,2014, the flow rate capacity and velocity requirements of IOJ-56I A of the Act and this subsection shall be satisfied by compliance with water quantity requirements in the Stormwater Management Act (10.1-603.2 et seq. of the Code of Virginia) and attendant regulations, unless such land-disturbing activities are in accordance with 4VAC50-60-48 of the Virginia Stormwater Management Program (VSMP) Permit Regulations.
- n.Compliance with the water quantity minimum standards set out in 4VAC50-60-66 of the Virginia Stormwater Management Program (VSMP) Permit Regulations shall be deemed to satisfy the requirements of Minimum Standard 19.

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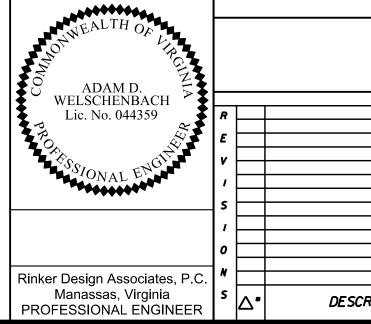
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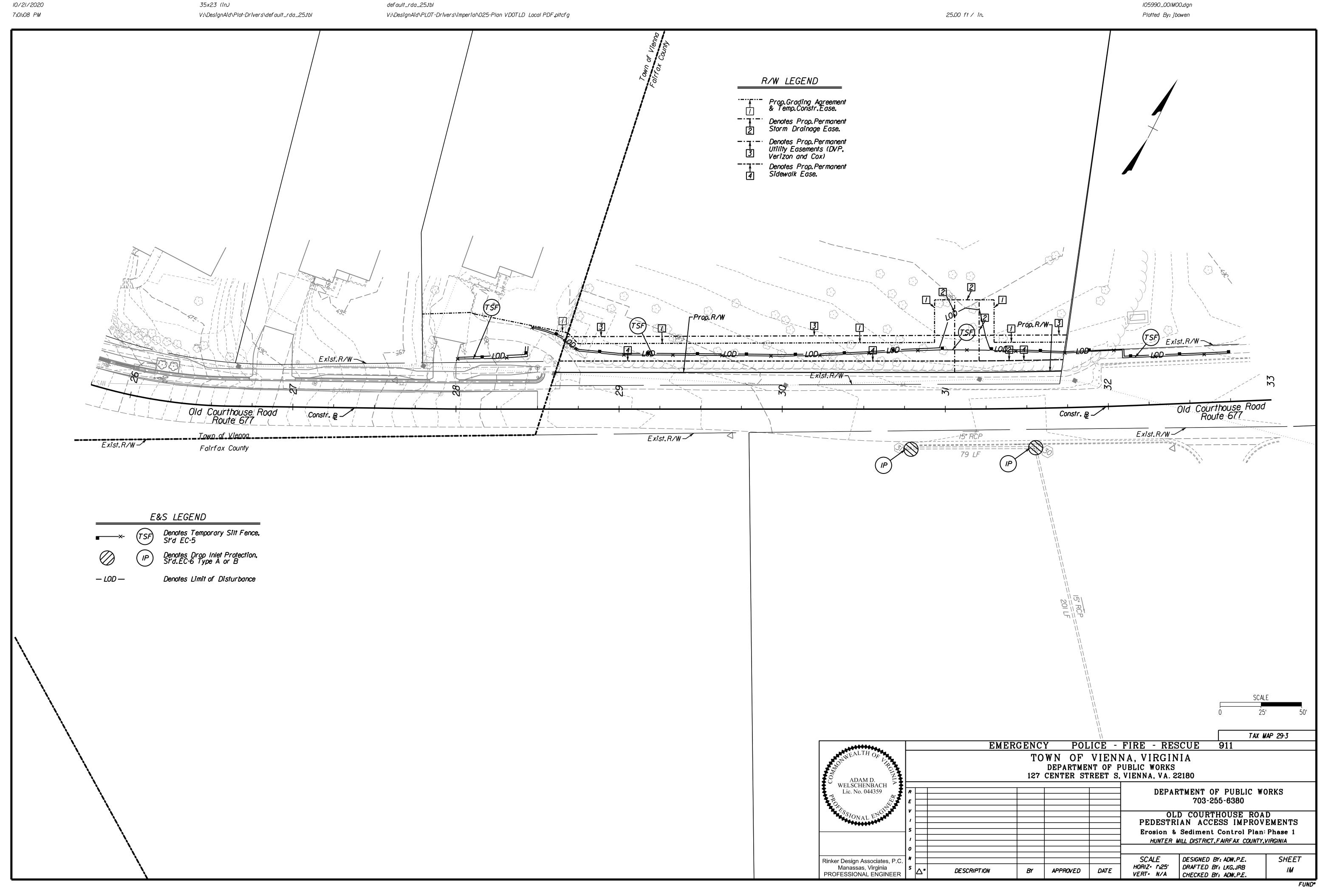
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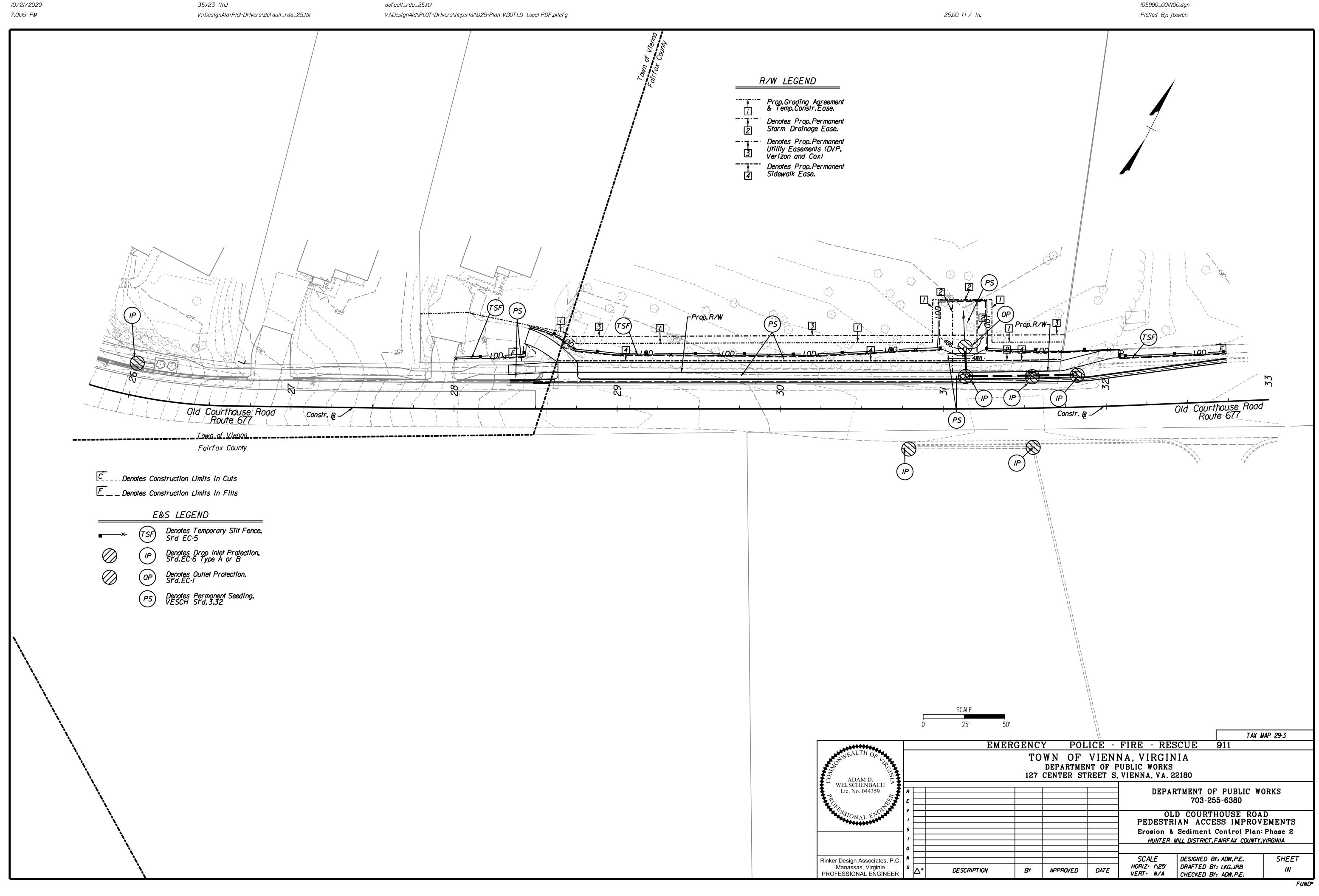
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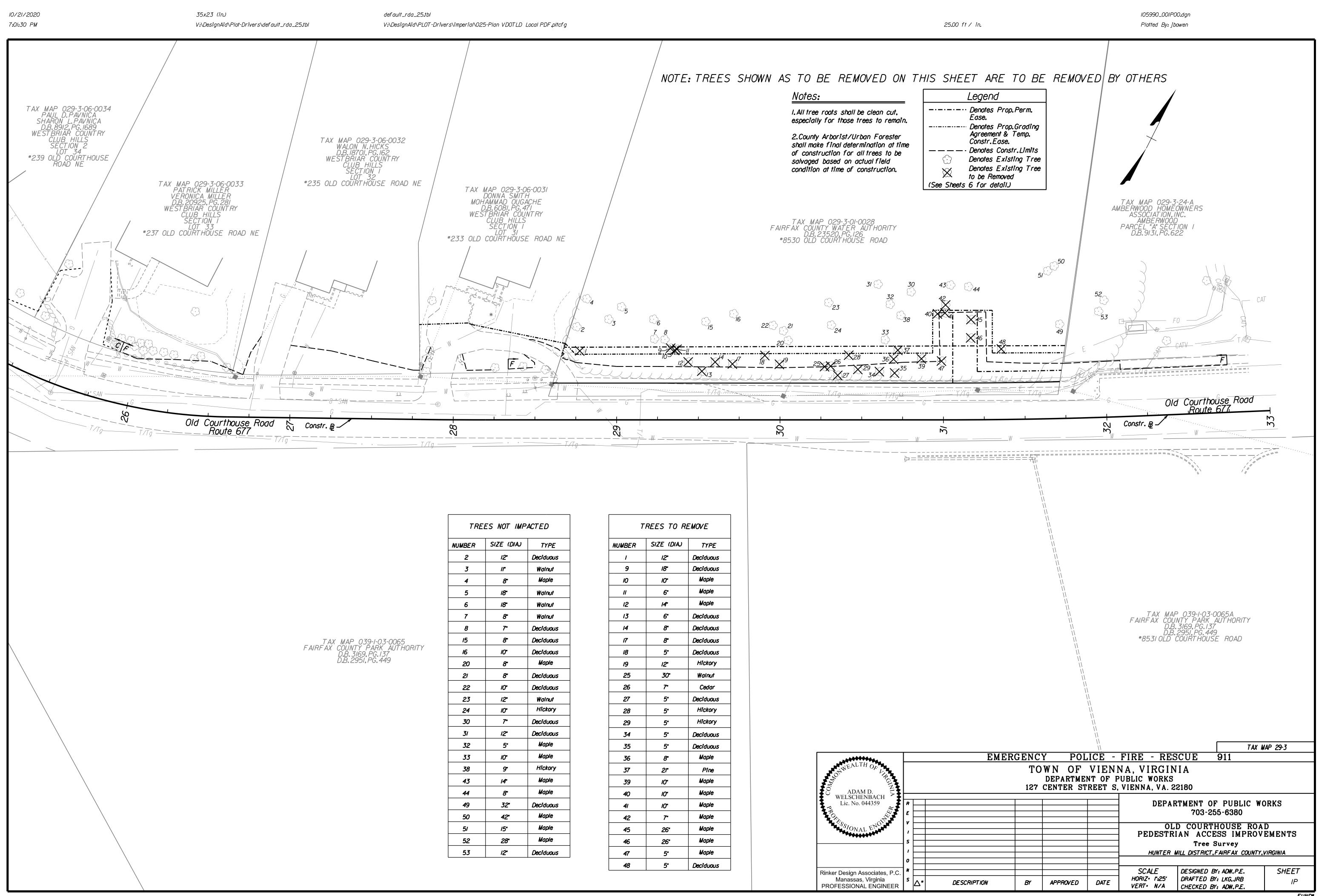
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CRIPTION	BY	APPROVED	DATE		RT= N/A		Br: LKG.JRB BY: ADW.P.E.		IL(2)

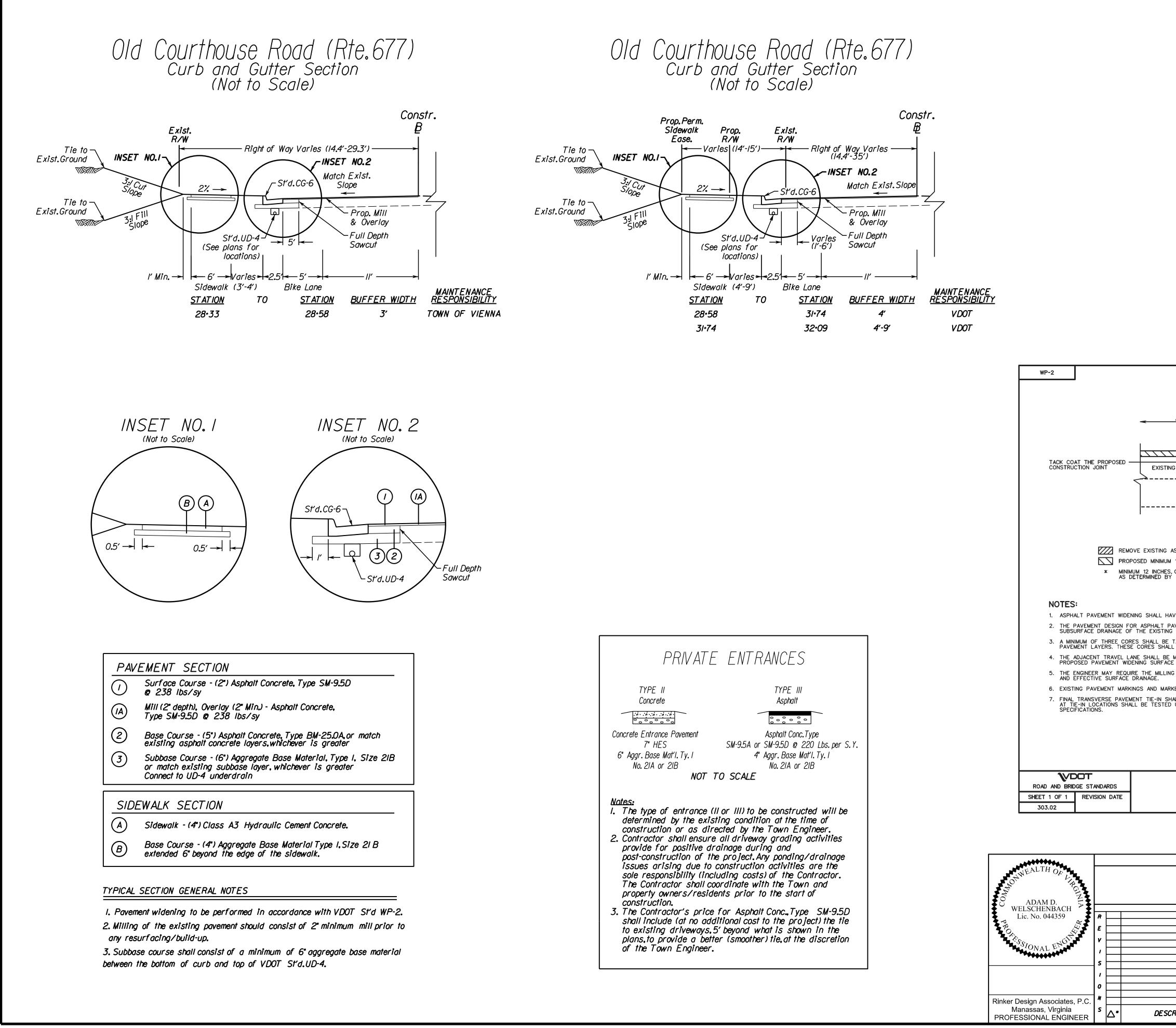






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JMBER	SIZE (DIA.)	TYPE
2	12"	Deciduous
3	<i></i>	Walnut
4	8"	Maple
5	<i>18</i> "	Walnut
6	<i>18</i> "	Walnut
7	8	Walnut
8	7"	Deciduous
15	8"	Deciduous
16	10 °	Deciduous
20	8	Maple
21	8"	Deciduous
22	10 °	Deciduous
23	12"	Walnut
24	10"	Hickory
30	7"	Deciduous
31	12"	Deciduous
32	5*	Maple
33	10"	Maple
38	9	Hickory
43	14"	Maple
44	8"	Maple
49	<i>32</i> "	Deciduous
50	42"	Maple
51	<i>15</i> "	Maple
52	28"	Maple
53	12"	Deciduous

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NUMBER	SIZE (DIA.)	ΤΥΡΕ
1	12"	Deciduous
9	I8"	Deciduous
10	10"	Maple
11	6"	Maple
12	14"	Maple
13	6"	Deciduous
14	8"	Deciduous
17	8"	Deciduous
18	5"	Deciduous
19	12"	Hickory
25	30"	Walnut
26	7"	Cedar
27	5"	Deciduous
28	5"	Hickory
<i>2</i> 9	5"	Hickory
34	5"	Deciduous
<i>3</i> 5	5"	Deciduous
36	8"	Maple
37	21"	Pine
39	10"	Maple
40	10"	Maple
41	10"	Maple
42	7*	Maple
4 5	26*	Maple
46	26*	Maple
47	5"	Maple
48	5"	Deciduous



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PROJEC	CT INFOR	RMATION						
PROJECT INFORMATION								
PROJECT NAME	Old Cou	irthouse Road	Pedes	strian A	ccess Improv	ement		
COUNTY PROJECT NUMBER	FFX IC	FFX 104325						
VDOT UPC NUMBER (IF APPLICABLE)	PE	PE – xxxxxx RW – xxxxxx CN – xxxxxx						
PROJECT LIMITS / LENGTH	North C	North County Line to Battery Park St / 0.40 ml.						
FUNDING SOURCE	Locality	Locality						
DATE OF FUNDING OBLIGATION	2016							
LATITUDE/ LONGITUDE	LAT	38° 55′ 0" N		LONG	77°14′38"W			
6TH ORDER HUC		081004 (PL 2	22 Dif	fîcult R	ในก)			
TYPE OF DEVELOPMENT: (SELECT ALL THAT APP	PLY)							
LINEAR DEVELOPMENT								
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STORMWATER MANAGEMENT TECHNICAL CRITERIA	USED:							
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1.) FOR ADDITIONAL DETAILS SEE THE LATEST REVISION OF DRAINAGE COMPUTATIONS ON SHEETS 2K(3) TO 2K(11d) AND EROSION AND SEDIMENT CONTROL PLAN SHEETS 1L-1N. 2.) THIS IS A COUNTY ADMINISTERED PROJECT AND THE STORM WATER POLLUTION PREVENTION PLANS (SWPPP) IS PREPARED BY FAIRFAX COUNTY. WHEN APPLICABLE, IT WILL BE INCLUDED WITH VPDES AND CONSTRUCTION PACKAGE.

NAME:		STORMWA	OTTER, CHI	NSPORT	ATION				
		CONSTRU	JCTION BRAN	NCH					
PHONE N EMAIL ADI		703–324 WAYNE.K	4–5111 OTTER@FAIR	FAXCOUN	NTY.GOV				
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PROJECT DATA SHEET

TABLE 1.

WATER QUALITY ANALYSIS PER VSMP TECHNICAL CRITERIA IIB / FFX CO STORMWATER MANAGEMENT ORDINANCE ARTICLE 4 **

RECEIVING WATERS	OUTFALL		TOTAL DISTURBED AREA		PRE	PRE DEVELOPMENT LAND USE		POST DEVELOPMENT LAND USE			PHOSPHORUS REMOVAL REQUIRED *	ON-SITE PHOSPHORUS REMOVAL PROVIDED •
	ID	LOCATION	(AC)	(SF)	FORESTED (AC)	TURF (AC)	IMPERVIOUS AREA (AC)	FORESTED (AC)	TURF (AC)	IMPERVIOUS AREA (AC)	(LB/YR)	(LB/YR)
Wolftrap Creek	1A, 3		0.22	9,583	0.09	0.05	0.08	0.00	0.10	0.12	0.14	N/A

NOTE:

* PHOSPHORUS REMOVAL TO BE PROVIDED BY THE PURCHASE OF OFFSITE NUTRIENT CREDITS. PLEASE SEE TABLE 2 BELOW FOR MORE INFORMATION. ** TABLE HEADING SHOULD BE REVISED IF TECHNICAL CRITERIA 5 IS USED FOR GRANDFATHERED PROJECTS.

TABLE 2.

OFFSITE COMPLIANCE FOR WATER QUALITY (NUTRIENT CREDITS)

NUTRIENT CREDIT BANK NAME	4TH ORDER HUC	NUTRIENT CREDIT TO BE ACQUIRED (LB/YR)	PURCHASE LETTER (MM/DD/YY) (3)
SEE TABLE BELOW	02070008	0.14	SEE TABLE BELOW

NOTE:

3. ADDITIONAL INFORMATION WILL BE DOCUMENTED IN THIS TABLE UPON PURCHASE OF NUTRIENT CREDITS. PLEASE SEE LEDGER BELOW FOR EVIDENCE OF NUTRIENT CREDIT AVAILABILITY (RESERVATION)

EVIDENCE OF NUTRIENT CREDIT RESERVATION

							3ank (DEQ Certifi		y & Balance R				
				т	his spread	lsheet app	lies ONLY to Projec	ts which dr	aw Nutrient	Credits from	n FCDOT's	Bulk Purc	hase.
	Τ					Dra	w Down Qu	antity f	or Indiv	ridual P	rojects		
Tracking #	Proj. #	Project Name	UPC # (If any)	Funding Source	Fund Number	Proejct Location 8 Digit HUC	Watershed Name	Phosphorous Removal Required (Ibs/yt)	Cost/Project	TP Transferred (LB)	TP Balance (LB)	TN Retired (LB)	TN Balan (LB)
2-1	2G40-088-012	Old Courthouse Road Pedestrian Improvements	N/A	C&I - Blke & Pedestrian Program	400-C40011	0207008	Middle Potomac – Catoctin (Difficult Run)	0.14	\$1,958.60	0.14	14.86	1.40	148.51
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Plotted By: localuser

	N/A	2/27/2017	3/6/2017	Pending	
nce	DEQ Permit #	Purchase Agreement Date	Date Requested	Date of Credit Transfer	Comments
	iits by Fa	irfax Cou			

SITE DEVELOPMENT AND INSPECTIONS DIVISION APPROVAL STAMP

	PROVED OF FAIRFAX OPMENT SERVICES ND SITE REVIEW DIVISIO	
BY DATE	1/2018	BY

FUND[®]

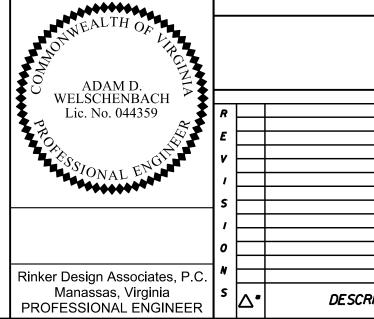
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EMER	GENC	Y POL	ICE -	FIRE - RES	CUE	911			
	F۵	IRFAX (COUN	LA VIRGIN	IA		13	9	
FAIRFAX COUNTY, VIRGINIA PUBLIC WORKS & ENVIRONMENTAL SERVICES - UTILITIES DESIGN & CONSTRUCTION DIVISION 12000 GOVERNMENT CENTER PARKWAY, SUITE 463 FAIRFAX, VA., 22035-0052									
				OFFIC		TAL FACILI	TIFS		
				OFFICE OF CAPITAL FACILITIES					
					100-024	4-2000			
				OLD COURTHOUSE ROAD					
				PEDESTRI	AN ACCE	SS IMPROV	EMENTS		
				P	ROJECT DA	TA SHEET			
				HUNTER MILL DISTRICT.FAIRFAX COUNTY.VIRGINIA					
				CONTRACT NO.CN	•	PROJECT	NO. FFX 104325		
				SCALE	DESIGNED BY	ADW.P.E.	SHEET		
DESCRIPTION	BY	APPROVED	DATE		DRAFTED BY	LKG.JRB	2J		

Stormwater Detention Exception Request Approval

County of Fairfax, Virginia To protect and enrich the quality of life for the people, neighborhoods and diverse communities of Fairfax County DEC 1 4 2018 Vanessa Aguayo, Project Manager Fairfax County Department of Transportation 4050 Legato Road, Suite 400 Fairfax, Virginia 22033-2895 Subject: Old Courthouse Road Pedestrian Access; 8833-FDOT-001-1; Tax Map No 029-3-((01))-0024A & 0028; Hunter Mill District Reference: Stormwater Detention Exception #8833-WSWD-001-1 Dear Ms. Aguayo: The referenced stormwater detention exception request has been received and reviewed for consistency with the Stormwater Management Ordinance (SWMO) Fairfax County Code section 124-6-1. Based on the justifications provided, the Director has determined that: i. The exception is the minimum necessary to afford relief; ii. Granting the exception will not confer any special privileges that are denied in other similar circumstances; iii. Exception requests are not based upon conditions or circumstances that are selfimposed or self-created; and iv. Reasonable and appropriate conditions shall be imposed as necessary upon any exception granted so that the intent of the Act and this Chapter are preserved. Therefore, your request to grant a partial exception of the stormwater detention requirement of the SWMO (124-4-4-D), is hereby approved on December 4, 2018, subject to the following condition: Detailed outfall analysis for the existing closed conduit system shall be provided to ensure capacity adequacy for the 10-year storm event. This exception approval in no way relieves you of any other County drainage requirements including adequacy of outfall and pro-rata share payments. Compliance with the SWMO, the Chesapeake Bay Preservation Ordinance, proffers and development conditions are also required. Department of Land Development Services 12055 Government Center Parkway, Suite 659 Fairfax, Virginia 22035-5503 Phone 703-324-1780 • TTY 711 • FAX 703-653-6678 1 www.fairfaxcounty.gov

Conditional Analysis:

·Detailed analysis has been provided on Sheet 2K(4).



Vanessa Aguayo, Project Manager 8833-WSWD-001-1 Page 2 of 2

This exception shall automatically expire, without notice, 24 months after the date of this letter, unless the subject plan has been approved.

Please ensure that a copy of this letter is made a part of the submitted plan.

