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Sheet No.	IE	Preliminary Right of Way Data Sheet
Sheet No.	IF - IF(I)	Survey Control Data Sheet
Sheet No.	IG - IG(2)	Construction Alignment Data Sheet
Sheet No.	IH - Iî	NOT USED
Sheet No.	IJ	TMP/SOC General Notes
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Sheet No.	IK(I)	TMP/SOC Phase 2 Sta.10.00 to 17.00
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Sheet No.	2K	Existing Drainage & Sanitary Descriptions,
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Sheet No.		Drainage, SWM, and Outfall Narratives (Phase 1 & Phase 2)
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Sheet No.		Culvert Computation
	2L - 2L(I)	BMP Notes & Details (For Information Purposes Only)
Sheet No.		Plan, Profile and Entrance Profile Sheets Sta. 10.00 to 17.00
Sheet No.		Plan, Profile and Entrance Profile Sheets Sta. 17.00 to 24.00
Sheet No.		Plan, Profile and Entrance Profile Sheets Sta. 24.00 to 31.00
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Sheet No.		Pavement Marking Plan Sta. 10.00 to 17.00
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Sheet No. Sheet No.		Pavement Marking Plan Sta.24.00 to 31.00 NOT USED
Sheet No.		Utilities Plan Sta. 10.00 to 17.00
Sheet No.		Utilities Plan Sta. 17.00 to 24.00
Sheet No.		Utilities Plan Sta. 24.00 to 31.00
Sheet No.	XI thru XI3	Old Courthouse Road (Route 677) Cross Sections

MAINTENANCE LEGEND:

OOO-TOWN OF VIENNA 1550 LF-SIDEWALK 0 LF-TRAIL 0 EA-BMP FACILITIES

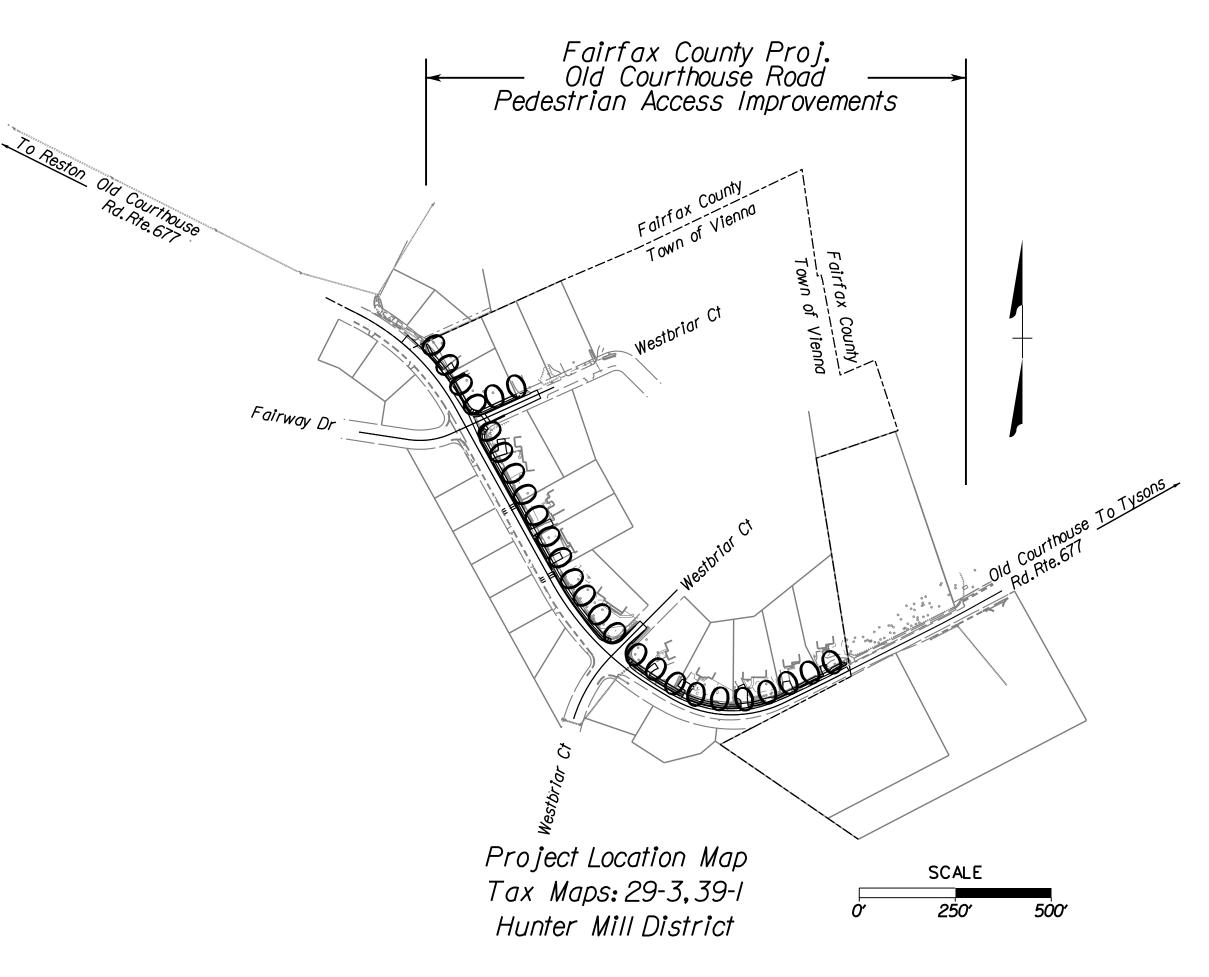
DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE VIRGINIA DEPARTMENT OF TRANSPORTATION.