If further assistance is desired, please contact Yosif Ibrahim, Senior Engineer III, Site Development and Inspections Division (SDID), at 703-324-1720 or yosif.ibrahim@fairfaxcounty.gov

Sincerely,

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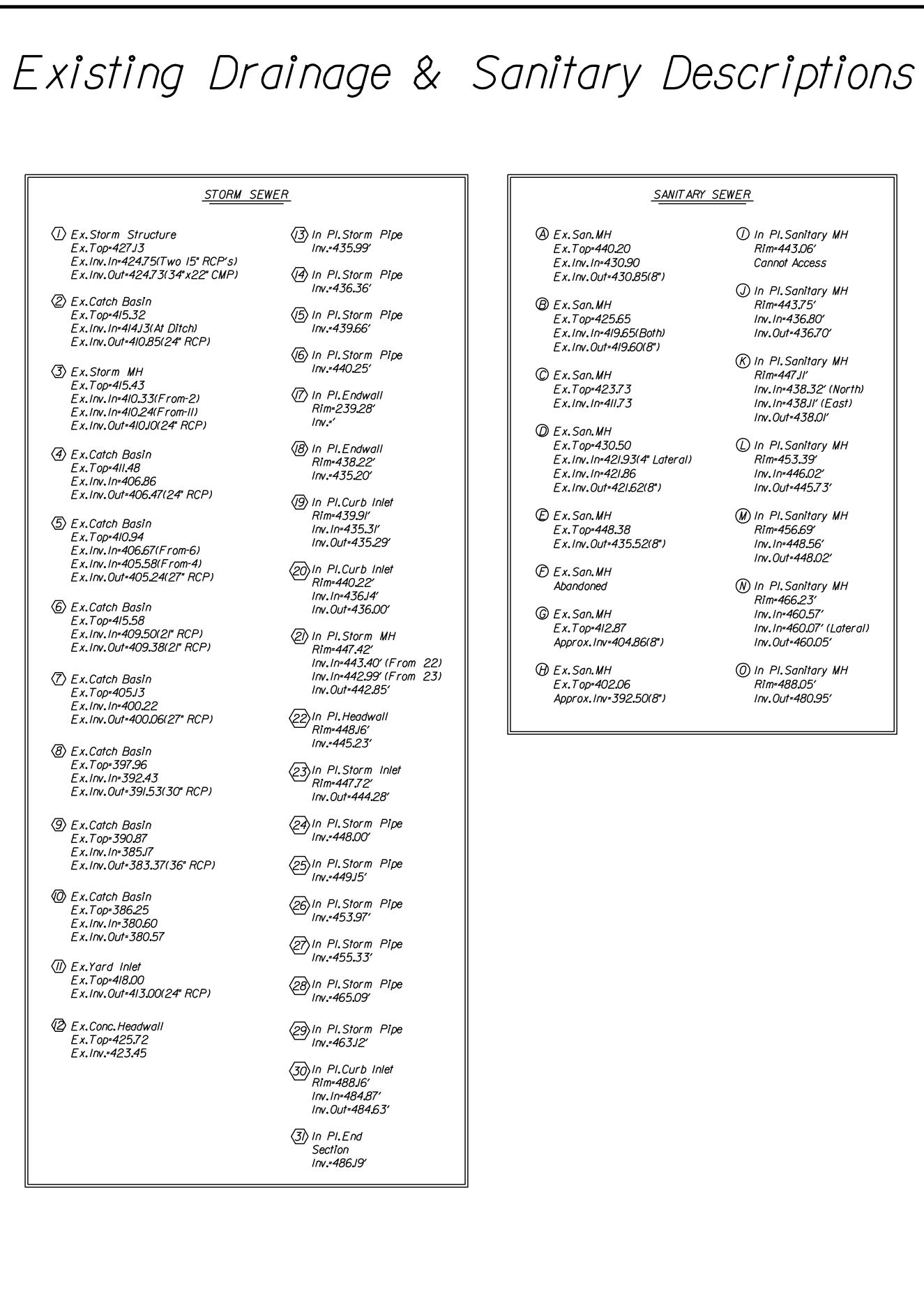
Spahabon Shahab Baig, P.E.

Chief, North Branch SDID Land Development Services (LDS)

cc: Shannon Curtis, Chief, Watershed Assessment Branch, Stormwater Planning Division, Department of Public Works and Environmental Services Yosif Ibrahim, Senior Engineer III, SDID, LDS Waiver File

						TAX M	AP 29-3
EMER	GENC	Y POL	ICE -	FIRE - RES	CUE	911	
		DEPARTMEI	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 23			
				DEPAR		' PUBLIC WC 5-6380	ORKS
				PEDESTRI SWM Detent	AN ACCE	HOUSE ROA SS IMPROV tion Request FARFAX COUNTY.	EMENTS Approval
						AINT AN COUNTIN	
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= 1:25' VERT= N/A	DESIGNED BA DRAFTED BY CHECKED BY	: LKG.JRB	SHEET 2J(I)

35x23 (in.) V:\DesignAid\Plot-Drivers\default_rda_25.tbl

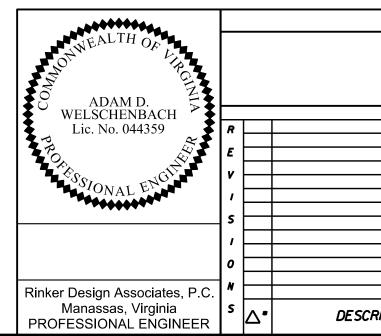


- ① In PI.Sanitary MH Rim=443.06′ Cannot Access
- (J) In Pl.Sanitary MH *Rim=443.*75′ Inv.In=436.80' Inv.Out=436.70'
- (K) In Pl.Sanitary MH Rim=447,11′ Inv. In=438.32' (North) Inv.In=438.II' (East) Inv.Out=438.01'
- () In Pl.Sanitary MH Rim=453.39′ Inv. In=446.02' Inv.Out=445.73'
- (M) In Pl.Sanitary MH Rim=456.69′ Inv.In=448.56' Inv.Out=448.02'
- (N) In Pl.Sanitary MH Rim=466.23' Inv.In=460.57' Inv.In=460.07' (Lateral) Inv.Out=460.05'
- (1) In Pl.Sanitary MH Rim=488.05′ Inv.Out=480.95'

Proposed	Draina
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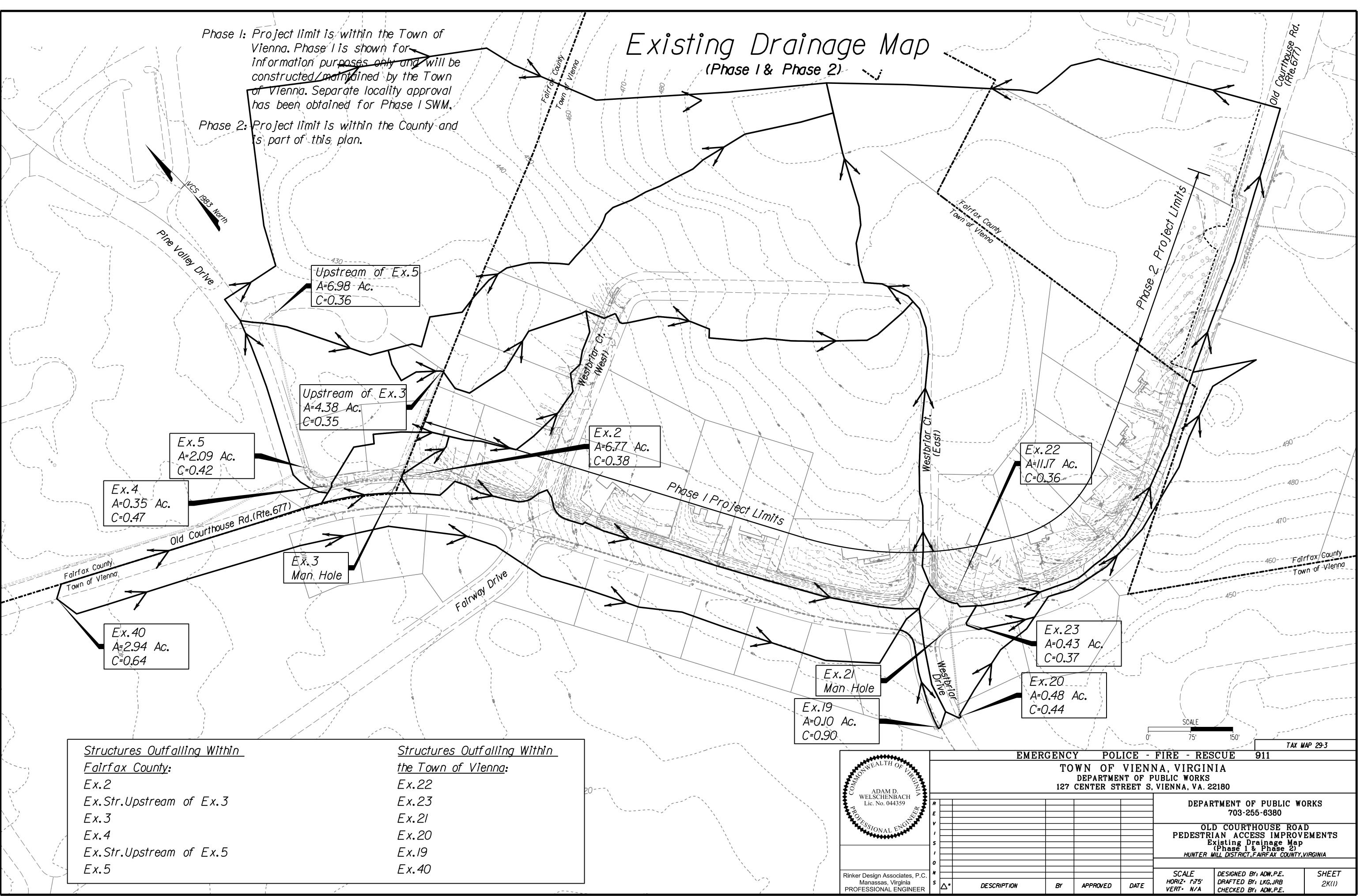
PHASE I Sheet 3 3-1 I St'd.DI-3BB Req'd. 4-2A I Filterra Tree E L=6' H=8.93' Inv.=415.08 Top=424.01 Inv.=442.59 Top (1) 3' x 3' Tree (St'd.IS-I Req'd. 1/2" Steel Plate Req'd. 6' - 4" SDR-35 | Connect UD-4 to DI See Details, 2L 215' - 15" Conc. Pi 3-1 to Ex.2 196' - 18" Conc. Pipe Req'd. (7' Cover) 4-2 to 4-1 (720' Radius with open joints (750' Radius wit - using 8' pipe joint lengths) - using 8' pipe Joints are to be opened a maximum Joints are to be of 25% of the spigot or tongue length. Inv(in) 415.08 Inv(out) 411.05 of 25% of the Inv.(in) 441.87 1 3-2 I St'd.DI-7 Reg'd. I St'd.DI-3B Req'd. 4-4 L=6' H=5.35' Inv.=421.16 Top=426.51 H-3.5' Inv.-442.8 St'd.IS-I Req'd. Connect UD-4 to DI 4-4 to 4-2 36' - 15" Conc. Pip Inv.(in) 442.85 3-2 to 3-1 38' - 15" Conc. Pipe Req'd. (4' Cover) I St'd.ES-IA 19"x. Inv.(in) 421.16 Inv.(out) 419.78 4-6 Inv.447.00 3-3 3.3 Lin.Ft.St'd.MH-I or 2 Req'd. 32' - 19"x30" Ellic I St'd.MH-I Frame & Cover Reg'd. 4-6 to 4-7 Inv.426.20 Top-430.20 Inv.(in) 447.00 St'd.IS-I Reg'd. 4-7 I St'd.DI-3C Reg L-8' H-3.6' Inv.-4 3-3 to 3-2 102' - 15" Conc. Pipe Reg'd. (3' Cover) Inv(in) 426.20 Inv(out) 422.76 I St'd. Monolithic Less Than Minim 3-4 I St'd.ES-I 15" Reg'd. See Detail Sheet Inv.429.50 St'd. IS-I Req'd. Connect UD-4 to 3-4 to 3-3 23' - 15" Conc. Pipe Req'd. (2' Cover) Inv.(in) 429.50 Inv.(out) 426.30 Existing Pipe To 3' - 34" x 22" Con 4-7 to Ex.21 3-5 Inv.(in) 445.30 I St'd.DI-3C Reg'd. L=6' H=4.0' Inv.=423.13 Top=427.13 Connect UD-4 to DI I St'd.DI-2B Req 4-8 L=10' H=4.0' Inv.= 3-5 to 3-2 St'd. IS-I Reg'd. 32' - 15" Conc. Pipe Reg'd. (2' Cover) Inv.(in) 423.13 Inv.(out) 422.76 Connect UD-4 to Connect 6" SDR-3 3-6 I St'd.DI-3B Reg'd. L=12' H=7.0' Inv.=422.68 Top=429.68 **4-8**A I Filterra Tree L St'd. IS-I Req'd. Inv.=451.37 Top= (2) 4' x 4' Tree (Connect UD-4 to DI 60' - 6" SDR-35 3-6 to 3-1 99' - 15" Conc. Pipe Reg'd. (5' Cover) See Details, 2L Se Inv.(in) 422.68 Inv.(out) 419.78 4-8 to 4-7 79' - 15" Conc. Pipe 3-7 I St'd.DI-3B Req'd. Inv.(in) 447.58 L=14' H=7.0' Inv.=430.08 Top=437.08 St'd. IS-I Rea'd. Sheet 5 Connect UD-4 to DI 5-1 I St'd.DI-2B Req L=10' H=4.0' Inv.= 3-7 to 3-6 161' - 15" Conc. Pipe Reg'd. (3' Cover) St'd. IS-I Reg'd. Inv.(in) 430.08 Inv.(out) 425.78 Connect UD-4 to Sheet 4 Connect 6" SDR-3 I St'd.DI-3B Req'd. 4-1 5-/A I Filterra Tree E L=8' H=4.0' Inv.=438.09 Top=442.09 Inv.=456.58 Top St'd.IS-I Req'd. (2) 4' x 4' Tree (Connect UD-4 to DI 33' - 6" SDR-35 See Details, 2L S 4-1 to 3-7 207' - 15" Conc. Pipe Req'd. (3' Cover) Inv(in) 438.09 Inv(out) 433.18 4-2 I St'd.DI-3B Req'd. L=6' H=4.0' Inv.=441.87 Top=445.87 St'd.IS-I Req'd. Connect UD-4 to DI Connect 4" SDR-35 PVC to DI

Phase I information shown for information only. All elements of Phase I are within the Town of Vienna, which maintains their own roadways.



ge Descriptions

Box (6' x 4') p - 446,13 Grate PVC Outfall Pip Series	be to Str.	.4-2	5-1 to 4-8	8	II9' - I5" Conc. (352' Radius - using 8' p Joints are to of 25% of t Inv,(in) 453,8	with open ipe joint lo be opened he spigot d) joints engths) 1 a maxin or tonque	านกา		
Pipe Req'd.(3' Co ith open joints e joint lengths) e opened a max	imum		5-2		I St'd. DI-2B L=I2' H=4.0' I Connect UD-4 Connect 6" SE	Req'd. Inv.=466.69 to DI	Тор-47	0.69	 	
spigot or tongu Inv.(out) 438.19 d.Type III Grate 85 Top=446.33	Req'd.		5-2A		I Filterra Tro Inv.=469.39 (2) 4' x 4' Tr I3' - 6" SDR-3	Top=472.9. ee Grate 35 PVC Ou	3	e to Str.	 5-2	
ipe Req'd.(3' Co Inv.(out) 441.97 x30" Req'd.	ver)		5-2 to 5	5-/	See Details,2 158' - 15" Cond (280' Radius - using 8' p Joints are to of 25% of t Inv,1in) 466,65	2 Series c.Pipe Req s with oper ipe joint h be opened he spigot d	'd.(3' Cove joints engths) a maxin or tongue	er) num		
liptical Conc. Pip Inv.(out) 445.40 q'd.)	(1.5' Cover) 「 						with	tructo Phas	e 2
Box Req'd. mum Height	48.92				PHAS	E 2		plan	here	in.
t 2K(8) DI To Be Extended Tonc.Pipe Reg'd.	(2' Cover	 	5-3		I St'd.DI-3B L=4' H=3.0' II I St'd.Monoliti Less Than M See Detail St Connect UD-4	nv.=485.30 hic Box Ru inimum He peet 2K(8)	Top=480 eq′d. eight	8.30		
Inv.(out) 443.40			Sheet 6 5-3 to 6	5-3	14' - 15" Çonc.	Pipe Rega	1.(2' Cover	·)		
-447.58 Top-4	151.58		6-1		Inv (in) 485.30 I St'd. DI-3C L=6' H=2.59'	0 Inv.lout Req'd.) 485.20			
35 PVC to DI Box (12' x 6') 5•454.91 Grate					I St'd. Monoliti Less Than M See Detail Sh Connect UD-4	hic Box Ru inimum He neet 2K(8)	ea'd.			
PVC Outfall P Series	ipe to St	r.4-8	6-1 to 5-3	3	38' - 15" Conc Inv.(in) 485.6() Inv.(out	d.(2' Cove) 485.40	r)		
ipe Req'd.(2' Co Inv.(out) 445.44			6-2		I St'd.DI-3B L=4' H=2.5' II I St'd.Monoliti Less Than M See Detail Sh Connect UD-4	nv.=485.85 hic Box Ru inimum He peet 2K(8)	eg'd.	8.35		
q'd. .=453.87 Top=4	157 . 87		6-2 to 6	5-/	24' - 15" Conc Inv.(in) 485,8	Pipe Req 5 Inv.(out	d.(I' Cover) 485.70	·)		
) DI 35 PVC to DI Box (12' x 6') p=460,12 Grate FVC Outfall P Series	tipe to Si	r.5-1	6-3		I St'd.ES-I 15 Inv.485.20 IJ7 C.Y.of EC Type A Insta	C-I Class A	l Req'd.			
		ا اـــــــــــــــــــــــــــــــــــ								
EMER	GENC	Y POL	ICE -	FIF	RE - RES	CUE	911	TAX M	AP 29-3	
	ТО		VIENN NT OF P	NA, UBL	VIRGIN SIC WORKS	IA				
					DEPAR	TMENT (703-2)F PUBI 255-638(RKS	
				EXI	PEDESTRI STING DRA PROPOSEI	INAGE &	ESS IM Sanita Age de	PROVI RY DI SCTRIP	EMEN' ESCRII TIONS	PTIONS
CRIPTION	BY	APPROVED	DATE		SCALE IORIZ= N/A IERT= N/A		BY: ADW.P. BY: LKG.JR BY: ADW.P.	B	SHE 2	ĸ
										FUND [®]



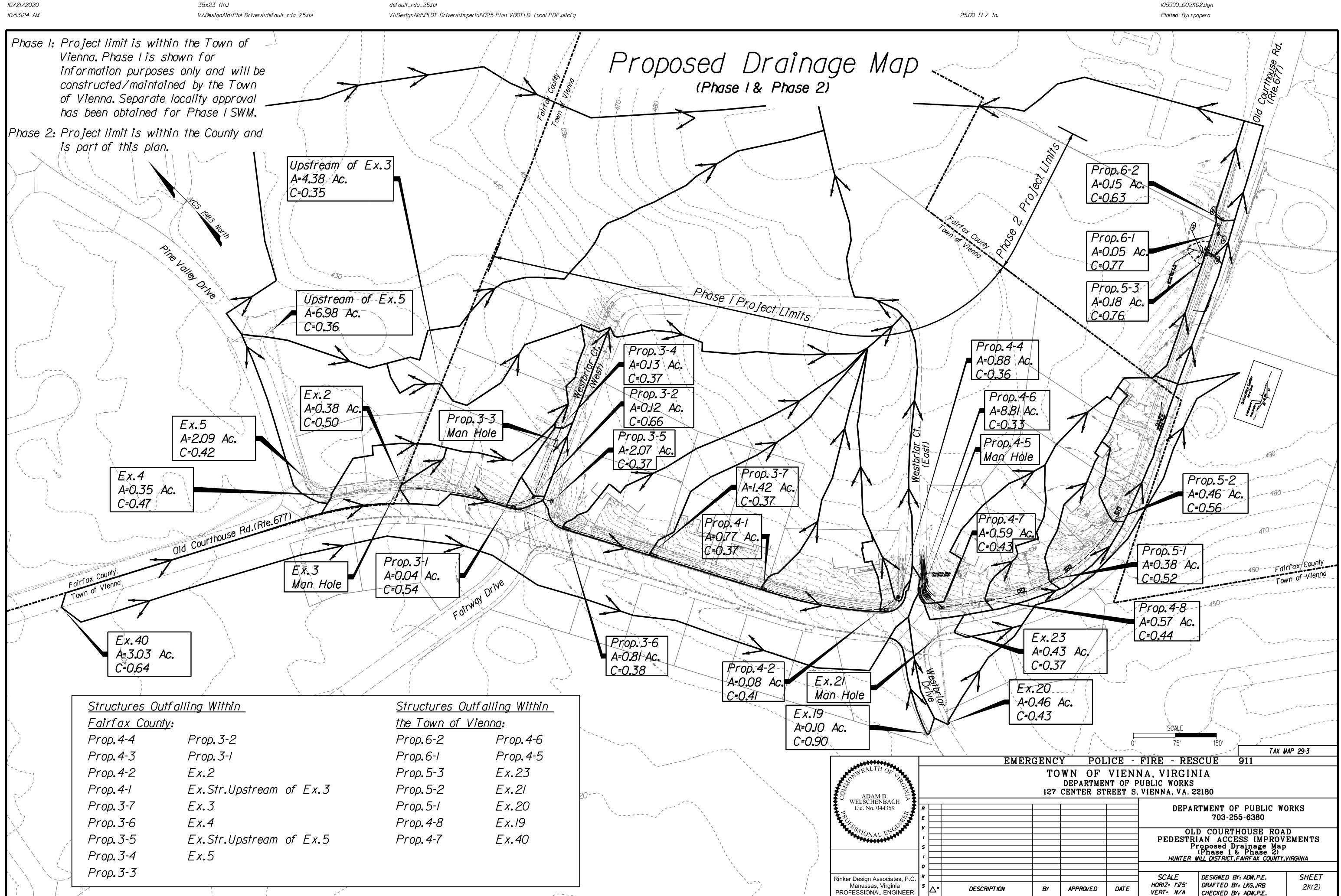
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35x23 (in.)

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10/21/2020

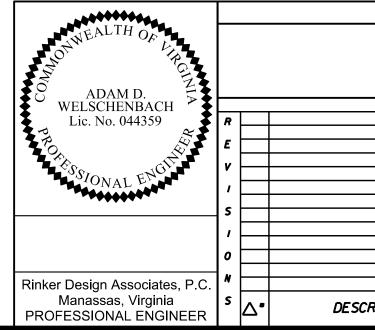
10:53:16 AM



Number	Type	Length (Ft)	Station	Draiange Area (Ac)	ن ن	CA	Sum CA	l (In/Hr)	a Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (Ft/Ft)	Sx Cross Slope (F1/F1)	T (Spread)(Ft)	W (Gutter Width) (Ft)	W/T	Sw (Gutter Slope) (F1/F1)	Sw/Sx	Ео (Арр.9С-8)	a	S'W	Se	Computed Length (Ft)	L. Specified Length (Ft)	רירד	E (App.9C-18)	0 Intercepted (CFS)	0b Carryover (CFS)	Depth at Curb (In)	Allowable Ponding Depth (Ft)	Height of Curb Opening h (Ft)	nlets u/p	Depth at Inlet kjuo (In)	T Spread & SAG (Ft)	Remarks	PRE-DEVELOPM INLET COMPUT FORM LD-204 INLET COMPUTATIONS ROUTE: Old Courthouse Road Pede	ΑΤΙΟ	
Ex2	YI-1	4	12•18	6.77	0.38	2.57																														DESIGNED BY: JZ	DATE:	5/06/2016
							2.57	4.00	10.29	0.00	10.29	0.0400	/		4.00									4		1.00	10.29	0.00	0.6						BHt .=0.3 56′	CHECKED BY: NVD	UNITS	ENGLISH

	Inlet																														S	ag Inle	ts Only			ך POST-D	EVE	ΞLO	PMEN	ТТ
Number	Туре	Length (Ft)	Station	Draiange Area (Ac)	J	СА	Sum CA	I (In/Hr)	a Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (Ft/Ft)	Sx Cross Slope (Ft/Ft)	T (Spread)(Ft)	W (Gutter Width) (Ft)	W/T	Sw (Gutter Slope) (F1/F1)	Sw/Sx	E0 (App.9C-B)	o	S'w	Se	Computed Lenath (Ft)	L. Specified	רערב	E (App.9C-18)		Ob Carryover (CFS)	Depth at Curb	Allowable Ponding Denth (Ft)	Height of Curb Opening h (F1)	2	Depth at Inlet (In)	T Spread @ SAG (Ft)	Remarks	INLET FORM LD-204 INLET COMPUT ROUTE: OID CO	ATION	NS		
3-1	DI-3BE	3 6	14.02.67	7 0.04	0.54	0.02																														DESIGNED BY:	JZ		DATE	9/05/2017
							0.02	4.00	0.09	0.00	0.09	0.0100	0.0208	1,16	2.00	1.7197	0.083	3 4.0048	2 1	3.5	0,1458	3 0.1666	5 1.978	86	3.03	33 1.00	0.0	9 0.00) 1.2	2						CHECKED BY:			UNITS	
3-2	DI-3B	6	53•45,1	<i>0,12</i>	0.66	0.08																																	•••••	
							0.08	4.00	0.32	0.00	0.32	0.0280	0.0520	1.58	1.50	0.95/2	20.083	3 1.6019		2.06	0,1146	0,1666	6 4.60	8 6	1.30	02 1.00	0 0.3	2 0.00	<u>) 1.5</u>	;						_				
3-5	DI-3C	6	53•47.5	4 1.95	0.37	0.72																						_		_		_		_	Back/Lt.	_				
					0.77		0.72	4.00	2.89	0.00		0.0280	0.0510	4.58	1.50	0.3278	80.083.	1.6333		2.08			_	_					_			_		_	Back/Lt.	4				
		-		0,12	0.37	0.04	0.04	4.00									-					-						_		_					Ahead/Rt.	-				
							0.04	4.00	0,18	0.00		0.0280									<u> </u>			6			3.0	<u> </u>			3 0.458	27 0 00	11 4.9		Ahead/Rt. Weir Flow	-				
3-6	DI-3B	12	15+11,20		0.38	031					5.06	0.0200	/											0			5.0	0	J.4	1 0.41	5 0.450	0.90	4.9	5.6/		-				
				0.07	0.50	0.51	0.3/	4.00	1.23	0.00	1.23	0.0720	0.0632	2.36	1.50	0.6.36	90.083	3 1.3180	0.947	1.86	0.10.34	4 0,1611	11.05	2 12	1.08	36 1.00	0 1.2.	3 0.00	2 2.	,						-				
							0.07							2.00															<u> </u>							-				
3-7	DI-3B	14	16•75	1.42	0.37	0.53																														1				
							0.53	4.00	2J0	0.00	2J0	0.0328	30.0200	6.56	1.50	0.228	70.083	3 4. 1650	0.645	2.64	0,1466	6 0,114 5	5 13.40)3 14	1.04	15 1.00	0 21	0.0) 2.7	7										
4-1	DI-3B	8	18•85.00	0.77	0.37	0.28																																		
							0.29	4.00	1,14	0.00	1,14	0.0116	0.0176	6.77	1.50	0.2214	40.083	3 4.7 3 30	0.649	2.68	0,149	0,1143	7.60	<u> 8</u>	1.05	52 1.00	0 1,14	1 0.00	2.6	5						_				
																													_			_				_				
4-2	DI-3B	6	21.08.13	0,11	0.4/	0.05	0.05	4.00				0.0050		100	150		0.087	7 1 0100	,	170	0.007									,		_				-				
							0.05	4.00	0,18	0.00	0.8	0.0256	6 0.0688	1.20	1.50	11/65	0.083	3 1.2108		1.76	0.09/8	8 0,1666	<u>אכי כן כ</u>	# 6	1.69	14 1.00	0 0.0	8 0.00) 1.3											
4-4	DI-7	2	02.33.5	4 0.88	0.36	032																		_		_			_		_									
					0.00	0.52	0.32	4.00	1.27	0.00	1.27	0.06/9	,		2.00	+	+				+	+		2		1.00	0 1.2	7 0.00	0.3	3		+			BHt.=0,139'	-				
Ex2	YI-1	4	12.18.20	0.38	0.5	0.19			· •⊆ /		·• <u>~</u>																			·						-				
							0.19	4	0.76	0	0.76	0.01			4		1							4		1	0.7	6 0	0.3	3					BHt.=0.062'	1				