THIS PROJECT IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE THIS PROJECT IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE MOST RECENT REVISIONS OF DEPARTMENT'S: 2020 <u>ROAD AND BRIDGE SPECIFICATIONS</u>, 2016 <u>ROAD AND BRIDGE STANDARDS</u>, 2009 <u>MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES</u> (MUTCD), 2011 <u>VIRGINIA SUPPLEMENT TO THE MUTCD</u>, 2011 <u>VIRGINIA WORK AREA PROTECTION MANUAL</u>, AND AS AMENDED BY CONTRACT PROVISIONS AND THE COMPLETE ELECTRONIC .PDF VERSION OF THE PLAN ASSEMBLY.

ALL CURVES ARE TO BE SUPERELEVATED, TRANSITIONED AND WIDENED IN ACCORDANCE WITH STANDARD <u>TC-5.11U</u>, EXCEPT WHERE OTHERWISE NOTED.



OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS PHASE 1 (INSIDE TOWN OF VIENNA LIMITS)

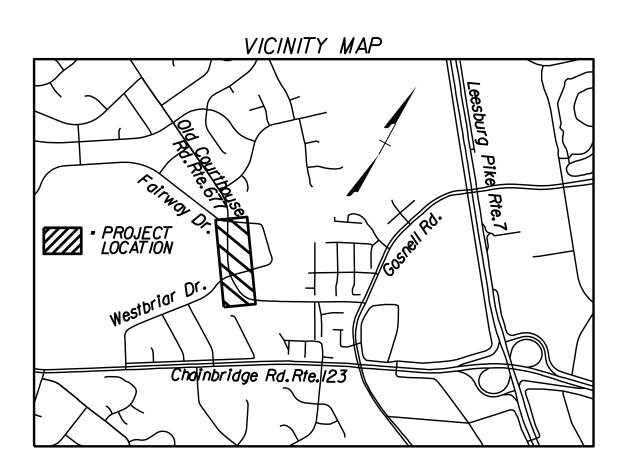


TOWN OF VIENNA, VIRGINIA DEPARTMENT OF PUBLIC WORKS

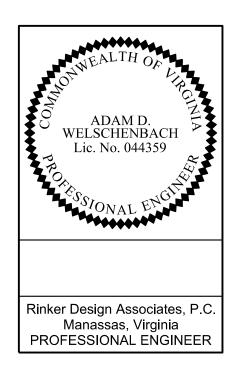


	OLD
Fr:	Rte.72
То:	North (
AADT (2019)	7,200
DHV	1,094
D (%) (design hour)	83.5
T (%) (design hour)	0
DESIGN V (MPH)	25
POSTED V (MPH)	25

FUNCTIONA	L CLASSIFICATION	
COURTHOUSE ROAD,	ROUTE 677 (VIENNA ROUTE	E 6668)
URBAN MINOR A	ARTERIAL (GS-6) - ROLLING	
4,Creek Crossing Rd `ity Line for City of Vienna	East City Line for City of Vienna Rte.123,Chain Bridge Road	East City Line for City of Vienna West City Line for City of Vienna
	10,000	8,300
	1,230	1,129
	84.0	82.6
	0	0
	25	25
	25	25







FINAL PLANS AUTHORIZED FOR CONSTRUCTION DATE DIRECTOR, TOWN OF VIENNA DEPARTMENT OF PUBLIC WORKS SHEET

VEMENTS MPRO. CESS 2020 DESTRIAN ACC - OCTOBER ۲ Д A AD PI RO AL FIN S E \Box RTHO \Box \bigcirc \bigcirc LD 0

35x23 (în.) V:\DesignAid\Plot-Drivers\default_rda_25.tbl EROSION AND SEDIMENT CONTROL EROSION CONTROL NARRATIVE PRIOR TO ANY LAND DISTURBING OPERATIONS, THE EROSION CONTROLS, AS SPECIFIED BY THE ENGINEERING PLANS, SHALL BE INSTALLED. ALL MECHANICAL AND VEGETATIVE PRACTICES SHALL BE IN CONFORMANCE WITH THE REQUIREMENTS CONTAINED IN THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL AND THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK. SLOPE AREAS THAT CANNOT BE ADEQUATELY STABILIZED BY SEEDING, SHALL BE SODDED AND STAKED. AS DISTURBED AREAS, NOT TO BE CONSTRUCTED UPON, ARE FINAL GRADED, THEY SHALL BE PREPARED, LIME AND FERTILIZER APPLIED, SEEDED AND MULCHED. FOR THE AREAS OUTSIDE THE V.D.O.T. RIGHT-OF-WAY, THE SEED SHALL CONSIST OF A MIXTURE OF KENTUCKY 31 TALL FESCUE AND KENBLUE IN ACCORDANCE WITH DEPARTMENT OF PUBLIC WORKS AND ENVIRONMENTAL SERVICES SPECIFICATIONS OR COUNTY APPROVED ALTERNATE SEED FOR AREAS LOCATED WITHIN THE V.D.O.T. RIGHT-OF-WAY, THE SEED MIXTURE SHALL BE IN CONFORMANCE WITH CURRENT V.D.O.T. REQUIREMENTS. SEDIMENT CONTROLS AND MECHANICAL DEVICES SHALL BE REMOVED FROM CONTRIBUTING AREAS AS THEY BECOME STABILIZED. THIS RESTORATION WORK WILL BE PERFORMED WITHIN 7 DAYS AFTER FINAL GRADING. ALL TEMPORARY SEDIMENT CONTROLS AND MECHANICAL DEVICES SHALL BE REMOVED FROM CONTRIBUTING AREAS AS THEY BECOME STABILIZED. FOR ADDITIONAL DETAILS, REFER TO THE CURRENT EDITION OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL. EROSION, SEDIMENTATION, AND LAND CONSERVATION NOTES 1. MEASURES TO CONTROL EROSION AND SILTATION SHALL BE PROVIDED PURSUANT TO AND IN COMPLIANCE WITH CURRENT FEDERAL, STATE AND LOCAL REGULATIONS. THE INFORMATION CONTAINED IN THE CONSTRUCTION PLANS AND/OR THE APPROVAL OF THE PLANS SHALL IN NO WAY RELIEVE THE CONTRACTOR OR HIS AGENT OF ANY LEGAL RESPONSIBILITY WHICH MAY BE REQUIRED BY THE CODE OF VIRGINIA OR ANY ORDINANCE ENACTED BY THE COUNTY OF FAIRFAX. 2. ALL AREAS, ON OR OFF-SITE, WHICH ARE DISTURBED BY THIS CONSTRUCTION AND WHICH ARE NOT PAVED OR BUILT UPON SHALL BE ADEQUATELY STABILIZED TO CONTROL EROSION AND SEDIMENTATION. ACCEPTABLE STABILIZATION SHALL CONSIST OF PERMANENT GRASS SEED MIXTURE INSTALLED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. ALL SLOPES 2:1 AND GREATER SHALL BE SODDED AND STAKED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE ENGINEER. 3. ANY DISTURBED AREA NOT PAVED, PERMANENTLY SEEDED, SODDED, OR BUILT UPON BY 1 NOVEMBER OR DISTURBED AFTER THAT DATE, IS TO BE SEEDED WITHIN 14 DAYS WITH OATS, ABRUZZI RYE OR APPROVED EQUIVALENT, AND MULCHED WITH HAY OR STRAW MULCH AT THE RATE OF 2 TONS PER ACRE. FOR ADDITIONAL DETAILS, REFER TO THE CURRENT EDITION OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL. 4. NO AREA SHALL BE DENUDED AND NOT BE DISTURBED FOR A PERIOD LONGER THAN 14 DAYS IN ACCORDANCE WITH THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL. 5. TEMPORARY DIVERSIONS, SEEDED AND MULCHED OR STAKED STRAW BALE DIVERSIONS AND OTHER CONTROL MEASURES NECESSARY ARE TO BE PLACED AS INDICATED ON THE DRAWINGS PRIOR TO OR AS THE FIRST STEP IN EXCAVATION. 6. WHEN IN ACCORDANCE WITH STATE AND FEDERAL JOB SAFETY REQUIREMENTS, ALL EXCAVATED MATERIAL IS TO BE PLACED ON TH UPHILL SIDE OF TRENCHES. NO MATERIAL IS TO BE PLACED IN STREAMS. ANY STOCKPILED MATERIAL WHICH WILL REMAIN IN PLACE LONGER THAN 14 DAYS IS TO BE SEEDED FOR TEMPORARY VEGETATION AND MULCHED. WHERE SPOIL IS PLACED ON THE DOWNHILL SIDE OF TRENCH, IT IS TO BE BACK-SLOPED TO DRAIN TOWARD THE TRENCH. WHEN NECESSARY TO DEWATER THE THE THE PUMP DISCHARGE HOSE MUST OUTLET IN A STABILIZED AREA OR A SEDIMENT BASIN. 7. WHERE STREAM CROSSINGS ARE REQUIRED FOR EQUIPMENT, TEMPORARY CULVERTS SHALL BE PROVIDED. 8. DURING CONSTRUCTION, ALL STORM SEWER INLETS WILL BE PROTECTED BY SILT TRAPS, MAINTAINED AND MODIFIED AS REQUIRED BY CONSTRUCTION PROGRESS. 9. ALL DISTURBED AREAS ARE TO BE SEEDED AND MULCHED OR SODDED WITHIN 5 DAYS AFTER BACKFILL OF THE APPLICABLE TRENC SECTION, IN ACCORDANCE WITH THE PROVISIONS CONTAINED IN THE PROJECT SPECIFICATIONS RELATING TO SEEDING AND SODDING. SPEED IS THE ESSENTIAL LAND CONSERVATION ELEMENT FOR A LINEAR PROJECT. 10. FOR FURTHER REQUIREMENTS AND DETAILS OF TREE PRESERVATION, PLANTING, EROSION AND SEDIMENT CONTROL, SEE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL AND/OR THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK. 11. ALL EROSION AND SEDIMENT CONTROLS SHALL BE REMOVED WITHIN 30 DAYS AFTER THE PROJECT IS STABILIZED. (MS-18) 12. AN EROSION AND SEDIMENT CONTROL CONTRACTOR CERTIFICATION (E.S.C.C.C.) IS REQUIRED FOR ALL LAND DISTURBING ACTIVITIES. 13. THE CONTRACTOR SHALL PROPERLY INSTALL AND MAINTAIN EROSION AND SEDIMENT CONTROLS FOR THE LIFE OF THE PROJECT; A ROUTINELY CHECK CONTROL DEVICES BEFORE, DURING AND AFTER STORM EVENTS. CONSTRUCTION NOTES 1. ALL CONSTRUCTION, INCLUDING ANY PROPOSED LANDSCAPING, SHALL CONFORM TO THE CURRENT EDITION OF THE FAIRFAX COUNT PFM AND THE VIRGINIA DEPARTMENT OF TRANSPORTATION (V.D.O.T.) STANDARDS AND SPECIFICATIONS AND SHALL CONFORM TO TH CURRENT EDITION OF THE VIRGINIA UNIFORM STATEWIDE BUILDING CODE. 2. ALL CONSTRUCTION WITHIN THE V.D.O.T. RIGHT-OF-WAY SHALL CONFORM TO THE PROVISIONS CONTAINED IN THE V.D.O.T. LAND US PERMIT ISSUED FOR THIS LOCATION. THE CONTRACTOR SHALL BE THOROUGHLY FAMILIARIZED WITH THE REQUIREMENTS OF THIS LAND USE PERMIT PRIOR TO THE START OF ANY CONSTRUCTION IN V.D.O.T. RIGHT-OF-WAY. THE SPECIAL PROVISIONS IN FORM MI ARE A PART OF THE VDOT LAND USE PERMIT.