Storm Computations for Outfalls in Fairfax County



					TAX I	IAP 29-3
EMER	GENC	Y POL	ICE -	FIRE - RES	CUE 911	
		DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 22		
	r				TMENT OF PUBLIC W	ADVS
				DEPAR	703-255-6380	URNS
					103-200-0000	
				OLI	D COURTHOUSE ROA	D
				PEDESTRI	AN ACCESS IMPROV	EMENTS
				Storm C	omputations for Out	falls in
				HUNTER M	omputations for Out Fairfax County IILL DISTRICT, FAIRFAX COUNTY,	VIRGINIA
				SCALE	DESIGNED BY: ADW.P.E.	SHEET
				HORIZ N/A	DRAFTED BY: LKG.JRB	2K(3)
RIPTION	BY	APPROVED	DATE	VERT N/A	CHECKED BY: ADW.P.E.	2/(3)

PRE-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

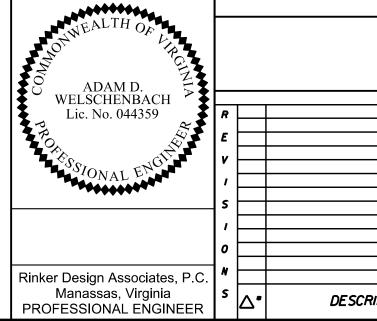
ST	RM SEWER DES	IGN COMF	UTATIO	NS																		DESIG	NED B	Y: <i>JZ</i>	,		DATE	: 12	2/13/2018
STO	RM FREQUENCY		10-Year]																		CHECK	KED BY	r: NV	D		UNIT	S: E	NGLISH
	. .	From F	Point	To Po	oint	Drain		С	A	Total	Rain		nof f	Invert E	levations	Length	Slope	Size	Shape	Number	Capacity	Friction		Norn	nal Flow			Flow	
	Pipe No.	Deferre	Cto	Deferrer	Cto	Area	Runoff Coeff.	Incre- ment	Accum- ulated	Inlet Time	Fall	Lateral		Upper End	Lower End	of Pipe		Dia.or Span/Ris		of Pipes			Depth of	Area of Flow, An	Hm	Vn	En	Time	Remarks
		Reference	Sta.	Reference	Sta.	(Acre)	"C"			(Minutes)	(In/Hr)	(CFS)	(CFS)			(Ft)	(Ft/Ft)		1	1 1000	(CFS)	(Ft/Ft)	(F†)	(SqFt)	(Ft)	(Ft/Sec.) (Ft)	(Sec)	
	Ex2toEx3	Ex2	12•18,20	Ex3	12+14,61	6.77	0.38	2.57	2.57	5.00	6.77	0.00	17.42	410.85	410.33	8	0.0650	24	Circular	1	57.68	0.0062	0.75	1.08	0.4/	<i>16,1</i>	4.77	0	
	Ex3toEx4	Ex3	12•14,61	Ex4	11.00.78	0.00		0.00	4,]/	16.25	4.45	0.00	24.34	410 . 10	406.86	120	0.0270	24	Circular	1	37 , 17	0.0120	1,18	1.93	0.55	12.6	3.65	10	
	Ex4toEx5	Ex4	11•00.78	Ex5	10•60 , 11	0.35	0.47	0.16	4.27	16.41	4.43	0.00	24.34	406.47	405.58	60	0.0/48	24	Circular	1	<i>2</i> 7 . 55	0.0120	1.46	2.46	0.60	9 . 9	2.98	6	
	Ex5toEx7	Ex5	10•60.11	Ex7		2.09	0.42	0.88	6.5/	16.51	4.42	0.00	33.58	405.24	400.22	209	0.0240	27	Circular	1	48.00	0.0122	1.39	2.57	0.63	13,1	4.04	16	

POST-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

STORM SEWER DESIGN COMPUTATIONS

STORM	SEWER DES	IGN COMF	UTATIO	NS																		DESIG	NED B	Y : <i>JZ</i>			DATE	<u> </u>	12/13/2
STORM	FREQUENCY		10-Year]																		CHECH	KED BY	Y: NV	D		UNIT	'S:	ENGLISI
		From I	Point	To Po	oint	Drain		С	Ά	Total	Rain	Rui	nof f	Invert E	levations	Length	Slope	Size	Shape	Number	Capacity	Friction			nal Flow			Flow	,
	Pipe No.	Reference	Sta.	Reference	Sta.	Area "A" (Acre)	Runoff Coeff. "C"	Incre- ment	Accum- ulated	Inlet Time (Minutes)		Lateral (CFS)	Total Q (CFS)	Upper End	Lower End	Length of Pipe (Ft)	(Ft/Ft)	Dia.or Span/Rise		of Pipes		Slope (Ft/Ft)	Depth of Flow.dn	Area of Flow, An (SqFt)	Hm (Ft)	Vn (Ft/Sec.	En (Ft)	Time (Sec)	
4	1-4to4-2	4-4	102+33.54	4-2	21.08.13	0.88	0.36	0.32	0.32	5.00	6.76	0.00	2,14	442.85	441.97	26.00	p.03385	/5	Circular	1	11.89	0.00110	0.36	0.29	0.21	7.34	1.20	3.54	
4	4-2to4-l	4-2	21.08.13	4-1	18•85.00	0.08	0.41	0.03	0.35	5.06	6.74	0.00	2.36	441,87	438J9	215.00	0.01712	15	Circular	1	8.4 5	0.00140	0.45	0.40	0.25	5.90	0.99	36.42	, ,
4	4-Ito3-7	4-1	18•85.00	3-7	16•75	0.77	0.37	0.29	0.63	5.67	6.55	0.00	4.23	438.09	433,18	207.00	0.02372	/5	Circular	/	9. 95	0.00450	0.57	0.54	0.29	7.78	1.51	26.62	1
	3-7to3-6	3-7	<i>l6•</i> 75	3-6	15•11.20	1.42	0.37	0.53	1,16	6,11	6.4/	0.00	7.59	430.08	425.78	161.00	0.02671	15	Circular	/	10.56	0.01440	0.79	0.81	0.36	9.36	<i>2J</i> 5	17.20	
	3-6to3-I	3-6	15•11.20	3-1	14•02.67	0.81	0.38	0.3/	1.47	6.40	6.33	0.00	9.54	422,68	419.78	105.00	0.02762	<i>I</i> 5	Circular	1	10.74	0.02280	0.92	0.97	0.38	9.89	2.44	10.62	
(11	3-ItoEx2	3-1	14•02.67	Ex2	12+18.20	0.04	0.55	0.02	2.38	6.57	6.28	0.00	14 . 97	415.08	411.05	190.00	0.02121	18	Circular	1	15.30	0.02120	1.20	1.52	0.46	9 . 87	2.71	19.26	
E	x2toEx3	Ex2	12•18.20	Ex3	12+14.61	0.38	0.50	0.19	2.57	6.90	6.20	0.00	<i>16</i> , <i>1</i> 5	410,85	410.33	8.00	0.06500	24	Circular	1	57.68	p.00530	0.72	1.03	0.40	15.75	4.57	0.5/	
E	x3toEx4	Ex3	12•14,61	Ex4	11.00.78	0.00		0.00	4,]	16.25	4.47	0.00	18 . 41	410,10	406.86	120.00	0.02700	24	Circular	1	37 ,]7	0.00690	0.99	1.56	0.50	11 . 80	<i>3.</i> /6	10,17	
E	x4toEx5	Ex4	11+00.78	Ex5	10•60.11	0.35	0.47	0,17	4.27	16.42	4.45	0.00	<i>19</i> , <i>1</i> 5	406.47	405.58	60.00	0.01483	24	Circular	1	27.55	0.00750	1.23	2.02	0.56	9.47	2.62	6.33	
E	x5toEx7	Ex5	10•60.11	Ex7		2.09	0.42	0.88	7.66	16.53	4.43	0.00	<i>33.</i> 97	405,24	400.22	209.00	p.02402	27	Circular	1	48.00	0.01250	1.40	2.60	0.64	13.09	4.06	15.97	
																								L'					
E E	3-4to3-3	3-4	54•76	3-3	54+51	013	0.37	0.05	0.05	5.00	6.77	0.00	0.33	429.50	426.30	23.00	0,13913	/5	Circular	1	24J0	0.00000	0.10	0.05	0.07	6.91	0.84	3.33	
3	3-3to3-2	3-3	54•51	3-2	53 • 45,11	0.00		0.00	0.05	5.05	6.75	0.00	0.33	426.20	422.76	102.00	p.03373	/5	Circular	1	11 . 86	0.00000	0.14	0.08	0.09	4.22	0.42	24 . 16	
	3-2to3-1	3-2	53•45 . //	3-1	14.02.67	0,12	0.66	0.08	0.89	5.46	6.62	0.00	5 . 91	421 , 16	419,78	48.00	0.02875	15	Circular	1	10.95	0.00870	0.65	0.65	0.32	9.10	1.94	5.28	
3	3-5to3-2	3-5	53•47.54	3-2	53·45,11	2.07	0.37	0.77	0.77	5.00	6.77	0.00	5,19	423,13	422.76	30.00	0.01233	15	Circular	1	7,17	0.00670	0.79	0.82	0.36	6.37	1.42	4.71	+

Storm Computations for Outfalls in Fairfax County



105990_002K04**.**dgn Plotted By:rpapera

12/13/2018 ISH

Remarks

						TAX MAP 29-3
EME	RGENC	Y POL	- ICE -	FIRE - RE	SCUE 911	
		DEPARTME	NT OF F	NA, VIRGI PUBLIC WORKS VIENNA, VA. 2		
				DEPA	RTMENT OF PUBLI 703-255-6380	C WORKS
				PEDESTR	D COURTHOUSE NAN ACCESS IMP Computations for Fairfax County MILL DISTRICT, FAIRFAX CO	ROVEMENTS
				SCALE	DESIGNED BY: ADW.P.E.	SHEET
RIPTION	BY	APPROVED	DATE	HORIZ• N/A VERT• N/A	DRAFTED BY: LKG, JRB CHECKED BY: ADW, P.E.	2K(4)
						FUND*

PRE-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

OKM LD	047																									
HYDRAUL	IC GRADE L	INE AN	ALYSIS																	DE	SIGNED	BY:	JZ	D	DATE: /2	2/13/2018
NCIDENCI	E PROBABIL	ITY [10-Year	• <u> </u>																CH	ECKED	BY:	NVD	L	JNITS: E	NGLISH
INLET		INVERT	DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	CTION L	055					Ad j.Ht	Inlet			Inlet	Top of MH	
OR	ST A.	EL.	OF FLOW	WATER	PIPE	DISCH.	PIPE	SLOPE,Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION	5174	OUTFLOW	OUTFLOW	SURF ACE	Do	Oo	Lo	(FT/FT)	Hf	Vo	Но	Vî	Vi*2/2g	0.35*MAX.	Angle	к	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(II)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
Ex7																								402.02		
Ex5	10•60 .]]	400.22	2.25	402.02	27	33.58	209	0.0123	2.568	13,1	0.662	9. 9	1.5	0.533	58 . 0	0.54	0.83	2.02	3.88	2.02	YES	1.01	3.58	406.63	410.44	0 . K .
Ex4	II+00 . 78	405.58	2.00	<i>407,18</i>	24	24.34	60	0.0121	0.726	9 . 9	0.380	12.6	2.5	0.865	41.0	0.43	1.06	2.30	0.73	2.30	YES	I . 15	1.88	409.06	410.98	<i>O.K.</i>
Ex3	12+14.61	406.86	2.00	409.06	24	24.34	120	0.0121	1.452	12.6	0.618	<i>I6</i> , <i>I</i>	4.0	1.405	75 . 0	0.62	2.50	4. 52	0.00	4.52	YES	2.26	3.71	412.77	4/5.43	0 . K.
Ex2	12•18.20	410.33	2.00	412.77	24	17.42	8	0.0062	0.050	16 . 1	1.004	0.0	0.0	0.000	0.0	0.00	0.00	1.00	17.42	1.30	YES	0.65	0.70	413.47	414,13	0.K.

POST-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

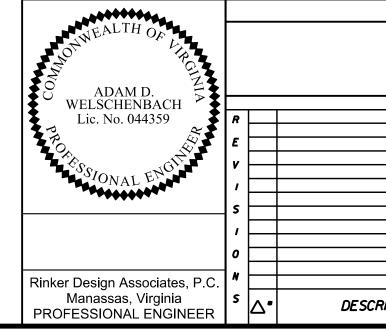
	IC GRADE L																				SIGNED					271372018 ENGLISH
	E PROBABIL		<u>10-Year</u> DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	CTION LO	055					Ad j.Ht	ECKED		NVD	Inlet	TOP OF MH	
INLET OR	ST A.	EL.	of flow	WATER	PIPE	DISCH.	PIPE	SLOPE,Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURFACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION		OUTFLOW	OUTFLOW	SURF ACE	Do	Oo	Lo	(FT/FT)	Hf	Vo	Но	Vi	Vi*2/2g	0.35*MAX.	Angle	K	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
3011011		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(II)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
Ex7																								402.02		
Ex5	10•60.11	400.22	2.25	402.02	27	33.971	209.00	0.01258	2.63	13.09	0.67	9 . 47	1.39	0.49	58.0	0.54	0.76	1.91	3.89	1.91	YES	0.95	3.58	406.64	410.44	0.K.
Ex4	<i>II+00.78</i>	405.58	2.00	407 . 18	24	19,146	60.00	0.00749	0.45	9 . 47	0.35	II . 80	2.16	0.76	41.0	0.43	0.93	2.03	0.73	2.03	YES	1.02	1.46	408.64	410.98	0.K.
Ex3	12+14.61	406.86	2.00	408.64	24	18.412	120.00	0.00692	0.83	II . 80	0.54	15.75	3.85	1.35	75.0	0.62	2.40	4.29	0.00	4.29	YES	2,14	2.97	411.62	415.43	0.K.
Ex2	12•18.20	410.33	2.00	411.93	24	<i>16,148</i>	8.00	0.00533	0.04	<i>15.75</i>	0.96	9 . 87	I . 5I	0.53	59 . 0	0.55	0.83	2.32	1,18	2.32	YES	I . 16	1.20	413,13	414,82	0 . K.
3-1	14•02.67	411.05	1.50	413,13	18	14 . 971	190.00	0.02/23	4.03	9 . 87	0.38	9 . 89	1.52	0.53	40.0	0.42	0.54	I .4 5	<i>0.</i>]4	I .4 5	YES	0.72	4.76	417,89	423 . 18	<i>O.K</i> .
3-6	15•11 . 20	419.78	1.25	420.78	15	9.543	105.00	0.02281	2.40	9 . 89	0.38	9.3 6	1.36	0.48	4.0	0.06	0.08	0.93	1.95	1.21	YES	0 . 61	3.00	423.78	429 . 18	О.К.
3-7	<i>l</i> 6•75	425.78	1.25	426.78	15	7.593	161.00	0.01444	2.33	9.36	0.34	7.78	0.94	0.33	0.0	0.00	0.00	0.67	3.37	0 . 87	YES	0.43	2.76	430.87	436.58	0.K.
4-1	18•85.00	433,18	1.25	434,18	/5	4.226	207.00	0.00447	0.93	7 . 78	0.23	5 . 90	0.54	0.19	0.0	0.00	0.00	0.42	I . 87	0.55	YES	0.28	1.20	438.66	441.59	0.K.
4-2	21:08,13	438.19	1.25	439,19	15	2.361	215.00	0.00140	0.30	5 . 90	0,14	7.34	0.84	0.29	36.0	0.39	0.32	0.75	0.22	0.75	YES	0.38	0.68	442.32	445 . 87	О.К.
4-4	102+33.54	441.97	1.25	442.97	15	2,140	26.00	0 . 00115	0.03	7.34	0.21	0.00	0.00	0.00	0.0	0.00	0.00	0.21	2,14	0 . 27	NO	0 . 27	0.30	443.27	446.3 5	О.К.
3-2	53•45 , //	419.78	1.25	420.78	15	5.913	48.00	0.00876	0.42	9,10	0.32	6.37	0.63	0.22	43.0	0.44	0.28	0.82	0.52	0.82	YES	0.41	0.83	421,81	426.16	О.К.
3-5	53•47.54	422.76	1.25	423.76	15	5,187	30.00	0.00674	0.20	6.37	0,16	0.00	0.00	0.00	0.0	0.00	0.00	0.16	5./9	0.20	NO	0.20	0.4/	424,17	426.63	О.К.
3-3	54•51	422.76	1.25	423.76	15	0.326	102.00	0.00003	0.00	4.22	0.07	6 . 91	0.74	0.26	0.0	0.00	0.00	0.33	0.00	0.33	YES	0.16	0,17	426.34	430.20	О.К.
3-4	54•76	426.30	1.25	427.30	15	0.326	23.00	0.00003	0.00	6.91	0.19	0.00	0.00	0.00	0.0	0.00	0.00	0.19	0.00	0,19	NO	0,19	0.19	429.60	430.75	О.К.

PROPOSED OUTFALL 1A DITCH COMPUTATIONS FORM LD-268

													Earth					
					С	A							n=.025 (USGS-CL)				Depth	
STA	A. TO STA.	FLOW	Area (Acres)	C-value	INCR.	ACC.	T _c	l l ₂	Q ₂	C or F	Slope Ft/Ft	ALLOW. VEL.	VEL.	I ₁₀	Q ₁₀	Depth	Available	
Old Co	urthouse Rd																	
Le	eft Side																	
31+12	31+12		0.40	0.70	0.28	0.28	5	5.23	1.46	C	0.0100	3.5	2.2	6.77	1.90	0.5	1.0	Design Ve

Storm Computations for Outfalls in Fairfax County

DESIGNED BY: AH CHECKED BY: SCT



105990_002K05**.**dgn Plotted By:rpapera



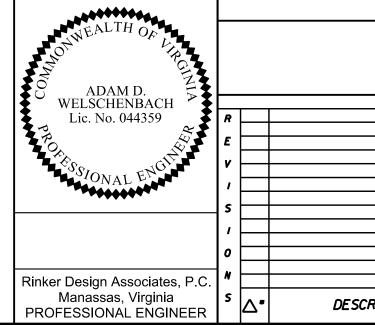
	DATE:	5/06/201	6				
-	UNITS	ENGLISH					
REM	ARKS						
			1				
Velocity <	Allowable	Velocity	1				
-		-					AP 29-3
EME	RGENCY				SCUE	911	
		DEPARTMEN	NT OF F	NA, VIRGIN Public works Vienna, va. 2			
				DEPAF		F PUBLIC WC 55-6380	ORKS
				PEDESTR	IAN ACCE	HOUSE ROA SS IMPROV Ons for Outf County FARFAX COUNTY.	EMENTS
RIPTION	ΒΥ	APPROVED	DATE	SCALE HORIZ• N/A VERT• N/A	DESIGNED B DRAFTED B CHECKED B	r: LKG.JRB	SHEET 2К(5)
							FUND

Storm Computations for Outfalls in the Town of Vienna

	INUTION		Type 94/1	Length (Ft)	 Station	Draiange Area	1407	J	СА	Sum CA	1/10/11/1	a Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow	S Gutter Slone	(FT/FT)	Sx Cross Slope (Ft/Ft)	T (Spread) (Ft)	W (Gutter Width) (F1)	W /T		SW (GUTTER STOPE) (Ft/Ft)	Sw/Sx	Eo (App.9C-8)	a	SʻW	Se	Computed Length (F1)	L. Specified Length (Ft)	רירד	E (App.9C-18)	0 Intercepted	(CFS)	ub carryover (CFS)	Depth at Curb (In)	Allowable Ponding Depth (Ft)	Height of Curb Opening h (Ft)	ag Ini	lets C	Depth at Inlet kjud (In)	T Spread @ SAG (Ft)	Remarks	PRE-DEN INLET C form ld-204 inlet computa route: Old Cour	0] 	MPU NS	JTAT	IO	NS s Improvemen	nts
Ex20 DI-3B 8 99-90 0.48 0.44 0.21 - - - - - - - - DESIGNATION - - - - - - - - - - - - - - - - - DESIGNATION - <	2	20 D	1-3B	8	 99•90	0.4	8 ().44	0.21			0.04	0.00		4 0 0		0000	4.61	150	0.7	05.00	0077	4,650				/70	7.017	0	114				0.00	0.0								DESIGNED BY:	JZ				570672016 ENGLISH	

	Inlet																														Sag In	ets Only] POST-DE	CVEL	OPME	ΝT	
Number	Type	Length (Ft) Station	Draiange Area (Ac)	J	СА	Sum CA	l (In/Hr)	a Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (F1/F1)	Sx Cross Slope (Ft/Ft)	T (Spread)(Ft)	W (Gutter Width) (F1)	W/T	Sw (Gutter Slope (F1/F1)	Sw/Sx	Eo (App.9C-8)	Ø	M,S	Se	Computed Length (F1)	L. Specified Lendth (Ft)	1/17	E (App.9C-18)		(CFS)	Ub Carryover (CFS) Depth at Curb	(U))	$d \subset 1$	Height of Curb Opening h (Ft)	Depth at Inlet	(u)	T Spread @ SAG (F1)	Remarks	INLET C FORM LD-204 INLET COMPUTA ROUTE: Old Cou	TIONS			
4-7	DI-3C	8 22.22.97	7 0,14	0.43	0.06																													Bac	ck/Lt.	DESIGNED BY:	JZ	DATI	2: :	5/06/2016
		-19 . 850′L	-			0.06	4.00	0.24	0.00																										ck/Lt.	CHECKED BY:	NVD	UNIT		ENGLISH
			0.45	0.43	0,19																													Ahe	ad/Rt.			0		211021311
						0.19	4.00	0.78	0.00			0.0217	3.90	1.50	0.3842	0.0833	3.84		2.61																ad/Rt.					
										1.02	0.0252												8			1.0	02	/	1.4	0.21	0.02 12.	00 2.	9	5.50 Weil	r Flow					
4-8	DI-2B	10 23.04		0.44	0.25																																			
		-19 . 500′L				0.25	4.00	1.00	0.00	1.00	0.0100	0.0208	5./6	2.00	0.3875	0.0833	4.00	0.88	3.50	0.1458	3 0,1484	4 5.89	9 2	1.7	0.1 0	0 1.0	00	0.00 2	2 .8							4				
																					_															-				
5-/	DI-2B			0.52	0.20								. ==		<u> </u>																					_				
		- <i>19.850′L</i>				0.20	4.00	0.79	0.00	0.79	0.0100	0.0208	4.3/	2.00	0.4575	0.0833	4.00	0.93	3.50	0,1458	<u> 0,1559</u>	9 5.18	2	1.9	5 1.0	0 0.	.79	0.00 2	2.6							-				
5-2	DI-2B			0.56	0.26		100	107		107		0.0000	5.05	0.00	0.7007	0.0077	100	0.07	7.50								07									4				
5 7	0/ 70	-19,850'L	-	0.45		0.26	4.00	1.03	0.00	1.03	0.0100	0.0208	5.25	2.00	0.3807	1.0833	4,00	0.87	3.50	0,1458	8 01475	5.98	3 2	2.0	1.0		03	0.00 2	2.8							-				
5-5	DI-3B	4 51*12 -20.330'i	0.01																						_											-				
		-20.5501		0.30							+ +													_	_											-				
			0.05	0.50	0.0/	0.14	400	0.56	0.00	0.56	0.0060	0.0442	303	200	06607	20833	188	0.07	201	0122	4 0.1629	2 374	1 1	1.0	7 10		56	0.00 2	25							-				
6-1	DI-3C	6 31+54.67	0.02	0.90	0.02		7.00	0.00				0.0772	5.05	2.00			1.00	0.91	L.JT			<u> </u>		1.0										Rac	ck/Lt.	4				
	2. 30	-20.330		0.30	0.00						+ +																								ck/Lt.	-				
					0.00	0.02	4.00	0.08	0.00		0.0044	0.0486	1.3.3	2.00	1.5004	2.0833	1.71		2.83																ck/Lt.	1				
			0.02	0.90	0.02																														ad/Rt.	1				
						0.02	4.00	0.07	0.00	1											1														ad/Rt.	1				
											0.0044												6			0.	.16).4	0.46	0.04 12.	00 2.	4			1				
6-2	DI-3B	4 31+81.68	0./5	0.63	0.09]				
		-20.3301				0.0	4.00	0.38	0.00	0.38	0.0112	0.0398	1.97	2.00	1.0157	0.0833	2.09	1.00	3.04	0,1268	8 0,1666	5 3.78	3 4	1.0	5 1.0	0 0.	.38	0.00 2	2.0											
Ex20	DI-3B	8 99•89.57	0.46	0.43	0.20																																			
						0.20	4.00	0.79	0.00	0.79	0.0200	0.0200	4.44	1.50	0.3378	0.0833	4.1650	0.827	2.64	0.1466	5 0,1413	6.764	4 8	1,18	3 1.0	0 0.	.79	0.00 2	2.2											

Note: All elements herein are part of Phase I plans and shown for information only. Separate locality approval has been obtained by Town of Vienna for SWM within Town of Vienna.