- 3. UNLESS MORE STRINGENT COMPACTION REQUIREMENTS ARE INDICATED ON THE PLANS OR IN THE SPECIFICATIONS, THE BACKFILL O EMBANKMENT MATERIAL, THE INSTALLATION OF TRENCH BACKFILL AND THE RESTORATION OF DISTURBED AREAS SHALL BE COMPAC IN ACCORDANCE WITH THE CURRENT EDITION OF THE V.D.O.T. ROAD AND BRIDGE SPECIFICATIONS. ALL COMPACTION SHALL BE AT OF THE OPTIMUM MOISTURE CONTENT.
- 4. ALL SUBGRADE, SUBBASE, BASE AND SHOULDER MATERIAL SHALL BE PLACED AND COMPACTED TO THE DENSITY SPECIFIED IN THE CURRENT EDITION OF THE V.D.O.T. ROAD AND BRIDGE SPECIFICATIONS. ALL COMPACTION SHALL BE AT ±2% OF THE OPTIMUM MOIS CONTENT.
- 5. THE CONTRACTOR SHALL PROVIDE ADEQUATE MEANS OF CLEANING TRUCKS AND/OR OTHER EQUIPMENT OF MUD PRIOR TO ENTERIN THE V.D.O.T. RIGHT-OF-WAY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CLEAN ALL STREETS, ALLAY DUST AND TO TAKE WHATEVER MEASURES NECESSARY TO ENSURE THE ROAD(S) ARE MAINTAINED IN A CLEAN, MUD AND DUST FREE CONDITION AT AL
- 6. TEMPORARY OR PERMANENT PAVEMENT PATCHES ARE TO BE PLACED IN ALL ROADWAY CUTS WITH HOT MIX THE SAME DAY THE (MADE IN ACCORDANCE WITH V.D.O.T. REQUIREMENTS. SEE SPECIAL PROVISIONS FOR PAVEMENT OPEN CUTS, FORM LUP-OC NOVA FOR
- 7. THE TOP ELEVATION OF EXISTING MANHOLES SHALL BE ADJUSTED TO MEET THE FINAL PAVEMENT ELEVATION AT THE TIME OF FINAL OPERATIONS. ALL MANHOLES ARE TO BE PROTECTED FROM THE TRAVELING PUBLIC. NO MANHOLE IS TO BE RAISED ABOVE THE OF THE ROADWAY WITHOUT THE APPROPRIATE TRANSITION.
- 8. CONTRACTOR IS TO ADJUST MANHOLE, VALVE, AND METER BOX COVERS BEFORE PLACING FINAL SURFACE PAVEMENT.
- 9. THE CONTRACTOR SHALL VISIT THE SITE AND SHALL VERIFY EXISTING CONDITIONS PRIOR TO SUBMITTING A BID FOR THE CONSTRUCTION OF THE PROJECT.
- 10. WHERE EXISTING NATURAL DRAINAGE DITCHES OR STREAM BANKS ARE DISTURBED DURING CONSTRUCTION THE CONTRACTOR SHALL RESTORE THESE AREAS TO ORIGINAL ALIGNMENT, GRADE AND INVERT.
- 11. PROPOSED TOP OF CURB GRADES SHALL BE FIELD ADJUSTED AS REQUIRED TO CONFORM TO THE INTENT OF THE TYPICAL SECTION A SMOOTH GRADE SHALL BE MAINTAINED FROM THE CENTERLINE TO THE PROPOSED EDGE OF PAVEMENT OR FACE OF CURB TO PR THE FORMING OF FALSE GUTTERS AND/OR THE PONDING OF WATER ON THE ROADWAY. THE EXISTING PAVEMENT SHALL BE RECAP AND/OR REMOVED AND REPLACED AS REQUIRED TO ACCOMPLISH THIS REQUIREMENT. ALL CURB FORMS SHALL BE INSPECTED FOR HORIZONTAL AND VERTICAL ALIGNMENT BY FAIRFAX COUNTY OR THEIR AUTHORIZED REPRESENTATIVES PRIOR TO PLACING OF CONCI
- 12. THE FOLLOWING PROVISIONS SHALL APPLY TO THE USE OF SHEETING AND SHORING: (A) SHEETING AND SHORING OR OTHER APPROVED METHODS FOR TRENCH BRACING WILL BE REQUIRED ON THIS CONTRACT AS NE TO MEET ALL SAFETY REQUIREMENTS.
 - UNLESS OTHERWISE DIRECTED BY THE ENGINEER, SHEETING AND SHORING WILL BE REMOVED FROM ALL TRENCHES PRIOR TO (B) BACKFILLING OPERATIONS.
 - (C) UNLESS SPECIFICALLY IDENTIFIED IN THE CONTRACT DOCUMENTS, NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR SHEE AND SHORING.