					ΤΑλ	MAP 29-3
EMER	GENC	Y POL	ICE -	FIRE - RES	CUE 911	
		DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 23		
				DEPAR	TMENT OF PUBLIC 703-255-6380	WORKS
				PEDESTRI Storm Co T	D COURTHOUSE RO AN ACCESS IMPRO omputations for Ou he Town of Vienna <i>ILL DISTRICT.FAIRFAX COUNT</i>	VEMENTS tfalls in
RIPTION	ΒΥ	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(6)

PRE-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

STORM SEWER DE	SIGN COM	PUTATIO	NS]	DESIG	NED B	Y : <i>JZ</i>	7		DATE	: 5,	/06/2016
STORM FREQUENC	Y [10-Year]																	(CHECK	KED BY	Y: NV	'D		UNIT	S: E	NGLISH
	From	Point	To P	oint	Drain		C	4	Total	Rain	Run	off	Invert E	levations	Length	Slope	Size	Shape	Number	Capacity	Friction	,	Nori	mal Flow	•		Flow	
Pipe No.					Area	Runoff Coeff.	Incre- ment	Accum- ulated	Inlet	Fall	Lateral	Total Q	Upper End	Lower End	of Pipe	.	Dia.or Span/Rise		of Pipes		Slope	Depth of	Area of		Vn	En	Time	Remarks
	Reference	Sta.	Reference	Sta.	(Acre)	"C"	/// C ///		(Minutes)(In/Hr)	(CFS)	(CFS)	E 110		(Ft)	(Ft/Ft)			ripes	(CFS)	(Ft/Ft)	(Ft)	Flow, An (SqFt)	(Ft)	(Ft/Sec) (Ft)	(Sec)	
Ex22toEx2l	Ex22	22+22.97	Ex21	22.10.51	<i>II.</i>]7	0.36	4.02	4.02	15.30	4.58	0.00	18 . 41	445.23	443.40	47	0.0389	34 X 22	Elliptical	1	62.26	0.0034	0.69	1.40	0.45	13.3	3.44	4	
Ex2ltoEx20	Ex21	22+10.51	Ex20	99•89.57	0.00		0.00	4 ,18	15.36	4.57	0.00	<i>19.48</i>	<i>442.8</i> 5	436,14	/58	0.0425	21	Circular	/	32.65	0.0157	0.97	1.38	0.47	14,2	4.09	//	
Ex20toEx19	Ex20	99•89.57	Exl9	99•90.18	0.48	0.44	0.21	4.39	/5.55	4.54	0.00	19 . 96	436.00	435.31	40	0.0173	24	Circular	/	29.71	0.008/	1.20	I . 97	0.56	10,1	2.80	4	
Exl9toExl8	Exl9	99•90.18	Exl8	99•90.37	010	0.90	0.09	4.48	15.61	4.54	0.00	20.32	<i>435.29</i>	435.20	8	0.0113	24	Circular		24.00	0.0084	1.41	2.37	0.59	8.6	2.55	1	

POST-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

STORM	SEWER DES	IGN COMP	PUTATIO	NS																		DESIG	NED B	Y : <i>JZ</i>	,		DATE]: (9/29/2
STORM	FREQUENCY		10-Year]																		CHECK	ED BY	r: NV	D		UNIT	' S: 1	ENGLISI
		From	Point	To Po	nint	Drain		C	A	Total	Rain	Rui	nof f	Invert E	levations	Length	Slope	Size	Shape	Number	Capacity	Friction		Norn	nal Flow			Flow	
	Pipe No.	Reference	Sta.	Reference	Sta.		Runoff Coeff. "C"	Incre- ment	Accum- ulated	Inlet Time (Minutes)	Fall	Lateral (CFS)	Total Q (CFS)	Upper End	Lower End	of Pipe (Ft)		Dia.or Span/Rise		of Pipes		Slope (Ft/Ft)	Depth of Flow.dn	Area of Flow, An (SqFt)	Hm (Ft)	Vn (Ft/Sec.	En (Ft)	Time (Sec)	
e	6-2106-1	6-2	31.81.68	6-1	31•54.67	0.15	0.63	010	0.0	5.00	6.76	0.00	0.64	485.95	485.70	24	0.0104	/5	Circular	1	6.59	0.0001	0.26	0,19	0,16	3.4	0.44	7	
E	6- <i>lto</i> 5-3	6-1	31.54.67	5-3	31+12	0.05	0.78	0.04	0.13	5,12	6.72	0.00	0.90	485.60	485.40	38	0.0053	15	Circular	1	4.69	0.0002	0.37	0.31	0.21	3.0	0.5/	13	
5	5-3to6-3	5-3	31+12	6-3	31+12	0.18	0.78	0.14	0.27	5.33	5.09	0.00	1.62	485.30	485.20	4	0.0071	/5	Circular	1	5.46	0.0007	0.47	0.42	0.25	3. 9	0.70	4	
																											!		
<u> </u>	5-2to5-1	5-2	25.99.96	5-1	24.30.32	0.46	0.56	0.26	0.26	5.00	6.77	0.00	1.75	466.69	453. 97	158	0.0805	15	Circular	1	18.33	0.0008	0.26	0,19	0.16	9.4	1.64	17	
<u> </u>	5- <i>lto</i> 4-8	5-1	24.30.32	4-8	23•04	0.38	0.52	0.20	0.46	5.28	6.68	0.00	3.07	453 . 87	447.68	119	0.0520	15	Circular	1	14.73	0.0024	0.39	0.32	0.22	9. 5	1.78	13	
4	4-8to4-7	4-8	23.04	4-7	22•22.97	0.57	0.44	0.25	0.71	5.49	6.61	0.00	4.73	447.58	445.44	79	0.0271	15	Circular	1	10.63	0.0056	0.58	0.56	0.30	8.4	1.68	9	
4	4-6to4-7	4-6	22•00	4-7	22•22.97	8.81	0.33	2.89	2.89	16.90	4.37	0.00	12.62	445.60	445.40	28	0.0071	24	Circular	1	19,12	0.0032	1.19	1.94	0.55	6.5	1.84	4	
4	-7toEx2I	4-7	22•22.97	Ex21	22+10.51	0.59	0.43	0.25	3. 85	16.97	4.36	0.00	21.48	445.30	443.40	50	0.0380	34 X 22	Elliptical	1	61.51	0.0047	0.76	1.59	0.49	13.8	3.71	4	**
Ex	21toEx20	Ex2I	22•10.51	Ex20	<i>99•89.</i> 57	0.00		0.00	4.01	17.03	4.36	0.00	22 , 17	442.85	436,14	158	0.0425	21	Circular	1	32.65	0.0204	1.06	1.52	0.49	14.6	4.36		
Ex	20toEx19	Ex20	99•89.57	Exl9	99•90.18	0.46	0.43	0.20	4.20	17.23	4.33	0.00	18.37	436.00	435.31	40	0.0173	24	Circular	1	29.71	0.0069	1,14	I . 85	0.54	10.0	2.68	4	
Ex	xl9toExl8	Ex19	99•90.18	ExI8	99•90.37	OLO	0.90	0.09	4.29	17.30	4.33	0.00	18.75	435.29	435.20	8	0.0113	24	Circular	1	24.00	0.0072	1.33	2,22	0.58	8.4	2.44	/	

CURB & GUTTER OUTFALL SPREAD COMPUTATIONS

Outfall	Outfall Location (Station)	Drainage Area (ac)	Cw	Peak Flow,Q (cfs)	Channel Slope (ft/ft)	Gutter Cross Slope (ft/ft)	Road Cross Slope (ft/ft)	Spread* (See Note I) (ft/ft)	Deµ at C (ft/
Outfall #2	Old Courtho	use Road							
Pre-Developed	5•75 RT	2.94	0.637	12.70	0.0270	0.0833	0.0200	14.76	4.
Post-Developed	5•75 RT	3.03	0.645	13,22	0.0270	0.0833	0.0200	14.99	4.

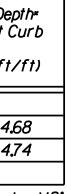
* Values derived using Bentley FlowMaster V8i.

Note I: Allowable Spread at Outfall *2 = I/2 Driving Lane • On-Street Parking Width • Gutter Width = 8' • 6' • I.5' = <u>I5.5'</u>

25.00 ft / in.

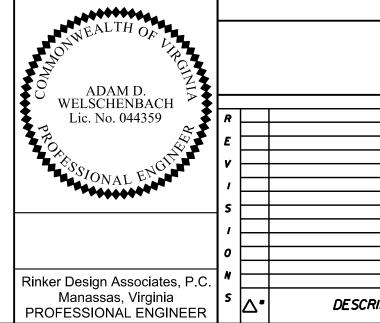
Storm Computations for Outfalls in the Town of Vienna

Note: All elements herein are part of Phase I plans and are shown for information only. Separate locality approval has been obtained by Town of Vienna for SWM within Town of Vienna.



DESIGNED BY: JZ CHECKED BY: NVD

DATE: 5/06/2016 UNITS: ENGLISH



9/29/2016

Remarks

** Note:

4-7 to Ex.21 is an existing pipe being extended.Velocity of existing pipe Ex. 22 to Ex.21 greater than 10 fps in pre-development conditions.

						X MAP 29-3
EME	RGENC	<u>Y POL</u>	<u>ICE -</u>	<u>FIRE - RES</u>	<u>SCUE 911</u>	
		DEPARTME	NT OF F	NA, VIRGIN Public works vienna, va. 2		
				DEPA	RTMENT OF PUBLIC 703-255-6380	WORKS
				PEDESTR Storm Q	D COURTHOUSE R IAN ACCESS IMPR Computations for Ou The Town of Vienna MILL DISTRICT, FAIRFAX COUN	OVEMENTS utfalls in
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(7)
						FUN

Storm Computations for Outfalls in the Town of Vienna

PRE-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

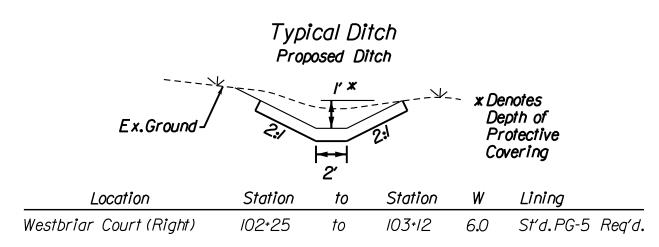
	IC GRADE L	_		_																	SIGNED		JZ			/06/2016
INCIDENC	E PROBABIL	ITY L	<u> 10-Year</u>	,																CH	ECKED	BY:	NVD	U	INITS: E	NGLISH
INLET		INVERT	DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	ICTION L	OSS					Ad j.Ht	Inlet			Inlet	Top of MH	
OR	STA.	EL.	OF FLOW	WATER	PIPE	DISCH.	PIPE	SLOPE.Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION		OUTFLOW	OUTFLOW	SURF ACE	Do	Qo	LO	(FT/FT)	Hf	Vo	Но	Vi	Vi*2/2g	0.35*MAX.	Angle	κ	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
JUNCTION		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
Exl8																								436.80		
Ex/9	99+90.18	435.20	2.00	436.80	24	20.32	8	0.0084	0.067	8.6	0 . 285	IO , I	1.6	0.559	3.0	0.05	0.07	0.92	0.41	0.92	YES	0.46	0.53	437.33	439.41	<i>O.K.</i>
Ex20	99 • 89 . 57	435.31	2.00	437.33	24	19 . 96	40	0.008/	0.325	IO , I	0.399	14.2	31	1.091	69.0	0.60	I . 87	3.36	0.96	3.36	YES	1.68	2.00	439.33	439.72	<i>O.K.</i>
Ex21	22+10.51	436,14	1.75	439.33	21	19 . 48	158	0.0/58	2.497	14.2	0.780	13.3	2.7	0.961	12.0	0,15	0.42	2 . /6	0.00	2 . 16	YES	1.08	3.57	443.82	447.42	<i>O.K.</i>
Ex22	22•22.97	443.40	1.83	444.87 3	4 X 2.	2 18.41	47	0.0034	0.160	13.3	0.686	0.0	0.0	0.000	0.0	0.00	0.00	0.69	0.00	0.69	NO	0.69	0.85	445.92	448. I6	0.K.

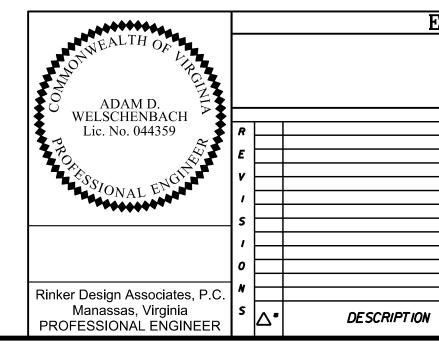
POST-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

	IC GRADE	_	ALYSIS																	DE	SIGNED	BY:	JZ	D	DATE: 9	9/29/2016
NCIDENC	E PROBABII	LITY [<u>10-Year</u>	•																CH	ECKED	BY:	NVD	U	JNITS: E	ENGLISH
INLET		INVERT	DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION			_	JUN	NCTION L	OSS	_				Ad j. Ht	Inlet			Inlet	Top of MH	
OR	ST A.	EL.	OF FLOW	WATER	PIPE	DISCH.	PIPE	SLOPE, Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION		OUTFLOW	OUTFLOW	SURF ACE	Do	00	LO	(FT/FT)	Hf	Vo	Но	Vi	Vi∗2/2g	0.35*MAX.	Angle	к	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
JUNCTION		PIPE	PIPE	ELEV.	(In/mm)	CFS/CMS	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
6-3																								486.20		
5-3	31+12	485.20	1.25	486.20	15	1,62	14	0.0007	0.009	3.88	0.06	2.95	0.14	0.05	90.0	0.70	0.09	0.20	0.71	0.26	YES	0.13	0.14	486.34	488.30	<i>O.K</i> .
6-1	31•54.67	485.40	1.25	486.40	/5	0.90	38	0.0002	0.008	2.95	0.03	3.41	0,18	0.06	0.0	0.00	0.00	010	0.26	0.13	YES	0.06	0.07	486.47	487,69	<i>O.K.</i>
6-2	31•81.68	485.70	1.25	486.70	15	0.64	24	0.0001	0.002	3.41	0.05	0.00	0.00	0.00	0.0	0.00	0.00	0.05	0.64	0.06	YES	0.03	0.03	486.73	487.85	О.К.
Exl8																								436.80		
Exl9	99•90.18	435.20	2.00	436.80	24	18.75	8	0.0072	0.057	8.45	0.28	9.95	1.54	0.54	3.0	0.05	0.07	0.89	0.39	0.89	YES	0.44	0.50	437.30	439.41	<i>O.K.</i>
Ex20	99•89.5 7	435.31	2.00	437.30	24	18.37	40	0.0069	0.276	9. 95	0.38	13.81	2.96	1.04	69.0	0.60	1.77	3,19	0.86	3,19	YES	1.60	1.87	439,17	439.72	<i>O.K.</i>
Ex21	22+10.51	436,14	1.75	439,17	21	17.51	158	0.0128	2.016	13.81	0.74	12.84	2.56	0.90	12.0	0,15	0.39	2.02	0.00	2.02	YES	1.01	3.03	443.76	447.42	О.К.
4-7	22+22.97	443.40	1.83	444.87.	34 X 2	2 16.81	50	0.0028	0,142	12.84	0.64	6.50	0.66	0.23	75.0	0.62	0.68	1.55	,	1.55	YES	0.78	0.92	445.96	448.92	О.К.
4-8	23•04	445.44	1.25	446.44	/5	4.73	79	0.0056	0.442	8.41	0.27	9.48	1.40	0.49	0.0	0.00	0.00	0.76	1.66	0.99	YES	0.50	0.94	448 . /6	451.58	<i>O.K</i> .
5-1	24.30.32	447,68	1.25	448.68	/5	3.07	119	0.0024	0.281	9 . 48	0.35	9.42	1.38	0.48	0.0	0.00	0.00	0.83	1.32	1.08	YES	0.54	0.82	454.26	457,87	<i>O.K.</i>
5-2	25•99.96	453.97	1.25	454.97	15	1.75	158	0.0008	0,121	9.42	0.34	0.00	0.00	0.00	0.0	0.00	0.00	0.34	1.75	0.45	NO	0.45	0.57	466.95	470.69	<i>O.K.</i>
Ex23	22•40.73	442.99	1.50	444,19	18	1,08	31	0.000/	0.003	6.33	0.16	0.00	0.00	0.00	0.0	0.00	0.00	0.16	1.08	0.20	NO	0.20	0.21	444.51	447.72	О.К.
4-6	22.00	445.40	2.00	447.00	24	12,62	28	0.0033	0.091	6.50	0.16	0.00	0.00	0.00	0.0	0.00	0.00	0.16	0.00	0.16	NO	0,16	0.26	447.26	447.60	О.К.

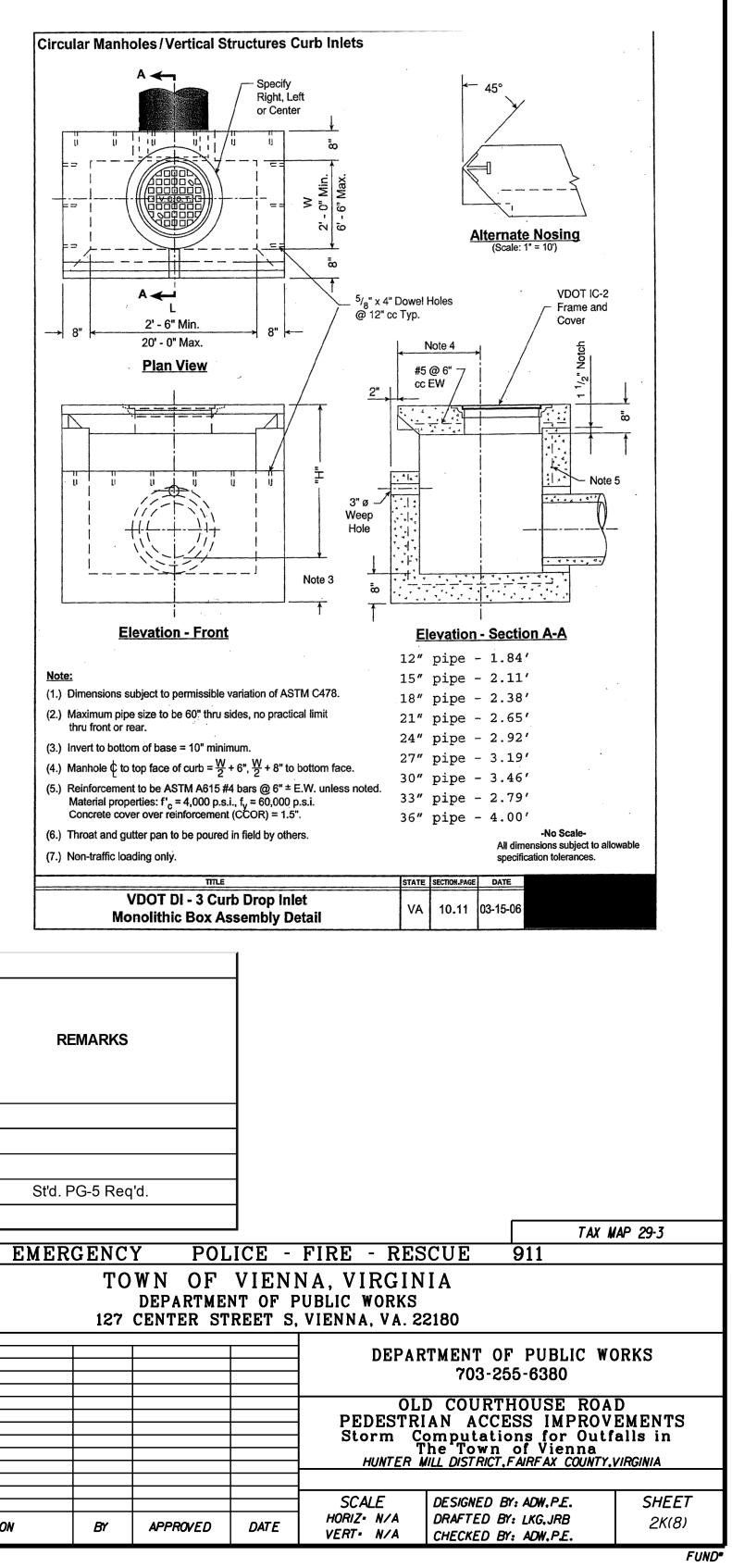
PROP		DIT	CH (COMF	PUTA	TION	S										DESIGNI	ED BY:	JZ	DA	TE: 5/	106/2016
FORM LD-	268																CHECKE	D BY:	SCT	UN	ITS: EM	IGLISH
													Earth		Pr	otective Lin	ing					
					c	CA							n=.025 (USGS-CL)		n=.05		n=.013 n=.035	•				Depth
STA. 1	ΓΟ STA.	FLOW	Area (Acres)	C-value	INCR.	ACC.	T _c	l ₂	Q ₂	C or F	Slope Ft/Ft	ALLOW. VEL.	VEL.	Q _n	VEL	Depth	Q _n	Depth	I ₁₀	Q ₁₀	Depth	Available I
Westbr	iar Court																					
Righ	t Side																					
102+25	103+12	▼	8.81	0.33	2.91	2.91	17	3.06	8.90	С	0.0640	2.3	7.0				8.90	0.3	4.16	12.09	0.4	1.0

DITCH TYPICAL





Note: All elements herein are part of Phase I plans and are shown for information only. Separate locality approval has been obtained by Town of Vienna SWM within Town of Vienna.



Drainage, SWM, and Outfall Narratives (Phase 1& Phase 2)

Introduction

This project is a pedestrian access improvement project along Old Courthouse Road NE between Pine Valley Drive and Gosnell Road. The project is mostly in the Town of Vienna, Virginia, but ties into existing sidewalks in Fairfax County at each end. The project proposes to add curb and gutter, sidewalk, and curb ramps with pedestrian crossings at Westbriar Court. Additionally, all drainage will be collected and conveyed via new proposed closed storm sewer systems which will tie into existing storm sewer systems. The project is located in the Wolftrap Creek watershed management area which is within the greater Difficult Run watershed (PL22).

The topography for this project is relatively flat with open ground cover and existing roadway. Areas adjacent to the project limits are residential. The construction area is adjacent to the existing roadway The project is within the limits of a watershed identified by Fairfax County as: Difficult Run (HUC Code

PL22). The outfall analysis is provided to demonstrate adequacy on this sheet.

The stormwater management (SWM) requirements are assessed in accordance with Virginia Department of Transportation (VDOT) and Department of Environment and Quality (DEQ) criteria for existing versus post-project conditions at outfalls within the receiving drainage basin. This project is not grandfathered, and technical criteria IIB will apply in accordance with Fairfax County Stormwater Management Ordinance. There are no stormwater management facilities proposed as part of this project. A waiver of detention requirements has been applied for this project to satisfy the water quantity requirements. The Virginia Runoff Reduction Method was used to determine the phosphorus removal requirement. Water quality requirements within the Town of Vienna project limits will be met through the use of Filterra tree box filters; water quality requirements within Fairfax County project limits will be met through the purchase of nutrient credits.

Storm sewer pipe computations, including LD-229, LD-204, and LD-347, are presented on sheets 2K(3), 2K(4), and 2K(5) for structures outfalling in Fairfax County and on sheets 2K(6), 2K(7), and 2K(8) for structures outfalling in the Town of Vienna. The storm sewer and inlet layouts are intended to drain the roadway in conformance with the VDOT Drainage Manual (VDM) Chapter 9 and convey the project runoff to an adequate outfall. A soils map and tabulation is provided on sheet 2E.

Compliance with the Virginia Erosion and Sediment Control Regulations Minimum Standard 19 (VESCR MS-19) is verified by the outfall analysis through the design of receiving channels and the analysis of existing downstream systems as required.

Drainage Design Criteria and Methodology

This narrative summarizes our understanding of the design criteria and methods of analysis employed in the design of Old Courthouse Road drainage systems. The criteria as defined in the latest edition of the VDOT Drainage Manual (VDM), including all of its Technical Supplements, and I&IM are generally applied. A list of computer software utilized for this project is also provided.

Hydrology

The Rational method was utilized to calculate flow rates to all structures, inlets, and culverts in cubic feet per second (cfs) for drainage areas less than 200 acres. Runoff coefficients were taken from Appendix 6E-1 of the VDM.

Rainfall Intensity

Rainfall intensities used for rational method design of facilities are based upon the NOAA "Atlas 14" Rainfall Precipitation Frequency Data and assigned B, D, & E factors. The following rainfall intensities are developed from chart #76 B, D, & E factors for Fairfax County, Virginia.

Recurrence	RAIN	IFALL INTENSITY Duration (To	' (INCHES PER Ho c – Minutes)	OUR)
Interval (yr)	5	10	15	30
2	5.23	4.19	3.51	2.41
10	6.77	5.45	4.62	3.26
25	7.69	6.15	5.22	3.73
100	9.10	7.28	6.22	4.57

The correction factors of 1.1 and 1.25 shall be applied to 25-yr and 100-yr storm intensities respectively

Storm Sewer Design

All storm sewer pipes along Old Courthouse Road are designed to convey the 10-year design storm event based upon Tables 9-1 & 9-2 of VDM Chapter 9.3.1. A minimum of 0.1-foot drop between the lowest incoming storm sewer pipe through a manhole or inlet and the outgoing storm sewer pipe invert is provided where possible. The Hydraulic Grade Line is analyzed for the 10-year storm event for all storm sewer systems with more than two links utilizing the PipeSoftVA 2.0 computer program. Specified storm sewer pipe materials shall comply with VDOT Drainage Manual and Road and Bridge Standards for "Allowable Pipe Material for Storm Sewer Systems."

Inlet Design

Detailed inlet reports have been provided as documentation for inlet design computations. They were generated using the InletSoftVA 2.0 modeling computer program, which utilizes the HEC-22 methodology to calculate the spread and depth for roadway inlets on grade and in sump.

<u>Roadway Inlets on Grade</u>: Drop inlets on grade are designed for intensities of four (4) inches per hour The maximum allowable spread from the face of curb for drop inlets on grade is half the width of the travel lane + the width of the gutter pan. The maximum allowable spread is 9.5 feet $(1/2 \times 16 \text{ feet} + 1.5)$ feet) for Old Courthouse Road, 8.1 feet (1/2 x 13.5 feet + 1.5 feet) for Westbriar Court (West), and 6 feet $(1/2 \times 12 \text{ feet} + 0 \text{ feet})$ for Westbriar Court (East).

A minimum of ninety percent capture efficiency has been attempted to maximize inlet efficiency. At super-elevation reversals, curb returns and intersections, we have made every attempt to provide 100% interception.

Roadway Inlets at Sumps: In order to correctly evaluate the performance of sump inlets, the overflow from upstream inlets has been accounted for. The maximum allowable spreads for sump inlets are the same as for inlets on grade. To compensate for partial clogging, the computed slot length value will be adjusted by multiplying by a factor of 2. Locations of 0.10% longitudinal slope approaching sumps will be checked to assure that the allowable maximum spread is not exceeded. Flanking inlets shall be located where the edge of pavement elevation is no higher than 0.3 feet above the edge of pavement elevation at the sag point.

Hydrology/Hydraulics Software Utilized In Drainage Computations

- InletSoftVA, Version 2.00.11 Virginia Edition
- PipeSoftVA, Version 2.00.11 Virginia Edition
- CulvertSoftVA, Version 2.00.03 Virginia Edition 3.

Stormwater Management Narrative

The project is within the limits of a watershed identified by Fairfax County as: Difficult Run (HUC Code PL22). Stormwater Management requirements are assessed for individual watersheds in accordance with VDOT and Fairfax County criteria for existing versus post-project conditions at outfalls within the receiving watersheds. "Site Area" as defined in SWPA 12-01 was calculated to develop the overall SWM approach (to meet the requirements and determine the treatment required). The overall site area is 1.30 acres, of which 1.08 acres is within the limits of the Town of Vienna and 0.22 acres is within Fairfax County. The total existing impervious area is 0.51 acres and the total proposed impervious area is 0.75 acres. This project results in 0.24 acres of new impervious area. An overview of the approach to the SWM requirements in the watershed is as follows:

As part of this project, there are no structural methods proposed. A waiver of the detention requirements has been requested for this project. A waiver of WQN requirements is justified as an adequate receiving channel is available. This project proposes 0.24 acre of new impervious area. At Outfall #1A in Fairfax County, there is a minor increase in peak flows due to an increase in impervious area. The outfall at this location is an existing channel. At Outfall #3 in Fairfax County, there is a negligible increase in peak flows due to an increase in impervious area. The outfall at this location is an existing storm sewer system. At Outfall #2 in the Town of Vienna, there is a minor increase in peak flows due to an increase in impervious area. The outfall at this location is existing curb and gutter. At Outfall #1 in the Town of Vienna, there is a minor increase in peak flows due to an increase in impervious area. The outfall at this location is an existing storm sewer system. The adequacy of project outfalls to convey storm water is discussed on this sheet. WQL within this sub-watershed is addressed by the proposed purchase of nutrient credits and Filterra tree boxes.

BMP Narrative

This project is not grandfathered and, therefore, technical criteria IIB applies to this project in accordance with the Fairfax County Stormwater Management Ordinance. This project is considered as a re-development project. The WQL requirements for this project were assessed in accordance with SWPA 12-01, and the Virginia Runoff Reduction Method spreadsheet was used to determine the phosphorus removal for the entire project.

In the Difficult Run watershed, the total site area in accordance with SWPA12-01 is 1.30 acres., of which 1.08 acres is within the limits of the Town of Vienna and 0.22 acres is within Fairfax County. The total existing impervious area is 0.51 acres and the total proposed impervious area is 0.75 acres. This project results in 0.24 acres of new impervious area. Per the calculations on sheet 2K(11) - 2K(11d), this project requires a total of 0.74 lb/year of total phosphorus load removal.

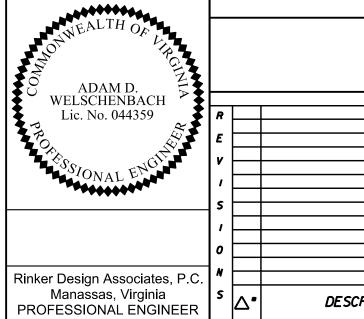
Within the Town of Vienna, 0.60 lb/yr of total phosphorus load removal is required. The phosphorus removal requirement will be met through four Filterra manufactured BMPs. Please refer to the drainage descriptions on sheet 2K and BMP notes and details on sheets 2L-2L(1) for more information.