	GENERAL NOTES	1	
(THESE GEN	ERAL NOTES SHALL BE USED WHERE THEY ARE APPLICABLE TO THE F		
	<u>CONSTRUCTION NOTES (CONTINUED)</u>		CONSTRUCTION NOTES (CONTINUED)
	 13. THE CONTRACTOR SHALL RESTORE ALL DRIVEWAYS DISTURBED DURING CONSTRUCTION. RES SHALL CONSIST OF THE FOLLOWING: * GRAVEL DRIVEWAY – PROVIDE MINIMUM 6" COMPACTED 21A 	STORATION	STORM SEWER 1. A WATERTIGHT CONNECTION SHALL BE MADE AT ALL PIPES ENTERING DRAINAGE STRUCTURES. IN ADDITION, WATERTIGHT CONNECTIONS SHALL BE MADE BETWEEN SECTIONS OF PIPE.
	* ASPHALT DRIVEWAY – PROVIDE 6" COMPACTED 21A WITH MINIMUM 2" SM-9. * CONCRETE DRIVEWAY – PROVIDE A NEAT SAWCUT CONNECTION, MINIMUM 4"	COMPACTED	2. LENGTHS OF PIPE SHOWN ON THE DRAWINGS ARE MEASURED FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE.
	 21A AND 5" CLASS A3 CONCRETE WITH WWF 6X6 – 14. WHERE A PROPOSED PIPE CROSSES OR PARALLELS A STREET, THE ASPHALT SHALL BE NEA SAWCUT TO FULL DEPTH. AFTER INSTALLATION OF THE PIPE, THE ROADWAY SHALL BE PATE 	ATLY	3. INVERT ELEVATIONS SHOWN ON THE DRAWINGS ARE TAKEN AT THE FACE OF THE STRUCTURE.
	ACCORDANCE WITH THE V.D.O.T. LAND USE PERMIT ISSUED FOR THE PROJECT. 15. HORIZONTAL LOCATION OF THE SERVICE CONNECTIONS AT SANITARY SEWER MAINS, IF INDIC.		 SHAPE THE INVERTS OF ALL STRUCTURES ACCORDING TO V.D.O.T. STANDARD IS-1. STORM SEWER AND CULVERT PIPE SHALL BE REINFORCED CONCRETE PIPE TO CONFORM TO THE CURPENT A A SHIT OF DESIGNATION, M170, UNITED TO DESIGNATED ON THE DEALS.
	ON THE CONSTRUCTION PLANS, WERE TAKEN FROM TELEVISION INSPECTION REPORTS. ALL CONNECTIONS WERE SHOWN TO BE IN THE UPPER TWO QUADRANTS OF THE MAINS. EXCEP	SERVICE	CURRENT A.A.S.H.T.O. DESIGNATION M170, UNLESS OTHERWISE DESIGNATED ON THE PLANS. CLASS III PIPE WILL BE REQUIRED WITHIN THE LIMITS OF V.D.O.T. RIGHT—OF—WAY, UNLESS OTHERWISE NOTED.
	INTERPOLATION OF THE INVERT AT THE SANITARY MAINS, THE ELEVATIONS AND VERTICAL LOCATIONS OF THE SERVICE CONNECTIONS ARE NOT KNOWN. IN SEWER MAINS AS NOTED, TELEVISION INSPECTIONS DID NOT DETECT ANY SERVICE CONNECTIONS AND THE LOCATIONS ON THE DRAWING ARE THE MOST PROBABLE POSITIONS FOR SERVICE CONNECTIONS.	SHOWN	6. MINOR FIELD ADJUSTMENTS IN THE ELEVATION AND ALIGNMENT OF THE STORM SEWER AND STRUCTURE MIGHT BE NECESSARY TO MEET EXISTING CONDITIONS AND PROPOSED FINAL GRADING. THE CONTRACTOR SHALL NOTIFY FAIRFAX COUNTY DEPARTMENT OF PUBLIC WORKS AND ENVIRONMENTAL SERVICES PRIOR TO MAKING ANY NECESSARY ADJUSTMENTS.
	16. THE PAVEMENT DESIGN IN THE CONSTRUCTION PLANS WAS PREPARED BASED ON AVAILABLE INFORMATION INCLUDING LABORATORY CALIFORNIA BEARING RATIO (C.B.R.) TESTS THAT WER DURING THE DESIGN SUBSURFACE INVESTIGATION. WHEN THE CONTRACTOR REACHES THE SU	E PERFORMED	7. TOP OF STRUCTURES SHALL BE SET TO MATCH CURB AND GUTTER, SIDEWALK AND/OR DITCH CONSTRUCTION.
	ELEVATION, ADDITIONAL CBR TEST LOCATIONS MAY BE DIRECTED BY THE ENGINEER FOR SEL BASED ON ACTUAL FIELD CONDITIONS OBSERVED. THE CBR SAMPLES AND TESTS SHALL BE A GEOTECHNICAL ENGINEERING FIRM UNDER CONTRACT WITH FAIRFAX COUNTY. THE RESULTS EVALUATION OF THE CBR LABORATORY TESTS SHALL BE OBTAINED IN WRITING PRIOR TO TH	ECTED LOCATIONS PERFORMED BY S OF THE ENGINEER'S IE PLACEMENT	 A VIDEO INSPECTION OF ALL NEW OR MODIFIED STORM SEWERS IN VDOT RIGHT OF WAY WILL BE REQUIRED NO MORE THAN 60 DAYS IN ADVANCE OF PROJECT ACCEPTANCE BY V.D.O.T. ALL STORMSEWER PIPES WITHIN THE PROJECT LIMITS WILL BE FLUSHED.
	OF ANY SUBBASE OR BASE MATERIAL IN THE AREA(S) UNDER CONSIDERATION. THE PROPOS PAVEMENT DESIGN FOR THE AREA(S) UNDER CONSIDERATION WILL EITHER BE CONFIRMED OR ENGINEER BASED ON THE RESULT OF THE C.B.R. TEST RESULTS. THE CONTRACTOR SHALL C	ADJUSTED BY THE	SIDEWALKS AND TRAILS