For the Fairfax County portion of this project, 0.14 lb/yr of total phosphorus load removal is required. There are no structural facilities proposed in this phase of the project. The phosphorus removal requirement for Fairfax County will be met through the purchase of nutrient credits.

Outfall Analysis Narrative

Project runoff and outfalls are located within the Difficult Run watershed (PL22). Analysis is provided for each outfall associated with the project. With this project, three outfalls have been identified as key locations of study for adequate outfall analysis. The pre- versus post-development drainage conditions of the outfalls are tabulated in the Outfall Analysis Summary Table on sheet 2K(10). The Outfall Maps on sheet 2K(10) identify the location and limits of analysis which are based on peak flow rate. The site's peak flow rate from the 10-year 24 hour storm event is less than or equal to 1.0% of the existing peak flow rate from the 10-year 24 hour storm event prior to the implementation of any stormwater quantity control measures. Offsite drainage areas flowing to or through the project are tabulated assuming actual or current proposed land use.

Soils data provided on sheet 2E is used to determine maximum allowable velocity for the 2-year storm. Soils data is taken from the mapping and data provided by Fairfax County. Individual outfalls are described in detail as follows: (See Sheet 2K(9A) for Outfall Narratives)



						TAX MI	AP 29-3
EME	RGENC	Y POL	'ICE -	FIRE - RES	CUE S	911	
		DEPARTME	NT OF P	NA, VIRGIN Public works VIENNA, VA. 2			
				DEPAR	TMENT OF 703-258	PUBLIC WO 5-6380	RKS
				PEDESTRI Drainage	AN ACCES ,SWM, and (Phase 1 &	OUSE ROAI SS IMPROVE Outfall Nari Phase 2) ARFAX COUNTY,V	EMENTS ratives
RIPTION	BY	APPROVED	DATE	SCALE HORIZ• N/A VERT• N/A	DESIGNED BY: DRAFTED BY: CHECKED BY:	LKG, JRB	SHEET 2K(9)
							5

10/21/2020 10:55:16 AM

Phase I information shown for information only. All elements of Phase I are within the Town of Vienna, which maintains their own roadways.

Outfall #1 (In the Town of Vienna)

Description: Outfall #1 is an outfall to an existing storm sewer system running from Old Courthouse Road to Westbriar Court. With the development on this project, there is an increase to the impervious area in the post-developed scenario. Therefore, there is a slight increase in the peak flow rate at this outfall. The proposed system collects runoff from Old Courthouse Road and Westbriar Court and surrounding off-site area.

Drainage Area: The proposed drainage area is 12.16 acres which is a decrease of 0.01 acre over the existing condition. There is an addition of 0.13 acre of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 19.77 cfs, which is an increase of 0.07 cfs over the predevelopment condition.

<u>Receiving System Cross Section and Capacity</u>: Outfall #1 is an existing storm sewer system downstream of an existing inlet on Old Courthouse Road. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.07 cfs to this system for the 10-year storm condition. The existing system is adequate to handle this negligible increase in peak flow. Please refer to the LD-229 Storm Computations on sheet 2K(7).

Limits of Study: The location of the outfall is shown on the Outfall Map on sheet 2K(10). The limit of analysis is 250 L.F. downstream and contained within an existing storm sewer system which extends from proposed structure 4-7 (Old Courthouse Road Station 22+25 LT) to existing structure Ex. 18.

<u>Permissible Velocity</u>: Permissible velocity is not applicable as the runoff is confined within a closed sewer system.

Easement Requirements: The existing system is located within the existing right-of-way or existing storm drain easement as necessary; therefore, no additional easement is required.

<u>Final Opinion</u>: The drainage to the existing system has an insignificant increase in peak flows and flows are contained within the existing closed storm sewer system. The peak flow rate for the project site increases by less than or equal to 1.0% of the existing peak flow rate prior to the implementation of any stormwater quantity control measures. Therefore, it is our professional opinion that Outfall #1 is an adequate outfall and the requirements of MS-19 are satisfied.

Outfall #2 (In the Town of Vienna)

Description: Outfall #2 is an outfall with sheet flow to existing curb and gutter running along Old Courthouse Road. With the development on this project, there is an increase in impervious area in the post-developed scenario. Therefore, there is a minor increase in the peak flow rate at this outfall.

Drainage Area: The proposed drainage area is 3.03 acres which is an increase of 0.09 acre over the existing condition. There is an addition of 0.08 acre of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 13.22 cfs, which is an increase of 0.52 cfs over the predevelopment condition. The existing curb and gutter is adequate to handle this minor increase in peak flow. Please refer to the Curb & Gutter Outfall Spread Computations on sheet 2K(7).

Receiving System Cross Section and Capacity: Outfall #2 is existing curb and gutter downstream of the project along Old Courthouse Road. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.52 cfs to this system for the 10-year storm condition. The existing curb and gutter is adequate to handle this increase in peak flow. Please refer to the Curb & Gutter Outfall Spread Computations on sheet 2K(7).

Limits of Study: The location of the outfall is shown on the Outfall Map on sheet 2K(10). The limit of analysis is 450 L.F. downstream of the intersection of Old Courthouse Road and Pine Valley Drive and contained within existing curb and gutter along Old Courthouse Road.

<u>Permissible Velocity</u>: Permissible velocity is not applicable as the runoff is confined within existing curb and gutter.

Easement Requirements: The existing system is located within the existing right-of-way or existing storm drain easement as necessary; therefore, no additional easement is required.

<u>Final Opinion</u>: The drainage to the existing curb and gutter has a minor increase in peak flows and flows are contained within the existing curb and gutter. Therefore, it is our professional opinion that Outfall #2 is an adequate outfall and the requirements of MS-19 are satisfied.

L______

Outfall #1A (In Town of Vienna)

Description: Outfall #1A is an outfall to an existing channel running along Westbriar Court. With the development on this project, there is an increase to the impervious area in the post-developed scenario. Therefore, there is a slight increase in the peak flow rate at this outfall. The channel collects runoff from Old Courthouse Road within Fairfax County limits and surrounding off-site area and re-enters a proposed storm sewer system downstream in the Town of Vienna.

Drainage Area: The proposed drainage area is 5.98 acres which is the same as in existing conditions. There is an addition of 0.04 acres of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 11.94 cfs, which is an increase of 0.18 cfs over the predevelopment condition.

Receiving System Cross Section and Capacity Outfall #1A an existing channel downstream of Old Courthouse Road in Fairfax County and running along Westbriar Court. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.18 cfs to this system for the 10-year storm condition. Cross section data and computations are shown on sheet 2K(10a). The capacity of the channel is shown to be adequate for conveyance of the 10 year storm event as required, with a water surface depth of 0.5 ft at cross section A-A, 0.4 ft at cross section B-B, and 0.5

ft at cross section C-C. The 10 year storm is contained within the channel.

Limits of Study: The points of study for the outfall are indicated by the cross sections shown on the Outfall Map on sheet 2K(10). The limit of analysis is 350 L.F. downstream of the proposed structure 6-3 (Old Courthouse Road Station 31+00 LT) in Fairfax County.

Permissible Velocity: Existing channel permissible velocities are based on soil classification and comply with VDM Appendix 7D-6 for existing and proposed vegetated channels and appendix 7D-2 for the existing and proposed channels without established linings or proposed channel protection. Soils data is taken from the tabulation on sheet 2E. Soil along the existing channel alignment is type 31C (Danripple Gravelly Loam), and has a maximum permissible velocity of 2.3 fps.

<u>Channel Velocity</u>: The 2 year velocity in the channel is 2.2 fps at cross section A-A, 1.9 fps at cross section B-B, and 2.3 fps and cross section C-C. These velocities are less than or equal to the permissible velocity of 2.3 fps.

Easement Requirements: Outfall #1A is an existing channel. The flows are contained within the channel, therefore no easement for the existing channel is required. Necessary easement will be procured for the proposed ditch to the existing channel.

Final Opinion: The drainage to the existing system has slight increase in peak flows and flows are contained within the existing channel. Therefore, it is our professional opinion that Outfall #1A is an adequate outfall and the requirements of MS-19 are satisfied.

Drainage, SWM, and Outfall Narratives (Phase 1 & Phase 2 Continued)

Locality Approval/Acceptance of SWM Strategy

Town of Vienna Virginia

Department of Public Works

Michael J. Gallagher, P.E. Director

May 1, 2017

Virginia Department of Transportation NoVA Local Assistance Program 4975 Alliance Drive Fairfax, Virginia 22030

RE: Locality Approval/Acceptance of SWM Strategy (Joint Town/County Project - Old Courthouse Road Pedestrian Enhancements)

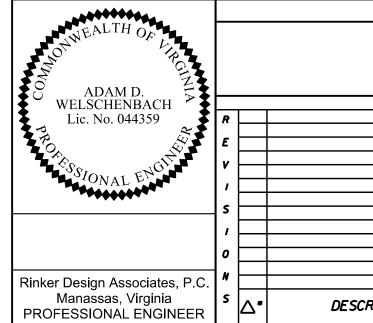
The Town of Vienna maintains all roadways within the Town's limits and the Town's stormwater system is operated under a separate permit from the State of Virginia per requirements of 4VAC50-60, "General Virginia Stormwater Management Program (VSMP) Permit for Discharges of Stormwater from Small Municipal Separate Systems."

The project is to construct pedestrian facilities along the north side of Old Courthouse Road from approximately 350' outside of the Town's northern limit, through the Town and tie to existing facilities at the other end of the Town's northern limits. The entire project is approximately 1900 LF of sidewalk, storm sewer and pedestrian facility improvements. The project is broken into two segments/phases to be constructed at the same time. Phase 1 is all the improvements (approx. 1550 LF) within the Town of Vienna. Phase 2 is all the improvements (approx. 350 LF) within the County. As part of the project's improvements a six (6) foot concrete sidewalk, storm sewer, and residential driveway entrance improvements will be constructed. The proposed improvements have been designed to minimize the amount of disturbance on residential properties and minimizing additional impervious areas on site.

Within the Town, the runoff from the project will be treated by the proposed BMP facilities (i.e. Tree Boxes) to be constructed within the project's limits for Phase 1. The Town confirms that these facilities are designed to handle impervious area draining to them as required by Town requirements. The County will provide their locality approval/acceptance separately.

In summary and as typically requested by VDOT's Location & Design Hydraulic section, this letter serves as concurrence that the project's Phase 1 for elements within the Town, as designed, meets the Town of Vienna's Stormwater Management Requirements.

Please let me know if there are further questions at 703-255-6389 or Michael.Gallagher@viennava.gov.



Outfall #3 (In Fairfax County) [Phase 2 Plan]

Description: Outfall #3 is an outfall to an existing storm sewer system running from east to west along Old Courthouse Road. With the development on this project, there is an increase to the impervious area in the post-developed scenario. Therefore, there is a slight increase in the peak flow rate at this outfall. The proposed system collects runoff from Old Courthouse Road and Pine Valley Drive and surrounding off-site area.

Drainage Area: The proposed drainage area is 17.30 acres which is a decrease of 0.07 acre over the existing condition. There is an addition of 0.04 acre of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 28.75 cfs, which is an increase of 0.01 cfs over the predevelopment condition.

<u>Receiving System Cross Section and Capacity</u>: Outfall #3 is an existing storm sewer system downstream of an existing inlet on Old Courthouse Road. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.01 cfs to this system for the 10-year storm condition. The existing system is adequate to handle this negligible increase in peak flow. Please refer to the LD-229 Storm Computations on sheet 2K(4).

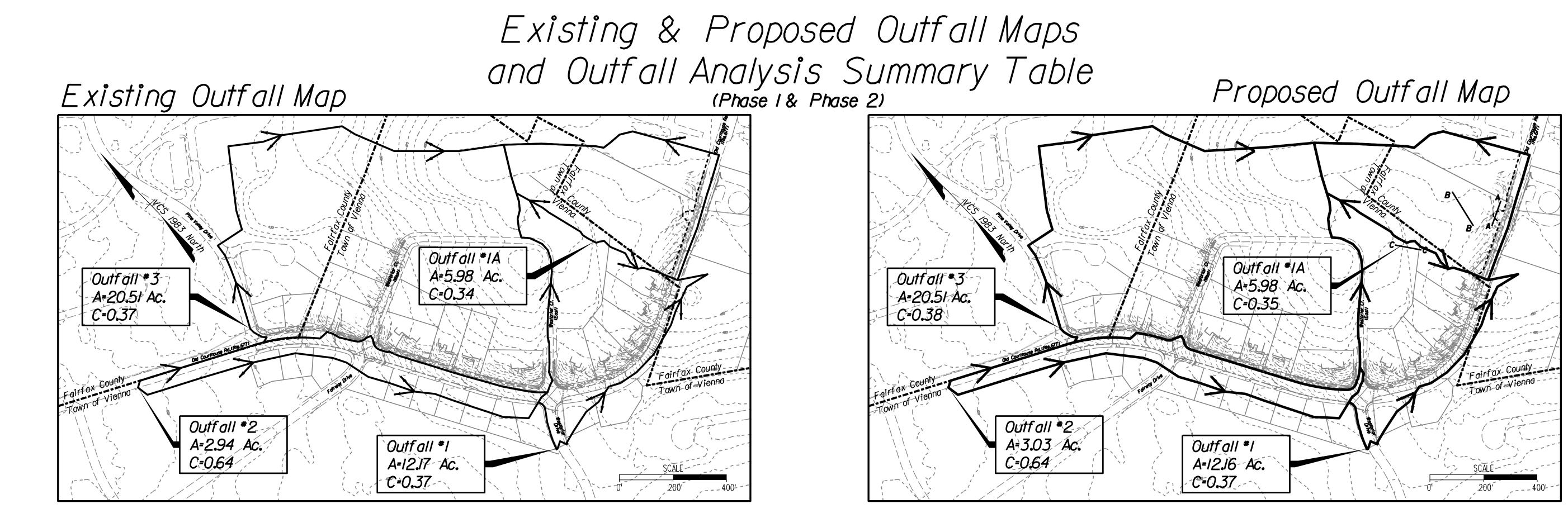
Limits of Study: The location of the outfall is shown on the Outfall Map on sheet 2K(10). The limit of analysis is 400 L.F. downstream and contained within an existing storm sewer system which extends from existing structure Ex. 2 (Old Courthouse Road Station 12+20 LT) to existing structure Ex. 7.

<u>Permissible Velocity</u>: Permissible velocity is not applicable as the runoff is confined within a closed sewer system.

Easement Requirements: The existing system is located within the existing right-of-way or existing storm drain easement as necessary; therefore, no additional easement is required.

Final Opinion: The drainage to the existing system has an insignificant increase in peak flows and flows are contained within the existing closed storm sewer system. The peak flow rate for the project site increases by less than or equal to 1.0% of the existing peak flow rate prior to the implementation of any stormwater quantity control measures. Therefore, it is our professional opinion that Outfall #3 is an adequate outfall and the requirements of MS-19 are satisfied.

					TAX	MAP 29-3
EMEF	GENC	Y POL	ICE -	FIRE - RES	CUE 911	
		DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 23		
				DEPAR	TMENT OF PUBLIC W 703-255-6380	ORKS
				PEDESTRI Drainage (Phase	D COURTHOUSE RO AN ACCESS IMPRO SWM, and Outfall Na e 1 & Phase 2 Contin ILL DISTRICT, FAIRFAX COUNT	VEMENTS rratives nued)
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(9A)



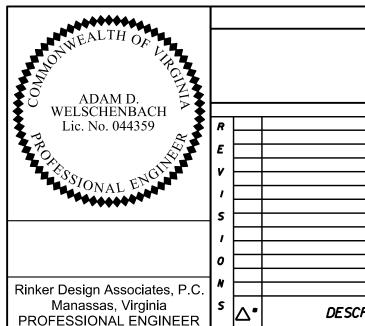
Outfall #I:Within the Town of Vienna (Phase I, for information only) Outfall #2:Within the Town of Vienna (Phase I, for information only) Outfall #3:Within Fairfax County (Phase 2, this plan)

OUTFALL	ANALYSIS	SUMMARY	TABLE
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	OUTFAL	L AN	ALYS	SIS	SUM	MAF	RY '	ΓΑΒΙ	ĿΕ																SIGNED B IECKED BY		DATE: Units		1372018 5115H				
						Dra	inage Area	(ac)					I-Va		Peak	Flow,Q	Relative	Increase		ease in		Outfall	Receiving Cl										
	Outfall	Outfall Location (Station)	Outfall Structure (if any)	Total Area	Imperv. (C=0.90)	Grass (C - 0 . 30)	Forest (C=0 . 25)	Resident. <12k sqft (C=0.45)	Resident. F I7k sqft (C=0.40)	Resident. >I/2 ac (C=0.35)	Cw	Tc (min)	(în/ 2-yr	(hr) IO-yr	(с 2-уг	fs) IO-yr	in Peak i 2-yr	Flow (cfs) IO-yr	Peak 2-yr	Flow IO-yr	Channel Velocity 2-yr Check (ft/s)	Natural Channel 2-yr Avail. Depth (in)	Manmade Channel IO-yr Avail. Depth (in)	Si Size (in)	formdrain <u>System</u> IO-yr Pipe Capacity (cfs)	Outfall Adequacy Yes/No	Rem	narks					
	Outfall *I	Westbriar C	Court																														
	Pre-Developed	99•90 LT	Ex.18	12,17	0.77	0.78	3.31	1,19	0.72	5.40	0.37	16,6	3.34	4.41	14.93	19.69					N/A	N/A	N/A	24	24.00		Existing Storm	-					
TOWN	Post-Developed	99•90 LT	Ex.18	12,16	0.90	0.72	3.23	1,19	0.72	5.40	0.37	17,1	3.29	4.35	14,97	19.77	0.04	0.07	0.3%	0.4%	N/A	N/A	N/A	24	24.00	Yes	Existing Storm	System					
OF	Outfall *IA	Old Courtha	use Road																														
VIENNA	Pre-Developed	31+00 LT	N/A	5.98	0.28	0.07	3.22	,	0.00	1.31	0.34	8.5	4.45	5.78	9.05	//.76					2.3	N/A	2.00	-	N/A	Yes	Sheet Flow to E	Existina C	Channel				
	Post-Developed	31•00 LT	6-3	5.98	0.32	0.20	3.04	I,II	0.00	1.31	0.35	8. 5	4.45	5.78	9,18	11.94	0,14	0.18	1.5%	1.5%	2.3	N/A	2.00	-	N/A	Yes	Ditch to Existin						
(Phase 1)																																	
	Outfall *2	Old Courtha		2.94	1,24	0.00	0.00	161	0.0	0.00	064	5.0	5 2 3	6.77	0.80	12.70					N/A	N/A	N/A		N/A	. Yoc	Existing Cuttor						
	Pre-Developed Post-Developed	5•75 RT	Ex.40 Ex.40	3.03	1.32	0.00	0.00	1.61 1.61	010	0.00	0.64 0.64	<u> </u>	5 .23 5 .23	6.77	9 . 80 10 . 21	13.22	0.40	0.52	4.1%	4.1%	N/A N/A	N/A N/A	N/A N/A	-	 		Existing Gutter Existing Gutter						
FAIRFAX												010	520				0110																
COUNTY	Outfall *3	Old Courtha	use Road																														
	Pre-Developed	8•57 LT	<i>Ex.</i> 7	20.51	0.46	1,18	0.60	2.74	0.22	15.31	0.37	-	013	0.20	1.00	1.54			7.074		N/A	N/A	N/A	27	48.00		Existing Storm						
(Phase 2)	Post-Developed	8•57 LT	<i>Ex</i> .7	20.51	0.99	1.06	0.60	2.74	0.22	14.90	0.38	-	013	0.20	1.04	1.60	0.04	0.06	3.83%	3.83%	N/A	N/A	N/A	27	48.00	Yes	Existing Storm	System					
Note: 2yr 24hr and	d 10 yr 24hr rainfo	all intensities	using for co	mputations	in Fairif	ax County	•					PROJE	CT SITE T	TOTALS																		TAX MAP 29-3	
,			5			,						P	re-Develope	ed	25.74											EMERGI	ENCY P	OLICE	- FIRE	E - RESCU			
											l	Po	ost-Develop	ed	26.22	34.59	0.48	0.66	1.88%	1.94%	COLOR AND	ADAM D. WELSCHENBA	ARCIN				TOWN OF DEPARTM 127 CENTER	MENT OF	F PUBLIC	C WORKS			
																					PK	WELSCHENBA Lic. No. 04435	CH 9 R E								ENT OF PUBL 703-255-6380		
																						STONAL ET	, , , , , , , , , , , , , , , , , , ,						P Exi:	OLD C EDESTRIAN sting and Pro alysis Summ HUNTER MILL (COURTHOUSE ACCESS IMF posed Outfall ary Table (Pha DISTRICT.FAIRFAX C	ROAD PROVEMENT: Maps and Out ase 1 & Phase COUNTY, VIRGINIA	; all 2)
																							0							<u> </u>			
																					Rink PRC	er Design Associ Manassas, Virg DFESSIONAL EN	ates, P.C. ^{II} inia S GINEER	ƥ	DESCRIPTION	,	BY APPROVED	D DAT	HOR	RIZ• 1°:200' DRA	SIGNED BY: ADW.P.E. AFTED BY: LKG.JRB ECKED BY: ADW.P.E.	з 2ки	

Outfall #I:Within the Town of Vienna (Phase I, for information only) Outfall #2:Within the Town of Vienna (Phase I, for information only) Outfall #3:Within Fairfax County (Phase 2, this plan)

PROJECT SITE TOTALS						
Pre-Developed	25.74	33.93				
Post-Developed	26.22	34.59	0.48	0.66	1.88%	1.94%



Worksheet for Outfall IA AA 2Yr

Input Data

Roughness Coefficient,0.025 Channel Slope,0.01000,ft/ft Left Side Slope,3.00,ft/ft (H:V) Right Side Slope, 3.00, ft/ft (H:V) Discharge, I.46, fps

Results

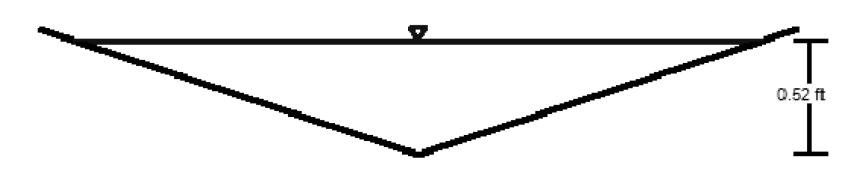
Normal Depth,0.47,ft Flow Area,0.67,sf Wetted Perimeter.2.98.ft Hydraulic Radius,0.22,ft Top Width 2.83.ft Critical Depth,0.43,ft Critical Slope,0.01631,ft/ft Velocity,2.19.ft/s Velocity Head, 0.07, ft Specific Energy,0.55,ft Froude Number,0.80 Flow Type,Subcritical

Outfall IA A-A (IO Yr) Cross Section

Worksheet for Outfall IA AA IOYr

Input Data Roughness Coefficient,0.025 Channel Slope,0.01000,ft/ft Left Side Slope.3.00.ft/ft (H:V) Right Side Slope, 3.00, ft/ft (H:V) Discharge, I.90 fps

Results Normal Depth,0.52,ft Flow Area,0.81 sf Wetted Perimeter, 3.29, ft Hydraulic Radius,0.25,ft Top Width.3.12.ft Critical Depth,0.48,ft Critical Slope,0.01574,ft/ft Velocity.2.34.ft/s Velocity Head,0.09,ft Specific Energy, 0.61, ft Froude Number,0.81 Flow Type,Subcritical



Worksheet for Outfall IA BB 2Yr

Input Data Channel Slope,0.03200,ft/ft Discharge,9.18 fps

Section Definitions Station (ft), Elevation (ft) 0.00,485.00 0+17,484.00 0•50,483.00 0.80,482.00 0.88,482.00 1.10,483.00 1.29,484.00 1.50,485.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0.00, 485.00), (0.50, 483.00), 0.060 (0.50,483.00),(1.10,483.00),0.050 (1.10,483.00),(1.50,485.00),0.060

Results Normal Depth,0.31,ft Elevation Range,482.00 to 485.00 ft Flow Area,4.95 sf Wetted Perimeter,24.08,ft Hydraulic Radius,0.21,ft Top Width 24.07, ft Normal Depth,0.31,ft Critical Depth,0.26,ft Critical Slope,0.06478,ft/ft Velocity,1.85,ft/s Velocity Head,0.05,ft Specific Energy,0.36,ft Froude Number,0.72 Flow Type,Subcritical

Worksheet for Outfall IA BB IOYr

Input Data Channel Slope,0.03200,ft/ft Discharge, II.94 fps

Section Definitions Station (ft), Elevation (ft) 0.00,485.00 0•17,484.00 0.50,483.00 0.80,482.00 0.88,482.00 1.10,483.00 1.29,484.00 1.50,485.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0.00, 485.00), (0.50, 483.00), 0.060 (0.50,483.00),(1.10,483.00),0.050 (1.10,483.00),(1.50,485.00),0.060

Results Normal Depth,0.35,ft Elevation Range,482.00 to 485.00 ft Flow Area.6.00 sf Wetted Perimeter, 26.25, ft Hydraulic Radius,0.23,ft Top Width, 26.23, ft Normal Depth,0.35,ft Critical Depth,0.30,ft Critical Slope,0.06232,ft/ft Velocity,J.99,ft/s Velocity Head,0.06,ft Specific Energy,0.41,ft Froude Number,0.73 Flow Type,Subcritical

Outfall Computations for Outfalls in the Town of Vienna

Worksheet for Outfall IA CC 2Yr

Input Data Channel Slope,0.02000,ft/ft Discharge,9.18 fps

Section Definitions Station (ft), Elevation (ft) 0.00,478.00 0•11,478.00 0.35,476.00 0.50,475.00 0.72,476.00 0.90,478.00 1.00,479.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0+00, 478,00),(0+35, 476,00),0,060 (0.35,476.00),(0.72,476.00),0.035 (0+72,476.00),(1+00,479.00),0.035

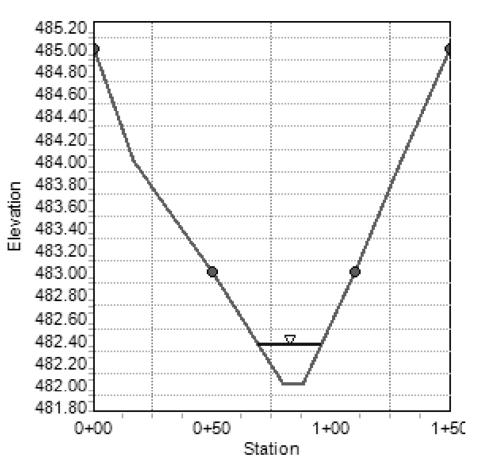
Results Normal Depth,0.47,ft Elevation Range,475.00 to 479.00 ft Flow Area,4.04,sf Wetted Perimeter.J7.32.ft Hydraulic Radius,0.23,ft Top Width, 17, 29, ft Normal Depth.0.47.ft Critical Depth.0.43.ft Critical Slope.0.02978.ft/ft Velocity,2.27.ft/s Velocity Head,0.08,ft Specific Energy,0.55,ft Froude Number,0.83 Flow Type,Subcritical

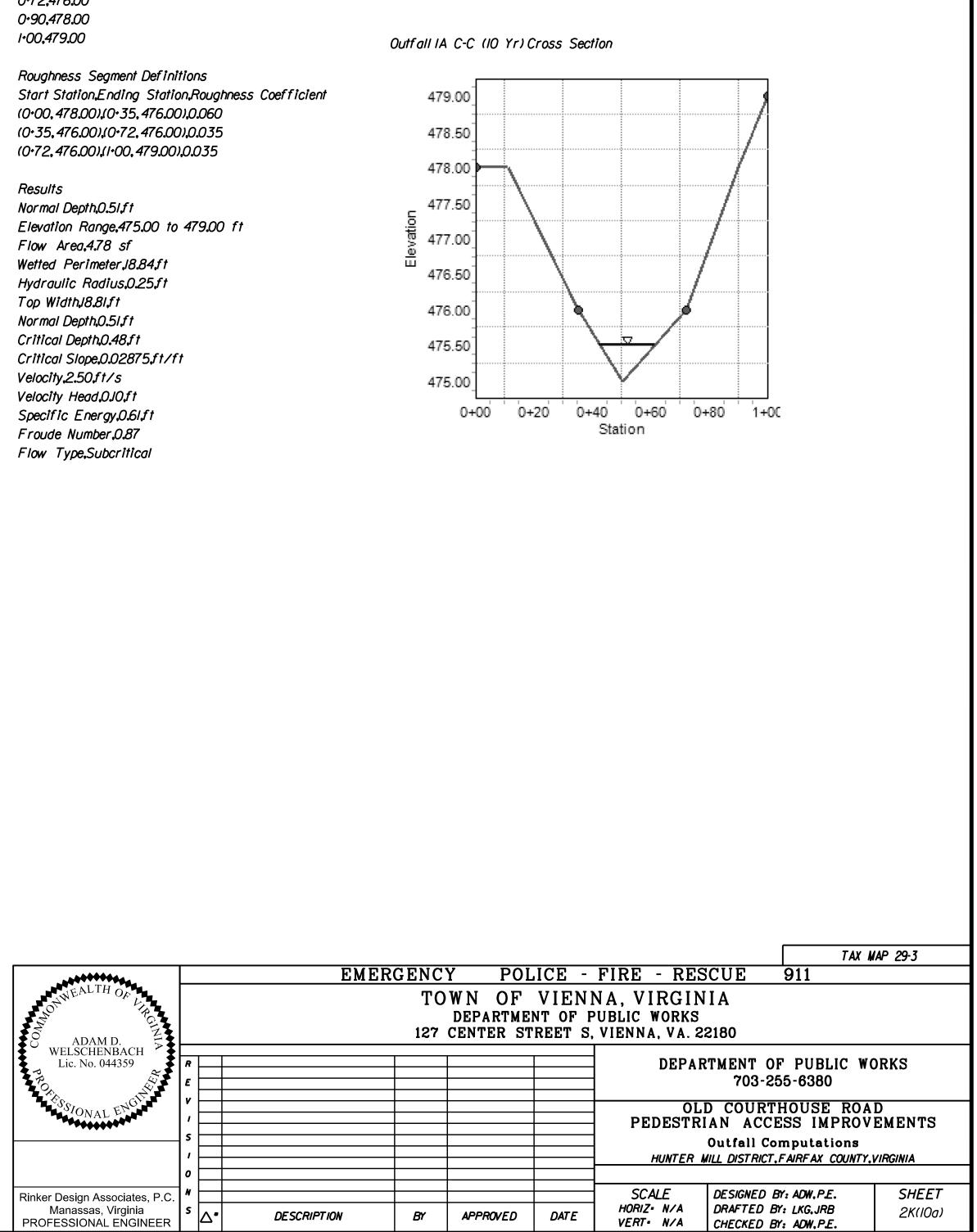
Worksheet for Outfall IA CC IOYr

Input Data Channel Slope,0.02150,ft/ft Discharge, II.94 fps

Section Definitions Station (ft), Elevation (ft) 0.00,478,00 0•11,478.00 0.35,476.00 0.50.475.00 0.72,476.00

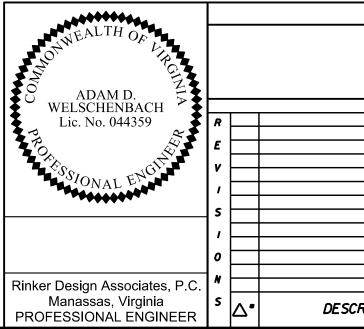
Outfall IA B-B (IO Yr) Cross Section

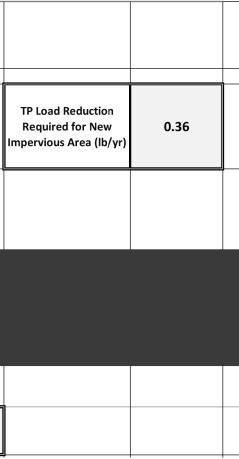




Note: All elements herein are part of Phase I plans and are shown for information only. Separate locality approval has been obtained by Town of Vienna for SWM within Town of Vienna.

Project Name: Old Courthouse Rd Pedestrian Act Date: 5/4/20		CLEAR ALL data input cells constant values										
ite Information	nt Project? Yes	calculation cells final results						l elements		•		
st-Development Project (Treatment Volume and	oads)							nase plan.				
	Disturbed Area (acres) \rightarrow 1.08	Check:						formation proval has			IIIY	
	Maximum reduction required: 20%	BMP Design Specifications List: 2011 Linear project? No)					wn of Vie			ทำ	
	in impervious cover (acres) is: 0.206 oad Reduction for Site (lb/yr): 0.59	Land cover areas entered correctly?√Total disturbed area entered?√						wn of Vie				
e-ReDevelopment Land Cover (acres) A Soils B Soils est/Open Space (acres) undisturbed.	Soils D Soils Totals				Site	Results (W	/ater Quali	ity Complian	nce)			
tected forest/open space or reforested land naged Turf (acres) disturbed, graded for Is or other turf to be mowed/managed	0.00 0.00 0.65 0.65				Area Checks	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	AREA CHECK	
pervious Cover (acres)	0.43 0.43				FOREST/OPEN SPACE (ac)	0.00	0.00	0.00	0.00	0.00	ОК.	
st-Development Land Cover (acres)	1.08				IMPERVIOUS COVER (ac) IMPERVIOUS COVER TREATED (ac)	0.19 0.19	0.15	0.11	0.04	0.00	ОК. ОК.	
	Soils D Soils Totals					0.19	0.15	0.36	0.04		AREA EXCEEDED!	
tected forest/open space or reforested land naged Turf (acres) disturbed, graded for ds or other turf to be mowed/managed	0.00 0.00 0.44 0.44				MANAGED TURF AREA TREATED (ac) AREA CHECK	0.24 ОК.	0.15 OK.	0.36 OK.	0.07 ОК.	0.00 ОК.	ОК.	
ervious Cover (acres)	0.63 0.63											
Area Check OK. OK.	OK. OK. 1.08				Site Treatment Volume (ft ³)	2,584						
ual Rainfall (inches)43et Rainfall Event (inches)1.00Forest/0	A Soils B Soils pen Space 0.02 0.03	C Soils D Soils 0.04 0.05			Runoff Reduction Volume and TP By Drainage Area							
al Phosphorus (TP) EMC (mg/L)0.26Managecal Nitrogen (TN) EMC (mg/L)1.86Imperviget TP Load (lb/acre/yr)0.411	l Turf 0.15 0.20 us Cover 0.95 0.95	0.22 0.25 0.95 0.95			RUNOFF REDUCTION VOLUME ACHIEVED (ft ³)	D.A. A 0	D.A. B	D.A. C	D.A. D 0	D.A. E	TOTAL	
initless correction factor) 0.90					TP LOAD AVAILABLE FOR REMOVAL (lb/yr)	0.55	0.41	0.44	0.13	0.00	1.53	
LAND COVER SUMMARY PRE-REDEVELOPMENT	LA Land Cover Summary-Post (Final)	ND COVER SUMMARY POST DEVELOPN	Land Cover Summa	any-Post	TP LOAD REDUCTION ACHIEVED (lb/yr) TP LOAD REMAINING (lb/yr)	0.27	0.21	0.09	0.06	0.00	0.63	
Pre-ReDevelopment Listed Adjusted ¹	Post ReDev. & New Impervious	Post-ReDevelopment	Post-Development New	· · · · · · · · · · · · · · · · · · ·								
Forest/Open Space Cover (acres) 0.00 0.00 Weighted Rv(forest) 0.00 0.00	Forest/Open Space Cover (acres) 0.00 Weighted Rv(forest) 0.00	Forest/Open Space Cover (acres) 0.00 Weighted Rv(forest) 0.00			NITROGEN LOAD REDUCTION ACHIEVED (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00	
% Forest 0% 0% Managed Turf Cover (acres) 0.65 0.44	% Forest 0% Managed Turf Cover 0.44	% Forest 0% Managed Turf Cover (carred) 0.44			Total Phosphorus			LINEAR PROJECT	:			
Weighted Rv(turf) 0.25 0.25	(acres) 0.25 Weighted Rv (turf)	(acres) 0.25			FINAL POST-DEVELOPMENT TP LOAD (lb/yr) TP LOAD REDUCTION REQUIRED (lb/yr)			1.62				
% Managed Turf 60% 51%	% Managed Turf 41%	% Managed Turf 51%	Nou Imposione Com		TP LOAD REDUCTION ACHIEVED (lb/yr)			X X				
Impervious Cover (acres)0.430.43Rv(impervious)0.950.95	Impervious Cover (acres)0.63Rv(impervious)0.95	ReDev. Impervious Cover (acres) 0.43 Rv(impervious) 0.95	New Impervious Cover (acres) Rv(impervious)	0.21	TP LOAD REMAINING (lb/yr): REMAINING TP LOAD REDUCTION REQUIRED (lb/yr):			x CHECK AREAS!	<u> </u>			
% Impervious 40% 49%	% Impervious 59%	% Impervious 49%							-			
Total Site Area (acres)1.080.87Site Rv0.530.59	Final Site Area (acres) 1.08 Final Post Dev Site Rv 0.66	Intel ReDev Site Rv 0.87 ReDev Site Rv 0.59			Total Nitrogen (For Information Purposes)							
Treatment Volume and Nutrient Load		Treatment Volume and Nutrient Load			POST-DEVELOPMENT LOAD (lb/yr) NITROGEN LOAD REDUCTION ACHIEVED (lb/yr)	11.61 0.00						
Pre-ReDevelopment Treatment Volume 0.0473 0.0430	Final Post- Development 0.0593	Post-ReDevelopment Treatment Volume 0.0430	Post-Development Treatment Volume	0.0163	REMAINING POST-DEVELOPMENT NITROGEN LOAD (Ib/yr)	11.61						
(acre-ft)	(acre-ft)	(acre-ft)	(acre-ft)									
Pre-ReDevelopment Treatment Volume 2,061 1,874 (cubic feet)	Final Post- Development 2,584 Treatment Volume (cubic feet)	Post-ReDevelopment Treatment Volume 1,874 (cubic feet)	Post-Development Treatment Volume (cubic feet)	710								
Pre-ReDevelopment TP Load 1.29 1.18	Final Post- Development TP Load 1.62 (lb/yr)	Post-ReDevelopment Load (TP) 1.18 (lb/yr)*	Post-Development TP Load (lb/yr)	0.45	TOTAL PHOSPHORUS SUMMAR	Y TAE	BLE	٨/	lote: The '	"CHECK ΔΙ	REAS!" message	in the
Pre-ReDevelopment TP Load per acre (lb/acre/yr) 1.20 1.35	Final Post-Development TP Load per acre 1.51	Post-ReDevelopment TP Load per acre 1.35			Site Results (Water Quality Compliance)				VRA	RM is gene	erated on accou	Int
Baseline TP Load (lb/yr)	(lb/acre/yr)	(lb/acre/yr) Max. Reduction Required			Total PhosphorusLINEAR PROJEFINAL POST-DEVELOPMENT TP LOAD (Ib/yr)1.62	СТ				•	g offsite areas e Areas tabs o	
41 lbs/acre/yr applied to pre-redevelopment area excluding pervious land proposed for new impervious cover) 0.36		(Below Pre- ReDevelopment Load)			TP LOAD REDUCTION REQUIRED (lb/yr)0.60TP LOAD REDUCTION ACHIEVED (lb/yr)0.63TP LOAD REMAINING (lb/yr):0.99					eadsheet.	σ Αισυδ ΙΟΝδ Ο	
ljusted Land Cover Summary: ReDevelopment land cover minus pervious land cover (forest/open space or naged turf) acreage proposed for new impervious cover.		TP Load Reduction Required for Redeveloped Area	TP Load Reduction Required for New Impervious Area (lb/yr)	0.36	REMAINING TP LOAD REDUCTION REQUIRED (lb/yr): 0.00							TAX MAP
iusted total acreage is consistent with Post-ReDevelopment acreage (minus eage of new impervious cover).		(lb/yr)			بچو	JEALTH ON		EMERC				911
Imn I shows load reduction requriement for new impervious cover (based on new elopment load limit, 0.41 lbs/acre/year).					ON NO	ADAMO			TOWN OF DEPARTM 127 CENTER S	ENT OF PUBL	IC WORKS	
	Post-Development Requirement for Si	ite Area				ADAM D. ELSCHENBACH Lic. No. 044359					DEPARTMENT OF	
	TP Load Reduction Required (lb/yr) Linear Project TP Load Reduction Required (lb/yr):	0.60 0.59				SIONAL ENGINE						5-6380 House Road Ss Improvem
	Nitrogen Loads (Informational Purpo	oses Only)					S					y Calculations
Pre-ReDevelopment TN Load (lb/yr)	9.26	Final Post-Development TN Load (Post-ReDevelopment & New 11.61										
		Impervious) (lb/yr)				esign Associates, P.C anassas, Virginia	s and a state of the state of t	DESCRIPTION	BY APPROVED		SCALE DESIGNED B ORIZ N/A DRAFTED B ERT N/A CHECKED BY	





BMP Design Specifications List: Site Summary - Linear Develo	2011 Stds & Spec				
Site Summary - Linear Develo		CS			
Site Summary Emetal Develo	opment Proj	ect***			
		ainfall (in):	43		
	Total Distur	bed Acreage:	1.08		
Site Land Cover Summary					
Pre-ReDevelopment Land Cover (acre	es) A soils	B Soils	C Soils	D Soils	Totals
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00
Managed Turf (acres)	0.00	0.00	0.00	0.65	0.65
Impervious Cover (acres)	0.00	0.00	0.00	0.43	0.43 1.08
Post-ReDevelopment Land Cover (ac	res)				
	A soils	B Soils	C Soils	D Soils	Totals
Forest/Open (acres) Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00
Impervious Cover (acres)	0.00	0.00	0.00	0.63	0.63
Site Tv and Land Cover Nutrient Load		Development	Post-	Post-	Adjusted Dro
	& New Ir	evelopment mpervious)	ReDevelopment	Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv Treatment Volume (ft ³)).66 ,584	0.59	0.95 710	0.59 1,874
TP Load (lb/yr)	1	62	1.18	0.45	1.18
Total TP Load Reduction Required (lb/yr)	0	.59	N/A***	N/A***	
	***This is a linea	ar development pr	oject		
	(P		evelopment Load ent & New Impervio	us)	Pre- ReDevelopment
TN Load (lb/yr)			.1.61	,	9.26
Maximum % Reductio		evelopment	t Project		
Total Runoff Volume Reduction (ft ³)	0		Error Summary:		
Total TP Load Reduction Achieved (lb/yr)	x				
Total TN Load Reduction Achieved (lb/yr)	0.00				
			Areas on D.A. tab(s) exceed Site tab a	reas
Remaining Post Development TP Load (lb/yr)					

Calculations - Town of Vienna

SITE COMPLIANCE SUMMARY TABLE -LINEAR DEVELOPMENT

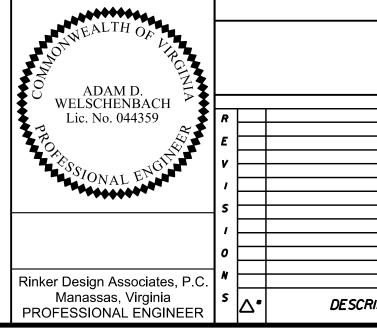
Site Compliance Summary	
Maximum % Reduction Required Below	20%
Pre-ReDevelopment Load	20%
Total Runoff Volume Reduction (ft ³)	0
Total TP Load Reduction Achieved (lb/yr)	0.63
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.99
Remaining TP Load Reduction (lb/yr) Required	0.00

f Total
0
60
40
L00

F	Total	

0	
41	
59	
00	

Pre- ReDevelopment TP Load per acre (lb/acre/yr)	Final Post-Development TP Load per acre (Ib/acre/yr)	Post-ReDevelopment TP Load per acre (lb/acre/yr)
1.35	1.51	1.35





Note: All elements herein are part of Phase I plans and are shown for information only. Separate locality approval has been obtained by Town of Vienna for SWM within Town of Vienna.

The "CHECK AREAS!" message in the Note: VRRM is generated on account of contributing offsite areas included in the Drainage Areas tabs of the spreadsheet.

						TAX M	IAP 29-3	
EME	RGENC	Y POL	ICE -	FIRE - RES	SCUE	911		
		DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 2				
				DEPARTMENT OF PUBLIC WORKS 703-255-6380				
				OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS Water Quality Calculations HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA				
RIPTION	Br	APPROVED	DATE	SCALE HORIZ• N/A VERT• N/A	DESIGNED BY DRAFTED BY CHECKED BY	: LKG.JRB	SHEET 2K(IIa)	
							FUND	

STR. 4-2A (FILTERRA)

Drainage Area A Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% 0
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	
Managed Turf (acres)	0.00	0.00	0.00	0.24	0.24	
Impervious Cover (acres)	0.00	0.00	0.00	0.19	0.19	
					0.43	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.24	0.19	873.02	0.00	0.55	0.27	0.27	

Total Impervious Cover Treated (acres)	0.19
Total Turf Area Treated (acres)	0.24
Total TP Load Reduction Achieved in D.A. (Ib/yr)	0.27
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00

STR. 4-8A (FILTERRA)

Drainage Area B Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0
Managed Turf (acres)	0.00	0.00	0.00	0.15	0.15	50
Impervious Cover (acres)	0.00	0.00	0.00	0.15	0.15	50
					0.30	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.15	0.15	653.40	0.00	0.41	0.21	0.21	

Total Impervious Cover Treated (acres)	0.15
Total Turf Area Treated (acres)	0.15
Total TP Load Reduction Achieved in D.A. (Ib/yr)	0.21
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00

STR. 5-1A (FILTERRA)

Drainage Area C Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	%
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	
Managed Turf (acres)	0.00	0.00	0.00	0.36	0.36	
Impervious Cover (acres)	0.00	0.00	0.00	0.11	0.11	
					0.47	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.36	0.11	706.04	0.00	0.44	0.09	0.35	
Total Impervious Cover Treated (acres)	0.11							
Total Turf Area Treated (acres)	0.36							
Total TP Load Reduction Achieved in D.A. (Ib/yr)	0.09							
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00							

Water Quality Calculations - Town of Vienna

0
56
44

STR. 5-2A (FILTERRA)

Drainage Area D Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0
Managed Turf (acres)	0.00	0.00	0.00	0.07	0.07	64
Impervious Cover (acres)	0.00	0.00	0.00	0.04	0.04	36
					0.11	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.07	0.04	201.47	0.00	0.13	0.06	0.06	
Total Impervious Cover Treated (acres)	0.04							
Total Turf Area Treated (acres)	0.07							
Total TP Load Reduction Achieved in D.A. (lb/yr)	0.06							
Total TN Load Reduction Achieved in D.A. (lb/yr)	0.00							

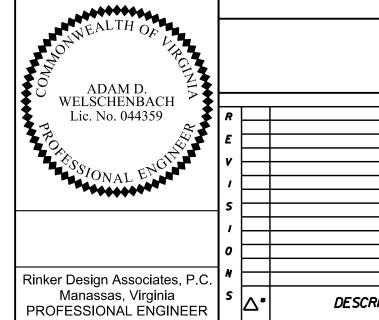
Drainage Area Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Managed Turf (acres)	0.24	0.15	0.36	0.07	0.00	0.82
Impervious Cover (acres)	0.19	0.15	0.11	0.04	0.00	0.49
Total Area (acres)	0.43	0.30	0.47	0.11	0.00	1.31

Drainage Area Compliance Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
TP Load Reduced (lb/yr)	0.27	0.21	0.09	0.06	0.00	0.63
TN Load Reduced (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00

% of Total	
0	
77	
23	



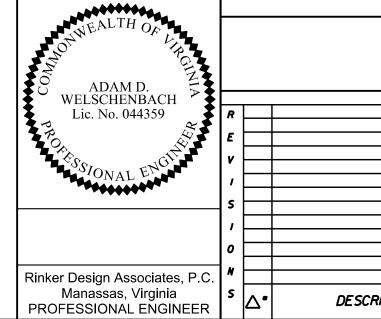
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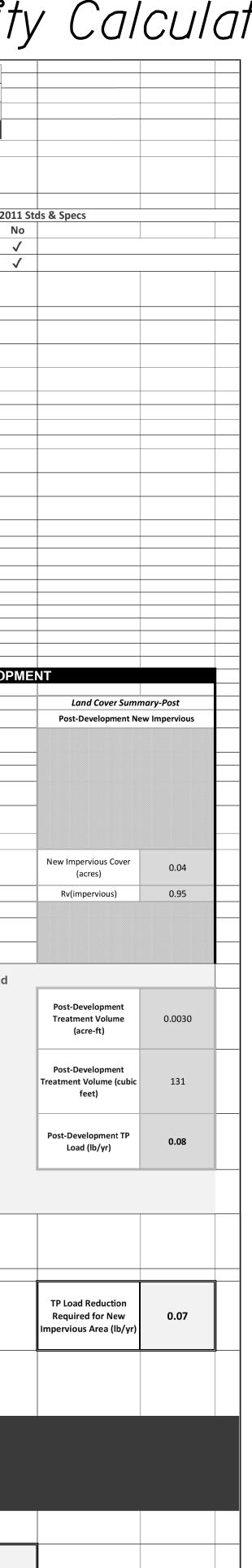
						TAX	MAP 29-3	
EME	RGENC	Y POL	'ICE -	FIRE - RE	SCUE	911		
		DEPARTME	NT OF F	NA, VIRGIN Public works Vienna, va. 2				
				DEPA		F PUBLIC W 55-6380	ORKS	-
				PEDESTR	RIAN ACCH ater Qualit	HOUSE ROA ESS IMPROV y Calculatior <i>FAIRFAX COUNTY</i>	/EMENTS ns	
RIPTION	BY	APPROVED	DATE	SCALE HORIZ• N/A VERT• N/A	DESIGNED E DRAFTED B CHECKED B	Y: LKG.JRB	SHEET 2K(IIb)	
							FUND)*

Site Information Post-Development Project Pre-ReDevelopment Land Cover (acress	(Treatme		5/4/2016 elopment Project?	N.		-			_
Post-Development Project	(Treatme			Yes				calculation cells final results	;
	(Treatme								
Pre-ReDevelopment Land Cover (acres				d Area <i>(acres)</i> →	0.22			Check:	
Pre-ReDevelopment Land Cover (acres				reduction required:	20%			Linear project?	No
Pre-ReDevelopment Land Cover(acres				ous cover (acres) is: tion for Site (lb/yr):	0.038 0.14		and cover areas ent Total disturbed		-
	5)								
Forest/Open Space (acres) undisturbed,	A Soils	B Soils	C Soils	D Soils 0.09	Totals 0.09				
protected forest/open space or reforested land Managed Turf (acres) disturbed, graded for yards or other turf to be mowed/managed				0.05	0.05				
Impervious Cover (acres)				0.08	0.08]			
Post-Development Land Cover (acres)					0.22				
Forest/Open Space (acres) undisturbed,	A Soils	B Soils	C Soils	D Soils	Totals				<u> </u>
protected forest/open space or reforested land Managed Turf (acres) disturbed, graded for				0.00	0.00	-			
yards or other turf to be mowed/managed Impervious Cover (acres)				0.10	0.12				+
Area Check	OK.	ОК.	ОК.	ОК.	0.22				+
Constants			Runoff Coefficien	ts (Rv)					
Annual Rainfall (inches) Target Rainfall Event (inches)	43 1.00		Forest/Open Space	A Soils 0.02	B Soils 0.03	C Soils 0.04	D Soils 0.05		+
Total Phosphorus (TP) EMC (mg/L) Total Nitrogen (TN) EMC (mg/L)	0.26 1.86		Managed Turf Impervious Cover	0.15 0.95	0.20 0.95	0.22 0.95	0.25 0.95		
Target TP Load (lb/acre/yr) Pj (unitless correction factor)	0.41 0.90								<u> </u>
LAND COVER SUMMARY PR	RE-REDEVE	LOPMENT					R SUMMARY P	OST DEVEL	OPME
Land Cover Summa Pre-ReDevelopment	ary-Pre Listed	Adjusted ¹		Land Cover Summa Post ReDev. & Ne	· · · ·		Land Cover Sun Post-ReDeve	-	
Forest/Open Space Cover (acres)	0.09	0.05		Forest/Open Space Cover (acres)	0.00		Forest/Open Space Cover (acres)	0.00	
Weighted Rv(forest) % Forest	0.05 41%	0.05 29%		Weighted Rv(forest) % Forest	0.00		Weighted Rv(forest) % Forest	0.00 0%	
Managed Turf Cover (acres)	0.05	0.05		Managed Turf Cover (acres)	0.10		Managed Turf Cover (acres)	0.10	
Weighted Rv(turf)	0.25	0.25		Weighted Rv (turf)	0.25		Weighted Rv (turf)	0.25	
% Managed Turf	23% 0.08	28%		% Managed Turf Impervious Cover	47% 0.12	-	% Managed Turf ReDev. Impervious	0.08	<u> </u>
Rv(impervious)	0.95	0.95		(acres) Rv(impervious)	0.95		Cover (acres) Rv(impervious)	0.95	
% Impervious	35%	43%		% Impervious	53%		% Impervious Total ReDev. Site Area	43%	
Total Site Area (acres) Site Rv	0.22	0.18		Final Site Area (acres) Final Post Dev Site Rv	0.22		(acres)	0.18	
Treatment Volume and						Treat	ment Volume and		ad
Pre-ReDevelopment Treatment Volume (acre-ft)	0.0075	0.0074		Final Post- Development Treatment Volume (acre-ft)	0.0113		Post-ReDevelopment Treatment Volume (acre-ft)	0.0082	
Pre-ReDevelopment Treatment Volume (cubic feet)	328	321		Final Post- Development Treatment Volume (cubic feet)	490		Post-ReDevelopment Treatment Volume (cubic feet)	359	
Pre-ReDevelopment TP Load (lb/yr)	0.21	0.20		Final Post- Development TP Load (Ib/yr)	0.31		Post-ReDevelopment Load (TP) (lb/yr)*	0.23	
Pre-ReDevelopment TP Load per acre (lb/acre/yr)	0.95	1.12		Final Post-Development TP Load per acre (Ib/acre/yr)	1.41		Post-ReDevelopment TP Load per acre (lb/acre/yr)	1.25	
Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area e land proposed for new impervious cove		0.07					Max. Reduction Required (Below Pre- ReDevelopment Load)	20%	
¹ Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land managed turf) acreage proposed for new impervio Adjusted total acreage is consistent with Post-ReDe	ous cover.						TP Load Reduction Required for Redeveloped Area (Ib/yr)	0.06	
acreage of new impervious cover). Column I shows load reduction requriement for new development load limit, 0.41 lbs/acre/year).									
			Post-Dev	elopment Requ	irement for	Site Area			
			TP Load	Reduction Required	(lb/yr)	0.13			
				oject TP Load Reductio					
				rogen Loads (Infoi	mational Pur	moses Oply)			
			INIL		Fu				