	BY MODIFYING CONSTRUCTION ACTIVITIES AND/OR SCHEDULING IN ORDER TO PERMIT THE AD THE CONTRACTOR SHALL NOT BE ENTITLED TO ANY MONETARY DAMAGES WHATSOEVER FOR FROM THIS TESTING. THE CONTRACTOR'S SOLE RELIEF IS A TIME EXTENSION GRANTED IN AC ARTICLE 8.3.	ANY DELAYS RESULTING	 A MINIMUM OF 2 FOOT CLEARANCE IS REQUIRED BETWEEN THE WALKWAY EDGE AND ANY VERTICAL OBSTRUCTIONS SUCH AS TREES, UTILITY POLES, SIGNS, ETC., UNLESS OTHERWISE SPECIFICALLY NOTED ON THE PLANS AT EACH LOCATION.
Y	TRANSPORTATION MANAGEMENT PLAN		2. A 4 FOOT UTILITY STRIP IS REQUIRED BETWEEN THE EDGE OF THE WALKWAY AND THE BACK OF THE CURB AND GUTTER, UNLESS OTHERWISE NOTED ON THE PLAN.
ΗE	1. THE CONTRACTOR SHALL BE REQUIRED TO SUBMIT CHANGES TO APPROVED TRANSPORTATIC PLANS FOR APPROVAL BY V.D.O.T. A MINIMUM OF 30 WORKING DAYS PRIOR TO BEGINNING		 A MINIMUM 10 FOOT SEPARATION IS REQUIRED BETWEEN THE EDGE OF THE SIDEWALK AND THE EDGE OF THE PAVEMENT FOR NON CURB AND GUTTER ROADS, UNLESS OTHERWISE NOTED ON THE PLANS.
RENCH,	 THE CONTRACTOR SHALL BE RESPONSIBLE FOR DAILY TRAFFIC CONTROL SUCH AS LANE CL DRIVEWAY ENTRANCES, ETC., TO PROPERLY MAINTAIN TRAFFIC THROUGHOUT THE PROJECT. EXISTING SUBFACE DASE AND SUBBASE MATERIAL WITCH WITCH WITCH DE DEMONSTREE OF OPPOTET. 		4. ALL HANDRAIL MATERIAL AND INSTALLATION SHALL CONFORM TO THE CURRENT VDOT ROAD AND BRIDGE STANDARDS 601.05 <u>STANDARD HANDRAIL METHOD OF LOCATING AND ERECTING</u> .
	 EXISTING SURFACE, BASE, AND SUBBASE MATERIAL WHICH WILL BE DEMOLISHED OR OBLITED CONSTRUCTION AND WHICH IS DETERMINED SUITABLE BY THE ENGINEER, SHALL BE SALVAGE FOR TRAFFIC MAINTENANCE. 		THE FOUNDATION DETAIL FOR HANDRAILS SHALL BE SPECIFIED IN THE CONSTRUCTION DRAWINGS. 5. ALL VEGETATIVE MATERIAL WITHIN 10 FEET OF VERTICAL CLEARANCE FROM THE PROPOSED TRAIL SHALL BE REMOVED PRIOR TO TRAIL CONSTRUCTION. THE HORIZONTAL CLEARING LIMITS SHALL
СН	4. ALL REQUIRED CONSTRUCTION SIGNING, TEMPORARY PAVEMENT WIDENING, TEMPORARY LAND TRAFFIC BARRIERS, TEMPORARY PAVEMENT MARKINGS, ERADICATION, ETC., SHALL BE INCLU CONTRACTOR'S BID PRICE FOR TRAFFIC MAINTENANCE.	DED IN THE	BE IN ACCORDANCE WITH THE DETAIL ON THE DETAIL SHEET. AN EXCEPTION TO THIS REQUIREMENT SHALL BE TO SAVE ANY TREES THAT ARE DESIGNATED ON THE CONSTRUCTION PLANS. 6. THE GRADES FOR PROPOSED SIDEWALK SHALL BE IN ACCORDANCE WITH CONSTRUCTION PLANS.
	 ALL TRAFFIC MAINTENANCE SHALL CONFORM WITH THE FOLLOWING AND THE LATEST REVISION WORK AREA PROTECTION MANUAL, VA ROAD AND BRIDGE SPECIFICATIONS, VA ROAD AND E FHWA MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES. ALL EXISTING SIGNS SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION. 		ANY FIELD ADJUSTMENT OF THE PROPOSED SIDEWALK GRADES SHALL BE APPROVED BY THE ENGINEER. THE FOLLOWING GRADE CRITERIA SHALL BE ADHERED TO: * MINIMUM: 1% EXCEPT IN SAGS WITH ADEQUATE DRAINAGE * MAXIMUM: ROADWAY GRADE OR 5%, WHICHEVER IS GREATER.
AND	7. CONTRACTOR SHALL MAINTAIN ACCESS TO ALL DRIVEWAY ENTRANCES DURING CONSTRUCTION		7. ALL DRIVEWAYS SHALL BE GRADED AS INDICATED ON THE CONSTRUCTION PLANS TO CONFORM TO ALLOWABLE CROSS SLOPES FOR THE WALKWAY. THE CROSS SLOPES FOR WALKWAYS SHALL BE
	 TEMPORARY CLOSURE AFFECTING EGRESS TO ADJACENT PROPERTIES SHALL BE COORDINATE PEDESTRIAN DETOUR SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION ACCORDING 		1/4" PER FOOT OF WIDTH. 8. THE MINIMUM ALLOWABLE TURNING RADIUS FOR WALKWAYS SHALL BE 20 FEET. ANY TURNING
Y	PROTECTION AND MUTCD GUIDELINES. 10. THE CONTRACTOR SHALL INFORM THE V.D.O.T. TRAFFIC OPERATIONS CENTER OF ANY LANE ON A DAILY BASIS BY CALLING 703-877-3449.	CLOSURES AND SUBSEQUENT OPEN	RADIUS LESS THAN 20 FEET, REQUIRED DUE TO FIELD ADJUSTMENTS, SHALL BE APPROVED BY INGS THE ENGINEER.
ŧΕ	SANITARY SEWER		9. ALL MATERIALS AND CONSTRUCTION METHODS SHALL CONFORM TO THE REQUIREMENTS FOR WALKWAYS IN THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.
SE 1P-63	1. ALL SANITARY SEWER IN THIS CONTRACT SHALL BE PVC SDR-35 UNLESS OTHERWISE NOT 2. LENGTHS OF PIPE SHOWN ON THE DRAWINGS ARE MEASURED FROM CENTER OF STRUCTURE		10. UNLESS MORE STRINGENT COMPACTION REQUIREMENTS ARE NOTED ON THE PLANS, THE SUBGRADE FOR ALL WALKWAYS SHALL BE COMPACTED TO A MINIMUM OF 95% OF THEORETICAL MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT WITHIN THE TOLERANCE SPECIFIED IN THE CURRENT
F CTED	CENTER OF STRUCTURE. 3. IF PVC PIPE IS USED, ALL LATERAL SPUR CONNECTIONS SHALL BE MADE WITH MANUFACTU	JRED	EDITION OF THE V.D.O.T. ROAD AND BRIDGE SPECIFICATIONS. 11. SHARED USE PATHS SHALL BE A MINIMUM OF 8.0 FEET FROM THE FACE OF CURB UNLESS OTHERWISE
±2%	PVC "TEE" OR "WYE" FITTINGS. NO SADDLES SHALL BE USED. 4. LATERAL SPURS SHALL EXTEND 12" INSIDE PROPERTY LINES. IF THE UPPER ENDS OF THE		NOTED ON THE PLAN. <u>TREE REMOVAL PROTECTION DURING CONSTRUCTION IN EASEMENTS</u>
STURE	LATERAL SPURS ARE GREATER THAN 6' BELOW THE GROUND SURFACE, THEN THE CONTRA SHALL INSTALL TWO 45' BENDS AND A VERTICAL RISER (ALL 4" DIAMETER) TO WITHIN 4' GROUND SURFACE. THE INVERT SHOWN IN THE PROFILE IS AT THE BOTTOM OF THE LOWER	OF THE	DENOTES AN EXISTING TREE NOT TO BE REMOVED OR DAMAGED. ALL REMAINING TREES WITHIN THE EASEMENT LIMITS CAN BE REMOVED AS REQUIRED TO PERFORM THE CONSTRUCTION. IN ADDITION, THE CONTRACTOR MUST REMOVE ANY TREES
IG	BEND. 5. AS-BUILT DRAWINGS FOR ALL NEW SANITARY SEWER INSTALLATIONS AND/OR ADJUSTMENT EXISTING SANITARY SEWER FACILITIES SHALL BE PREPARED IN ACCORDANCE WITH SECTION		ADVERSELY AFFECTED BY THE CONSTRUCTION TO THE EXTENT THAT IT IS LIKELY TO DIE IN THE OPINION OF THE COUNTY ARBORIST. HOWEVER, EVERY EFFORT WILL BE MADE TO AVOID THE REMOVAL OR DISTURBANCE OF REMAINING TREES.
L TIMES. CUT IS	OF THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL. 6. WHEN AN EXISTING SANITARY SEWER LINE IS REPLACED, ALL EXISTING SANITARY SEWER LA		DENOTES TREES TO HAVE TREE PROTECTION BARRICADE (SNOW FENCE) AT DRIP LINE AS PER ARTICLE 12 OF THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL.
OR DETAILS. AL PAVING TOP ELEVATION	SPUR LINES SHALL BE RECONNECTED TO NEW SANITARY SEWER. THESE SANITARY SEWER L SPUR LINE STREET TAPS SHALL BE PROVIDED STARTING AT A POINT 2-FEET TO 6-FEET F SEWER, OR IF NECESSARY, AT A LONGER DISTANCE TO ENSURE THAT THE EXISTING SANITA LATERAL PIPE IS IN GOOD CONDITION.	ATERAL TROM THE MAIN	DENOTES AN EXISTING TREE TO BE REMOVED. THIS INCLUDES THE REMOVAL AND / OR GRINDING OF STUMPS TO A MINIMUM OF 12" BELOW FINISHED GRADE.
	7. THE CONTRACTOR SHALL OBTAIN A FAIRFAX COUNTY PLUMBING PERMIT PRIOR TO THE CON ANY SANITARY SEWER SPUR LINE TO THE MAIN SANITARY SEWER.	INECTION OF	REVISED: APRIL. 2017 TAX MAP 29-3
		WEALTH OF	<u>EMERGENCY POLICE - FIRE - RESCUE 911</u> TOWN OF VIENNA, VIRGINIA
			DEPARTMENT OF PUBLIC WORKS 127 CENTER STREET S, VIENNA, VA. 22180
I. RECLUDE PPED		WELSCHENBACH Lic. No. 044359	DEPARTMENT OF PUBLIC WORKS 703-255-6380
RETE.	<u>For All excavation work ANYWHERE in Virginia!</u> <u>Always call 811 before you dig in Virginia!</u>	SIONAL ENG	OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS STANDARD FAIRFAX COUNTY GENERAL NOTES HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA
IEEDED		/ 0	GENERAL NOTES HUNTER MILL DISTRICT, FAIRF AX COUNTY, VIRGINIA
ETING		inker Design Associates, P.C. Manassas, Virginia PROFESSIONAL ENGINEER	DESCRIPTION BY APPROVED DATE SCALE HORIZ= N/A VERT= N/A DESIGNED BY: Fairfax County DRAFTED BY: Fairfax County CHECKED BY: Fairfax County SHEET