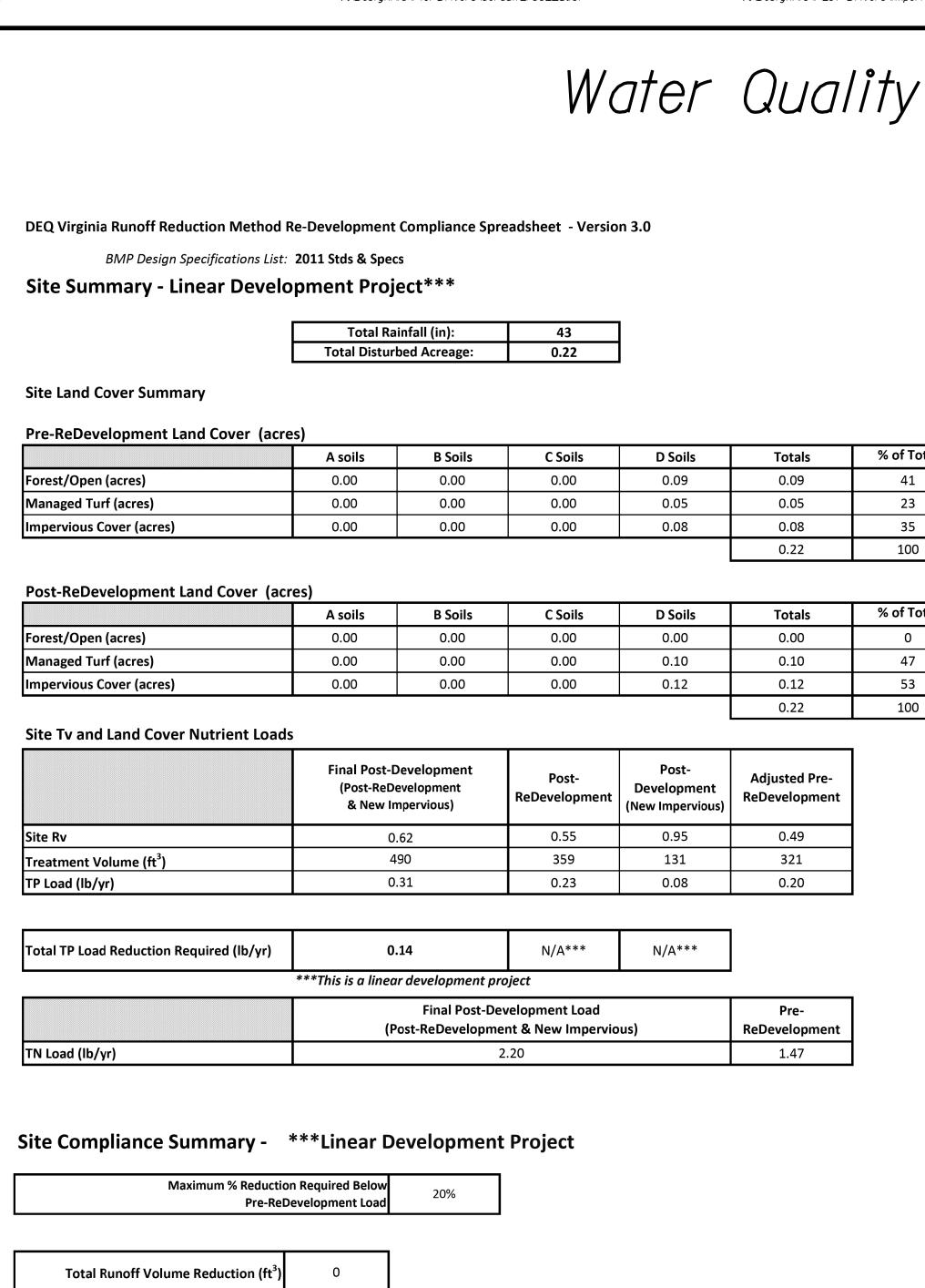
'ity Calculations - Fairfax County

Area Checks	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	AREA CHECK
FOREST/OPEN SPACE (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
IMPERVIOUS COVER (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
IMPERVIOUS COVER TREATED (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
MANAGED TURF AREA (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
MANAGED TURF AREA TREATED (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
AREA CHECK	OK.	ОК.	ОК.	OK.	ОК.	
Site Treatment Volume (ft ³)	490]				
Runoff Reduction Volume and TP By Drainage Area						
, <u> </u>	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	TOTAL
RUNOFF REDUCTION VOLUME ACHIEVED (ft ³)	0	0	0	0	0	0
TP LOAD AVAILABLE FOR REMOVAL (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
TP LOAD REDUCTION ACHIEVED (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
TP LOAD REMAINING (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
NITROGEN LOAD REDUCTION ACHIEVED (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
	0.00	0.00	0.00	0.00	0.00	0.00
Total Phosphorus		_	LINEAR PROJECT:			
FINAL POST-DEVELOPMENT TP LOAD (lb/yr)			0.31			
TP LOAD REDUCTION REQUIRED (lb/yr)			0.14			
TP LOAD REDUCTION ACHIEVED (lb/yr)			0.00			
TP LOAD REMAINING (lb/yr):			0.31			
REMAINING TP LOAD REDUCTION REQUIRED (lb/yr):			0.14			
Total Nitrogen (For Information Purposes)						
POST-DEVELOPMENT LOAD (lb/yr)	2.20	1				
NITROGEN LOAD REDUCTION ACHIEVED (Ib/yr)	0.00					
REMAINING POST-DEVELOPMENT NITROGEN LOAD (Ib/yr)	2.20					





						TAX	MAP 29-3
EME	RGENC	Y POL	ICE -	FIRE - RE	SCUE	911	
		DEPARTME	NT OF F	NA, VIRGIN Public works Vienna, va. 2			
				DEPA		F PUBLIC W 5-6380	ORKS
						HOUSE ROA SS IMPROV	
					•	<pre>v Calculation FAIRFAX COUNTY</pre>	
				SCALE	DESIGNED B	Y. ADW. P.F.	SHEET
RIPTION	BY	APPROVED	DATE	HORIZ N/A VERT N/A	DRAFTED BY	: LKG.JRB	2K(IIc)
							FUND"



Total Runoff Volume Reduction (ft ³)	0
Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	031
Remaining TP Load Reduction (lb/yr) Required	0.14

Water Quality Calculations - Fairfax County

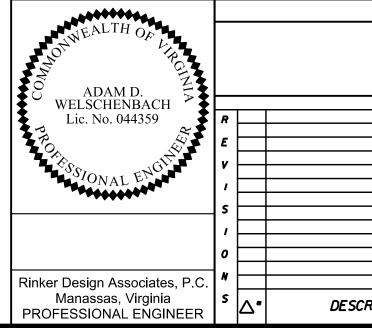
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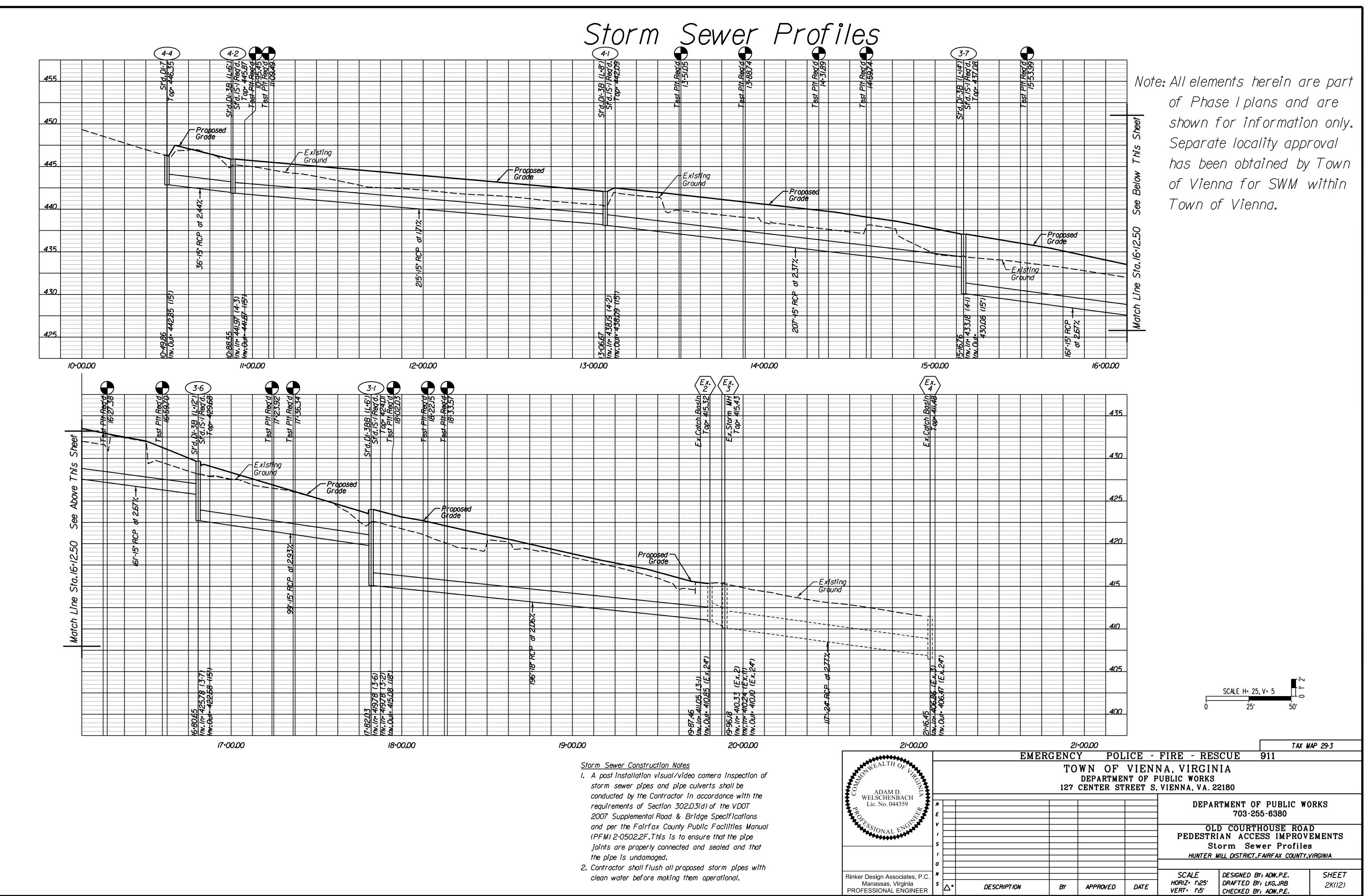
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Pre- ReDevelopment TP Load per acre (lb/acre/yr)	Final Post-Development TP Load per acre (Ib/acre/yr)	Post-ReDevelopment TP Load per acre (Ib/acre/yr)
1.12	1.41	1.25

NOTE: BMP requirements within Fairfax County are proposed to be met through the purchase of nutrient credits. See Sheet 2K for Nutrient Credit Purchase (Reservation).



						MAP 29-3			
EMERGENCY POLICE - FIRE - RESCUE 911 TOWN OF VIENNA, VIRGINIA DEPARTMENT OF PUBLIC WORKS 127 CENTER STREET S, VIENNA, VA. 22180									
				DEPAR	RTMENT OF PUBLIC V 703-255-6380	ORKS			
				OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS Water Quality Calculations HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA					
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(IId)			

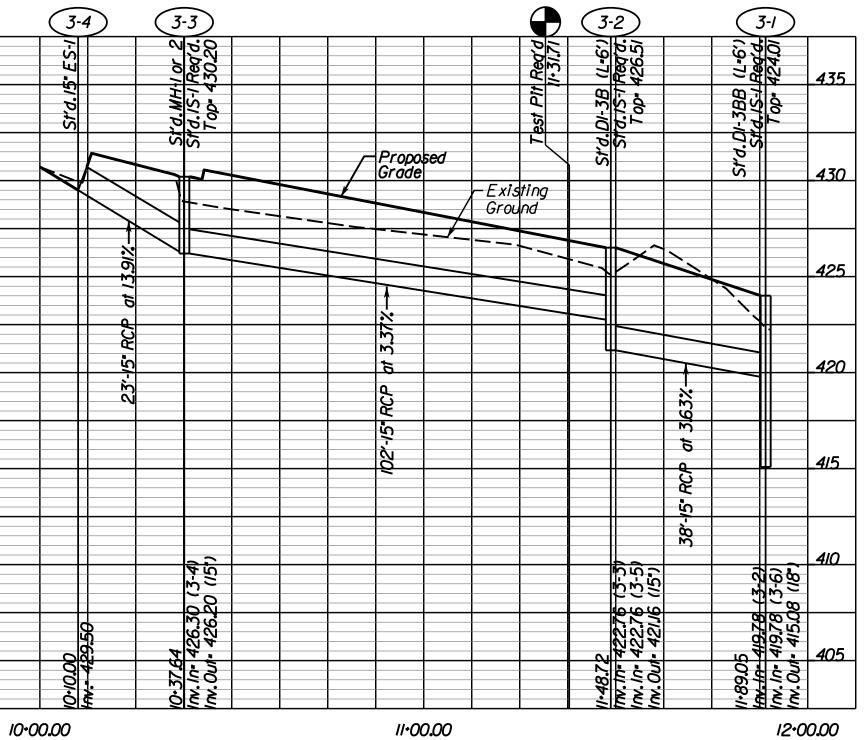


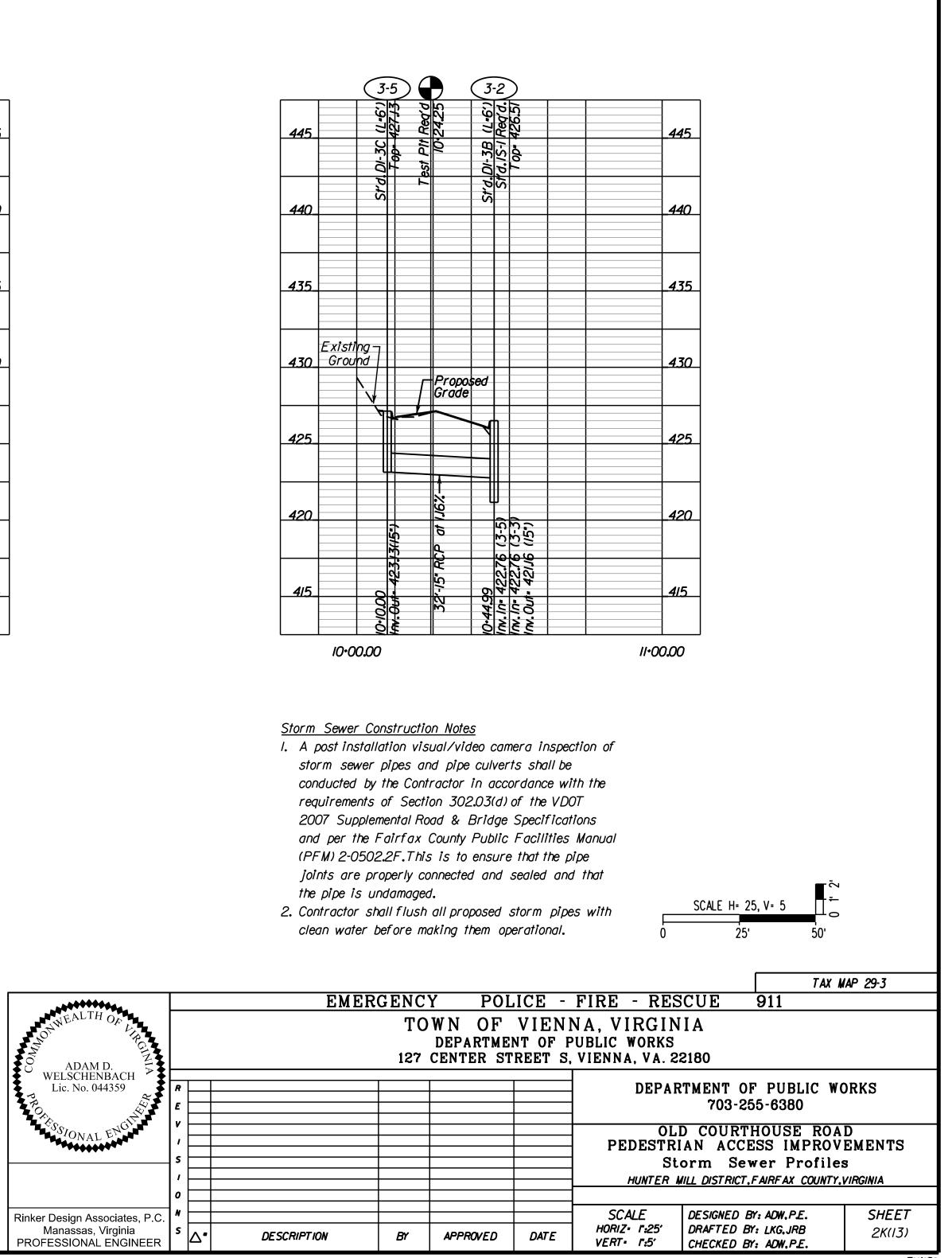
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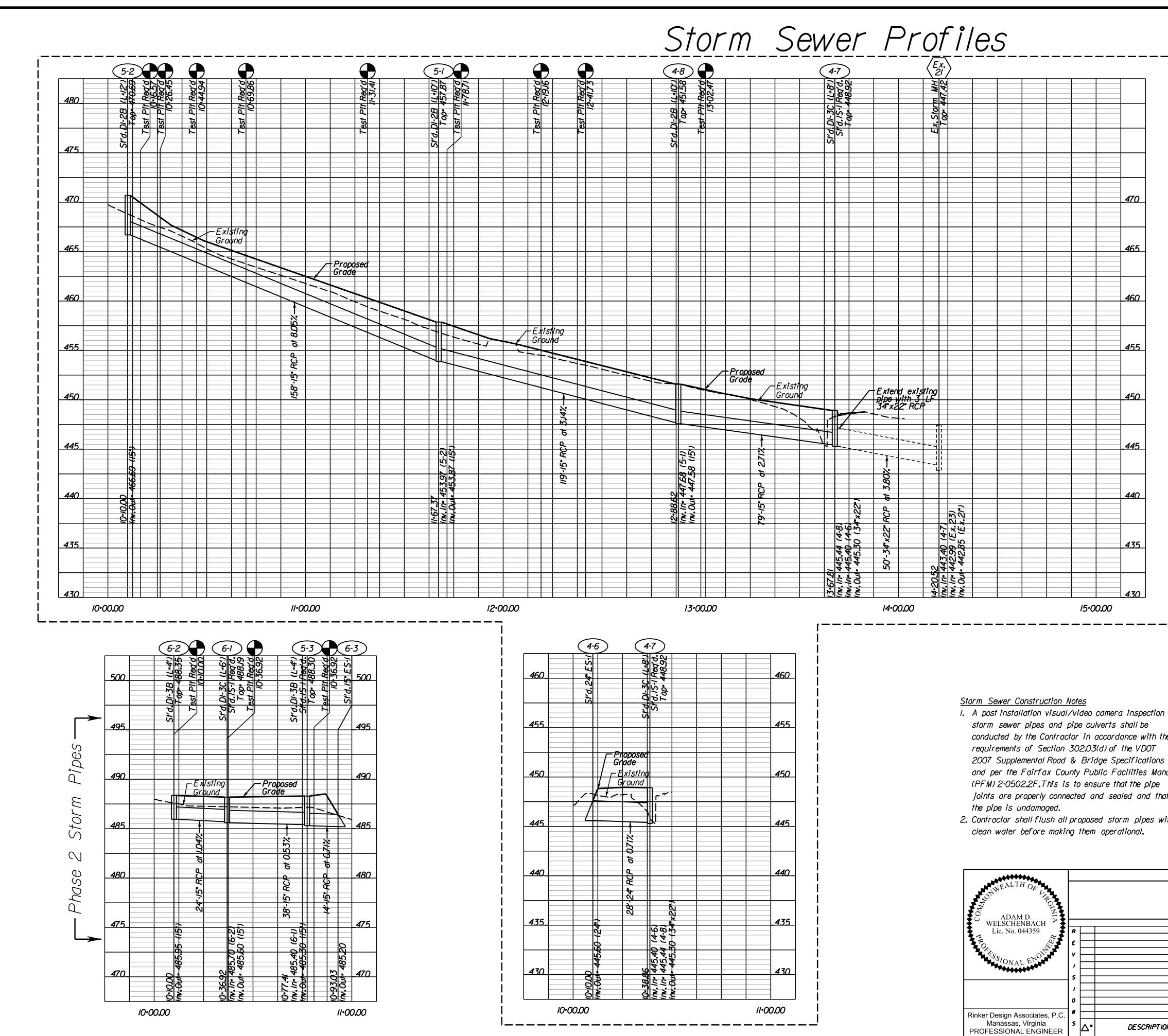
> Note: All elements herein are part of Phase I plans and are shown for information only. Separate locality approval has been obtained by Town of Vienna for SWM within Town of Vienna.

435

Storm Sewer Profiles







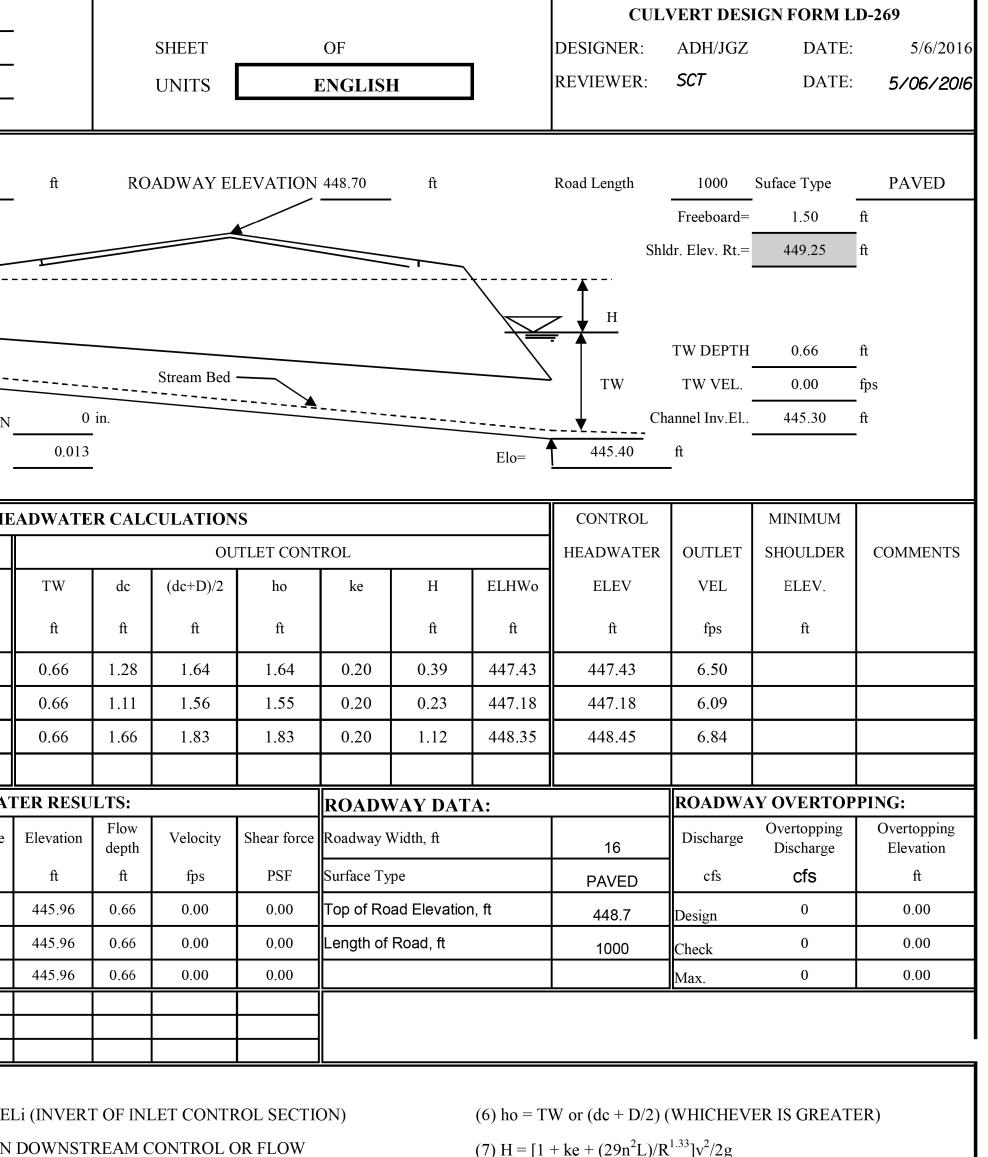
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	< _ /	Vote:	plans only. S	and o eparc ed by	are showi ate localit Town of	nre part of Pl n for informa y approval has Vienna SWM	ntion s been
ction o	of						
, ith the OT tions Manuc pipe	al						
d that es witt	'n				5 0	SCALE H= 25, V= 5	
Ē	EMER	GENC	Y POL	ICE -	FIRE - RES		MAP 29-3
			DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 23		
					DEPAR	TMENT OF PUBLIC W 703-255-6380	ORKS
					PEDESTRI St	D COURTHOUSE ROA IAN ACCESS IMPROV orm Sewer Profile MILL DISTRICT, FAIRFAX COUNTY	/EMENTS es
RIPT ION		ΒΥ	APPROVED	DATE	SCALE HORIZ• 1:25' VERT• 1:5'	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(14) FUND [•]

Storm	Com

CULVERT COMPUTATION FOR STR. 4-6 HEADWATER DEPTH

PROJECT	Old Courthouse Rd ImprovementsCOUNTYFairfax													
ROAD	Old Court	thouse R d		COUNTY										
CULVERT	4-6				VA									
	21+99													
	HYDROL	OGICAL D	ATA											
Method:	INPUT							Roadwa	y Width	16				
Drainage Area:	8.81			_										
Time of Concer	ntration	16.9			Sł	nldr. Elev. Lt. =	448.70	ft						
DESIGN FLO	WS					ELHWd=	447.20	ft 🗡		·- <u>¥</u> /				
R.I. (years)		FLOW (cfs))		Elsf	£= 445.60	ft		Hwi					
10	Design	12.70			Eli	i= 445.60	ft			\				
2	Check	9.62			L	= 28	ft							
100 Max. 21.48					S	= 0.71%	-			PEPRESSIO				
CULVERT DE	ESCRIPTION	N:			SKEW	r= 0	0			"n" Bed				
TYPE:	Single/Mul	tiple Confor	ming				-							
Inlet Edge Des	cription:				TOTAL	FLOW PER			HE					
Groove End Pro	ojecting				FLOW	BARREL		INLET CO	NTROL					
Single / Multipl	e Conforming	g / Broken B	ack Culve	erts	Q	Q/N	HWi/D	HWi	FALL	ELHWi				
MATERIAL	SHAPE	Size (in)	N	Mannings n	cfs	cfs		ft	ft	ft				
Concrete	Circular	24	1	0.01	12.70	12.7	0.91	1.60	0.00	447.20				
					9.62	9.6	0.79	1.29	0.00	446.89				
					21.48	21.5	1.42	2.85	0.00	448.45				
Broken Back	Culvert	·		TAILWA	TER DATA	:	IL			TAILWA				
LENGTH	Elev.	SKEW ^O		Channel Sha	ape					Discharge				
				Bottom Wid	lth ft	0.00	"n" =	0.035]	cfs				
						0.00		0.000	J					
					Slope Lt: (H:1V) 0.00 Slope Rt: (H:1V) 0.00					Design				
				Channel Sl		0.0100				Check Max.				
				Distance						I'IUA.				
				Elevation										
		10		"n" =										
TECHNICAL														
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(2) $HWi/D = H$										BASED O				
(3) FALL = HV	•		ll is zei	RO FOR CUI	LVERTS ON C	HADE				TH IN CHA				
SUBSCRIPT I							COMMENT	rs / DISCU	SSION:					
HWd		EADWATE		i	INLET									
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					@ culvert fac	e								

putations (For Information Only, Phase I Element)



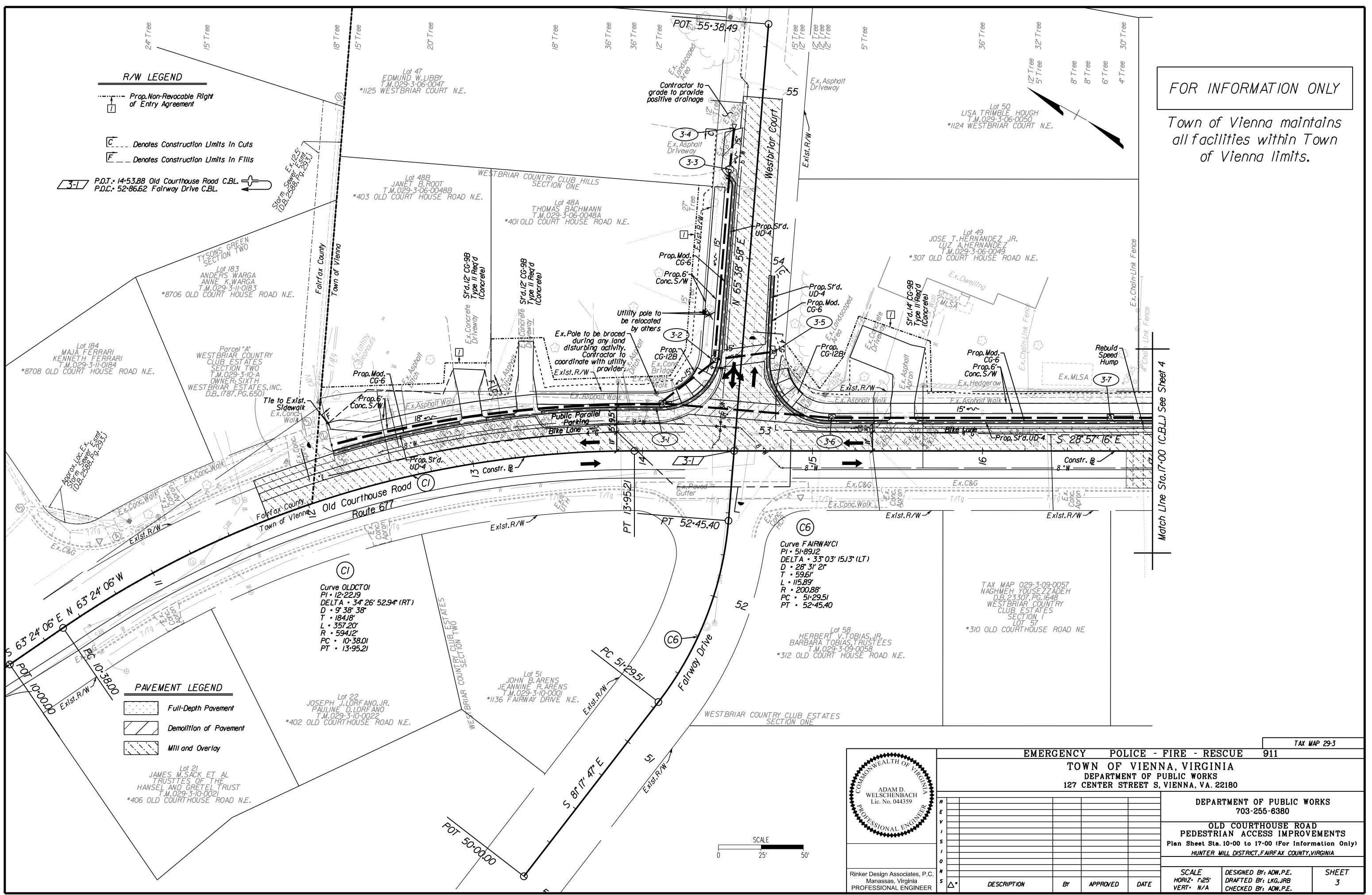
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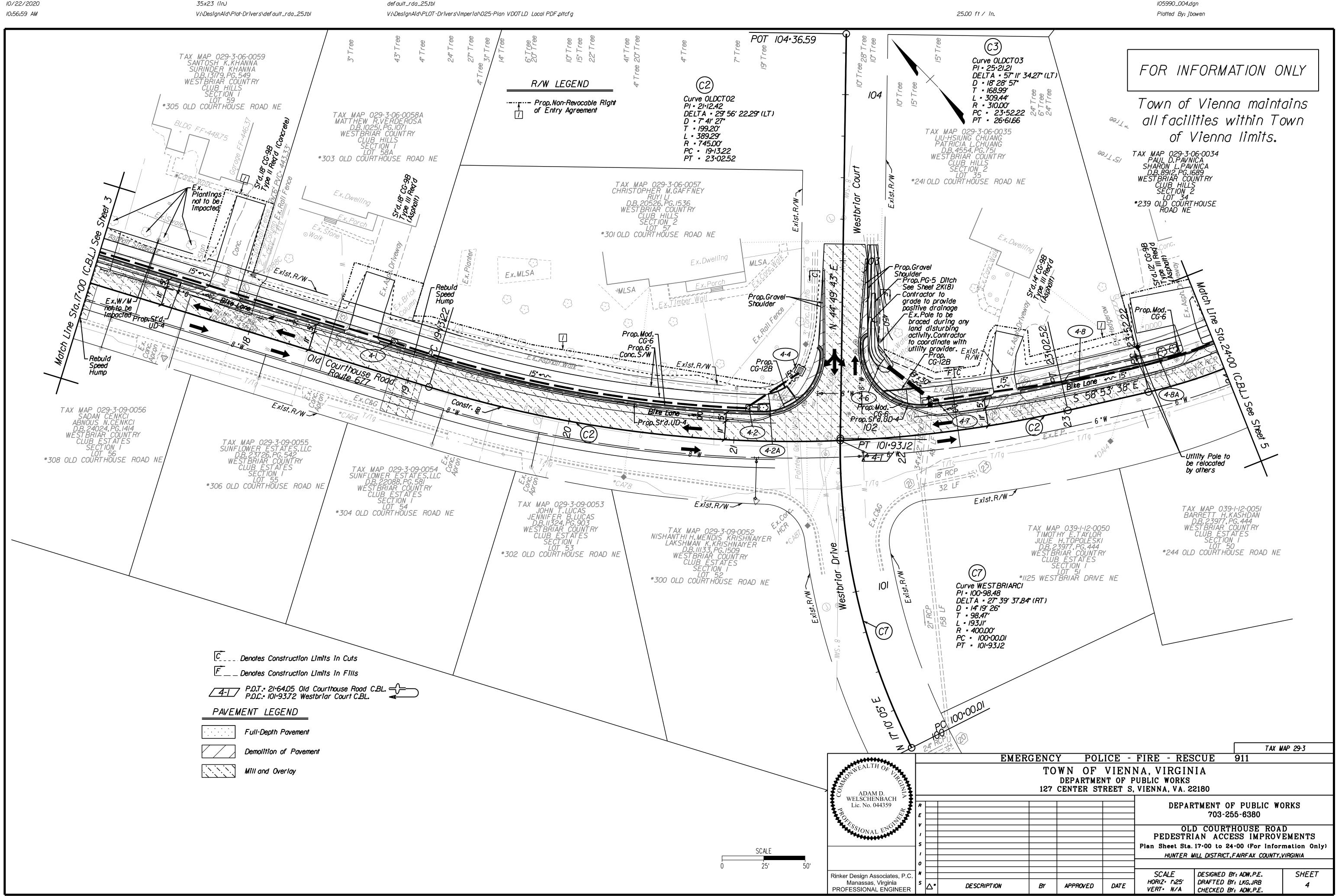
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SIZE	8:	n:	
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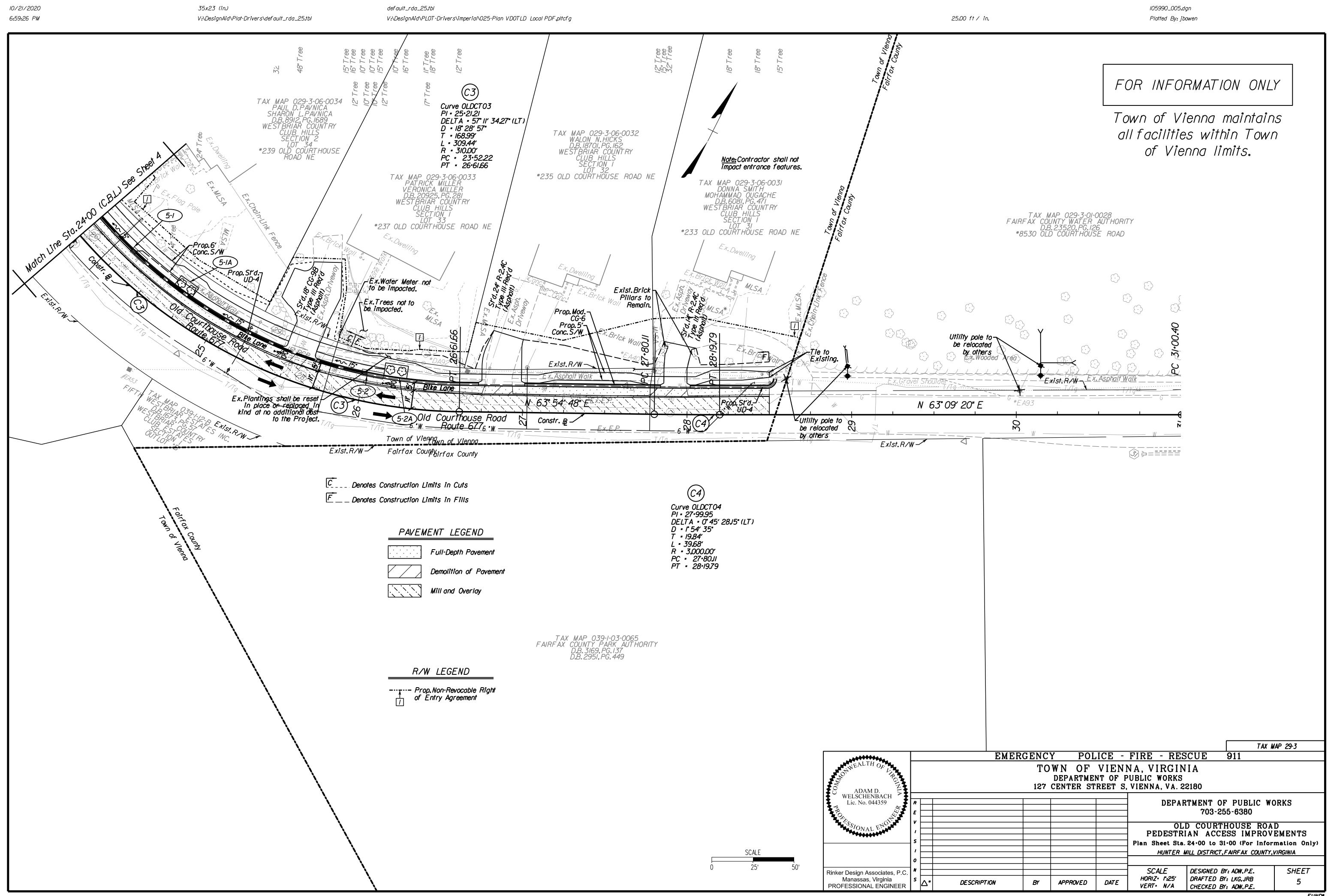
									TAX	MAP 29-3		
ſ				EMER	GENC	Y POL	ICE -	FIRE - RES	CUE 911			
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	WELSCHENBACH Lic. No. 044359	R E						DEPAR	TMENT OF PUBLIC W 703-255-6380	ORKS		
	Press Notifi								COURTHOUSE ROA			
	TONAL ET	1								CESS IMPROVEMENTS		
ł		s							Culvert Computation ILL DISTRICT.FAIRFAX COUNTY			
		0							ILL DISTRICT, FAIRFAA COUNT	, VIRGINIA		
ŀ	Rinker Design Associates, P.C.	•						SCALE	DESIGNED BY: ADW.P.E.	SHEET		
	Manassas, Virginia PROFESSIONAL ENGINEER	S	∆ •	DESCRIPTION	BY	APPROVED	DATE	HORIZ= N/A VERT= N/A	DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	2K(I5)		

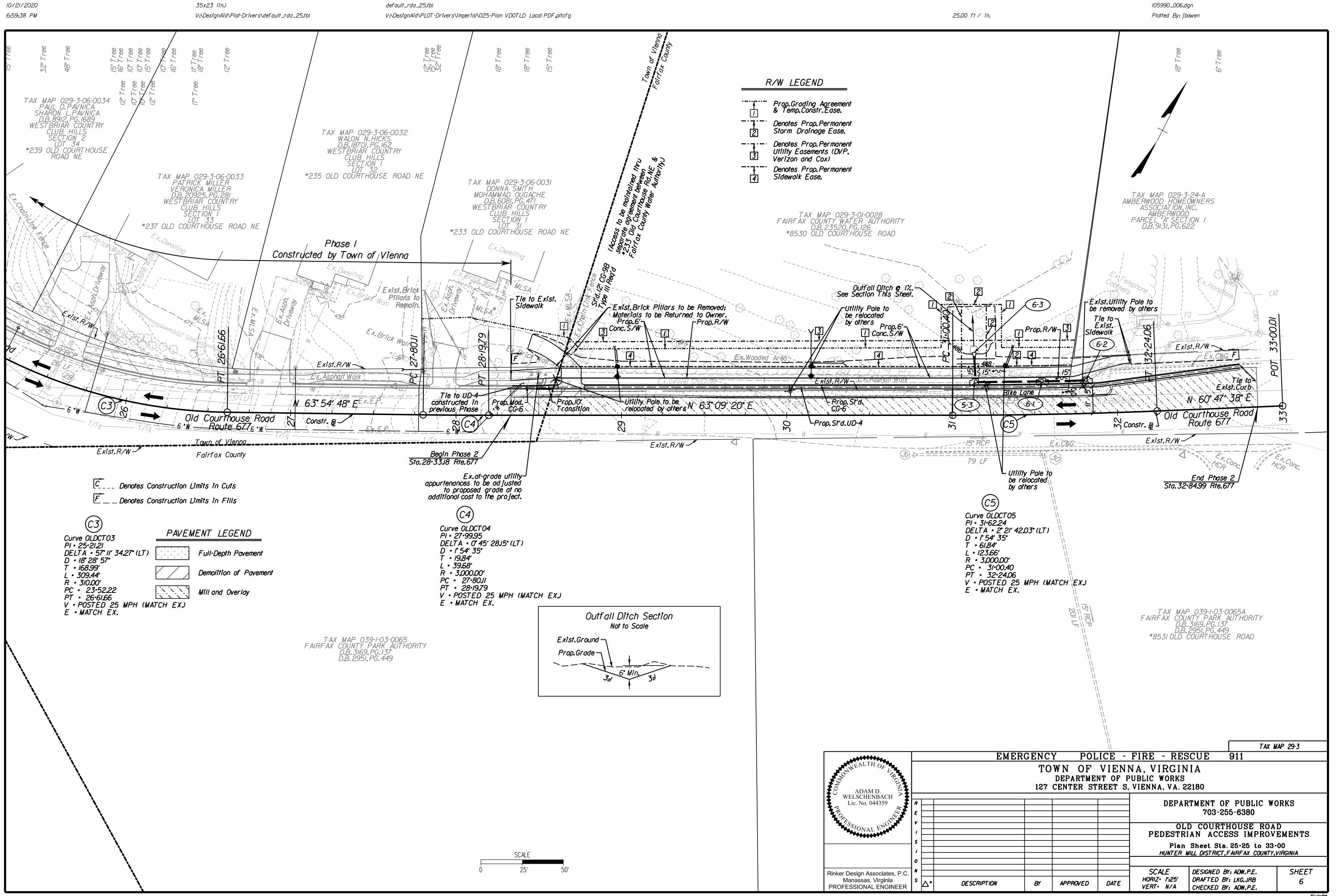




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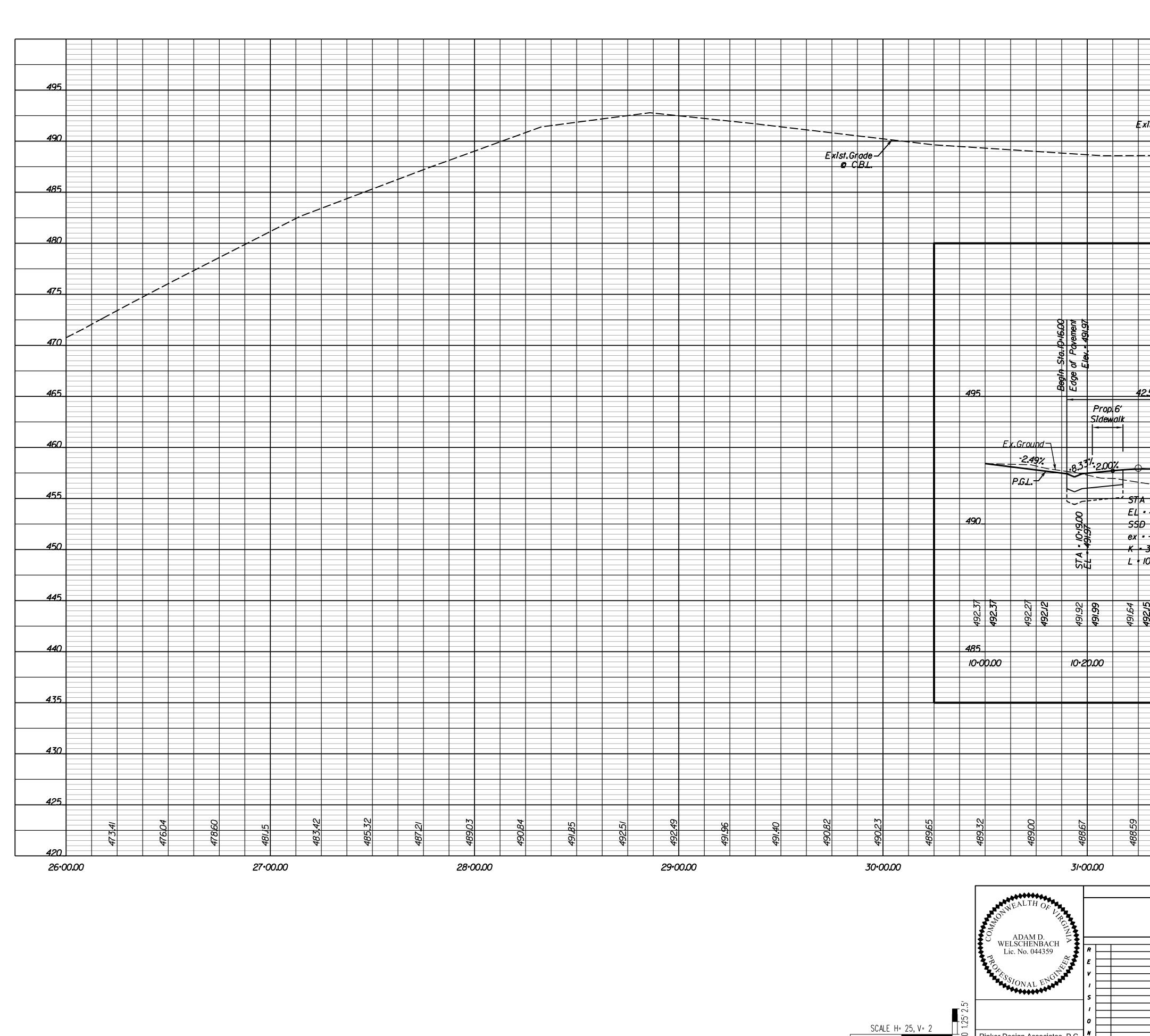


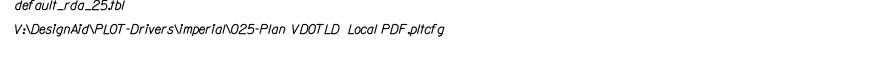
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Rinker Design Associates, P.C. Manassas, Virginia PROFESSIONAL ENGINEER

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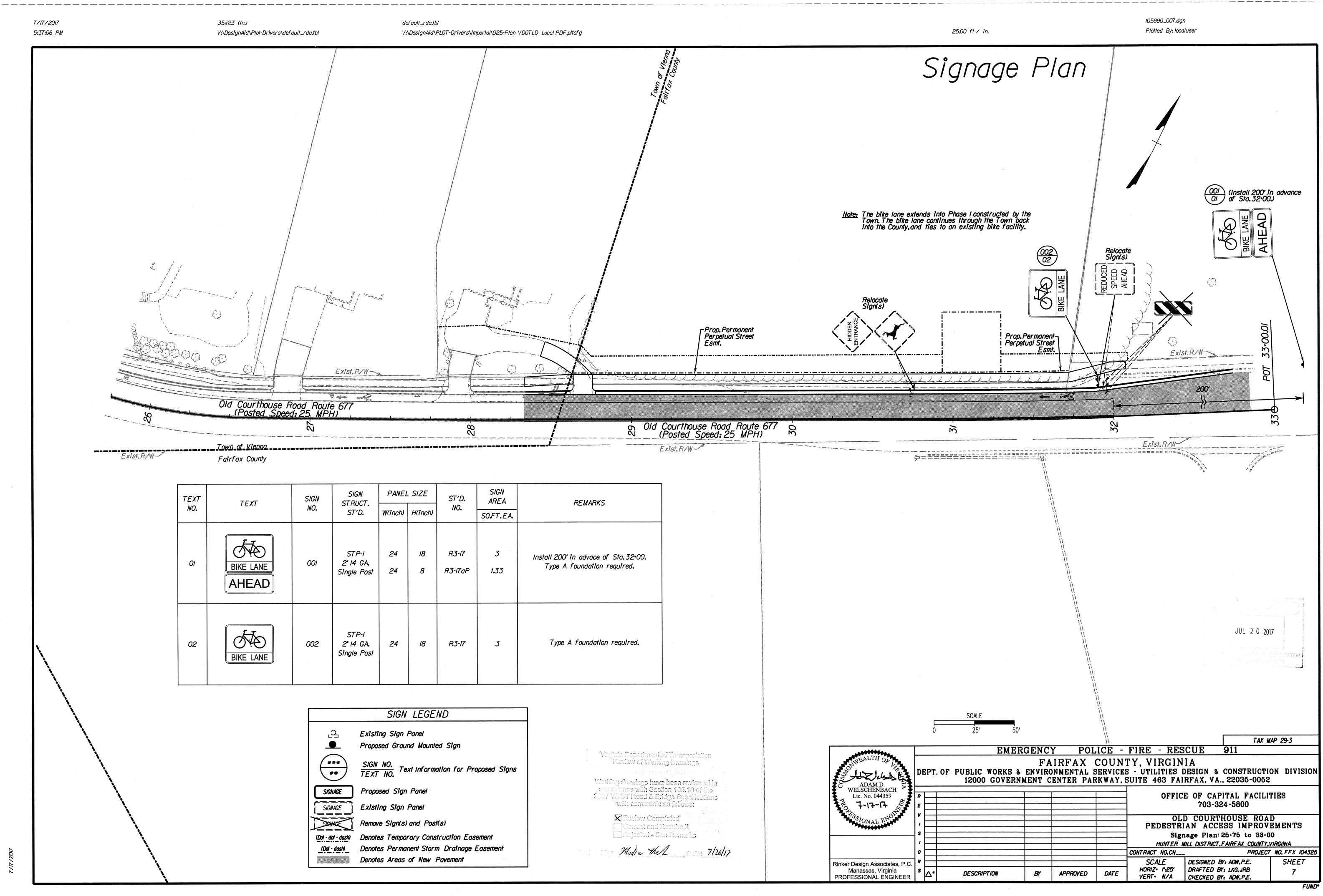
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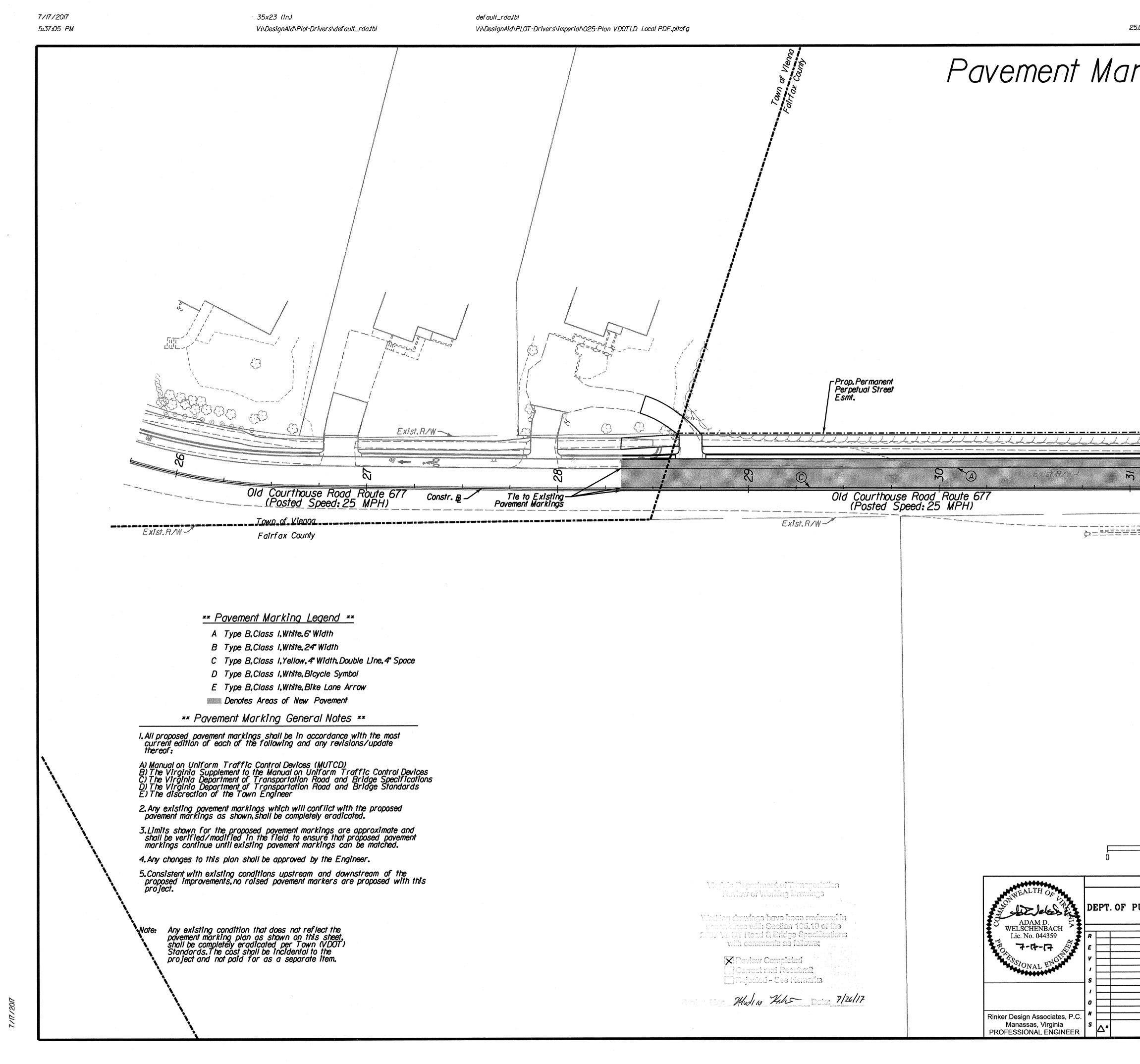
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