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UTILITIES NOTES

- THE UTILITY INFORMATION SHOWN ON THESE PLANS IS TAKEN FROM INFORMATION PROVIDED BY AN UNDERGROUND UTILITY DESIGNATING AND LOCATING COMPANY AND IN SOME CASES, FROM INFORMATION RECEIVED FROM THE UTILITY COMPANIES. THE DEPARTMENT OF PUBLIC WORKS AND ENVIRONMENTAL SERVICES DOES NOT GUARANTEE THAT THE UTILITY INFORMATION SHOWN ON THE PLANS IS COMPLETE OR ACCURATE. THE CONTRACTOR MUST VERIFY THE UTILITY LOCATIONS PRIOR TO CONSTRUCTION. 2. ALL EXISTING UNDERGROUND UTILITIES SHALL BE MARKED IN THE FIELD BY MISS UTILITY PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SCHEDULING THE FIELD MARKING OF UTILITIES WITH MISS UTILITY. 3. ALL EXISTING UNDERGROUND UTILITIES SHALL BE PHYSICALLY LOCATED BY THE CONTRACTOR PRIOR TO THE BEGINNING OF ANY CONSTRUCTION IN THE VICINITY OF THESE UTILITIES. 4. THE CONTRACTOR SHALL CONFORM TO THE PROVISIONS AS SPECIFIED IN THE CURRENT VIRGINIA ADMINISTRATIVE CODE (VAC) SECTION <u>20 VAC 5 – 309–140, EXCAVATOR'S RESPONSIBILITIES TO</u> <u>AVOID DAMAGE, DISLOCATING OR DISTURBANCE OF UTILITY LINES</u>, AS FOLLOWS: "ANY PERSON EXCAVATING AROUND UNDERGROUND UTILITY LINES SHALL TAKE ALL REASONABLE STEPS TO PROTECT SUCH UTILITY LINES. THESE STEPS SHALL INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING: 1. THE EXCAVATOR SHALL PLAN THE EXCAVATION IN SUCH A MANNER TO AVOID DAMAGE TO, AND MINIMIZE INTERFERENCE WITH, UNDERGROUND UTILITY LINES IN AND NEAR THE CONSTRUCTION AREA. 2. THE EXCAVATOR SHALL EXPOSE THE UNDERGROUND UTILITY LINE TO ITS EXTREMITIES BY HAND DIGGING WITHIN THE EXCAVATION AREA WHEN EXCAVATION IS EXPECTED TO COME WITHIN TWO FEET OF THE MARKED LOCATION OF THE UNDERGROUND UTILITY LINE. 3. THE EXCAVATOR SHALL NOT UTILIZE MECHANIZED EQUIPMENT WITHIN TWO FEET OF THE EXTREMITIES OF ALL EXPOSED UTILITY LINES. 4. THE EXCAVATOR SHALL MAINTAIN A REASONABLE CLEARANCE, TO INCLUDE THE WIDTH OF THE UTILITY LINE, IF KNOWN, PLUS 24 INCHES,
 - BETWEEN THE MARKED OR STAKED LOCATION OF AN UNDERGROUND UTILITY LINE AND THE CUTTING EDGE OR POINT OF ANY MECHANIZED EQUIPMENT, CONSIDERING THE KNOWN LIMIT OF CONTROL OF THE CUTTING EDGE OR POINT TO AVOID DAMAGE TO THE UTILITY LINE.
 - 5. THE EXCAVATOR SHALL PROVIDE PROPER SUPPORT FOR UNDERGROUND UTILITY LINES DURING EXCAVATION ACTIVITIES, DURING BACKFILL OPERATIONS, THE EXCAVATOR SHALL USE THE SAME OR SIMILAR BACKFILL MATERIAL THAT WAS ORIGINALLY AROUND THE UTILITY LINE, ENSURE THERE IS PROPER COMPACTION AROUND THE UTILITY LINE, PROTECT ALL TRACER WIRES, AND PROTECT OR REPLACE WARNING TAPES."
- 5. DISCONNECTED, PRIOR TO CLEARING THE SITE OF TREES, BUILDINGS, FOUNDATIONS, ETC. WITHIN THE LIMITS OF CONSTRUCTION IN ACCORDANCE WITH THE REQUIREMENTS INDICATED ON THE CONSTRUCTION PLANS.
- CONTRACTORS SHALL NOTIFY OPERATORS WHO MAINTAIN UNDERGROUND UTILITY LINES IN THE AREA OF PROPOSED CONSTRUCTION, EXCAVATION OR BLASTING AT LEAST 2 WORKING DAYS, BUT NOT MORE THAN 10 WORKING DAYS PRIOR TO COMMENCEMENT OF EXCAVATION OR DEMOLITION IN ACCORDANCE WITH CHAPTER 63 OF FAIRFAX COUNTY CODE. NAMES AND TELEPHONE NUMBERS OF SELECT OPERATORS OF UNDERGROUND UTILITY LINES IN FAIRFAX COUNTY APPEAR ON THIS SHEET. THESE NUMBERS WILL ALSO BE USED TO SERVE EMERGENCY CONDITION NOTICE AS REQUIRED BY CHAPTER 63 OF THE FAIRFAX COUNTY CODE. THIS IS NOT A COMPLETE LIST OF ALL UNDERGROUND UTILITY OPERATORS IN FAIRFAX COUNTY.

GENERAL NOTES 2: UTILIT

(THESE GENERAL NOTES SHALL BE USED WHERE THEY ARE APPLICABLE TO THE PROJECT PLANS)

MISS UTILITY: CALL 1-800-552-7001 OR 811 VA811.COM PRIMARY UTILITY COMPANIES

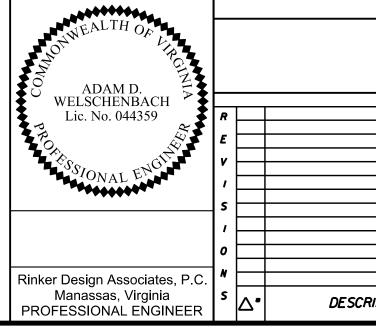
PRIMARY UTILITY COMPANIES

AMERICAN TELEPHONE & TELEGRAPH CO. (AT&T) CITY OF FALLS CHURCH PUBLIC UTILITIES COLUMBIA GAS OF VIRGINIA COLUMBIA GAS TRANSMISSION CORP. COLONIAL PIPELINE CO. COX COMMUNICATIONS DOMINION TRANSMISSION, INC. DOMINION VIRGINIA POWER FAIRFAX CO. WASTEWATER COLLECTION DIVISION FAIRFAX WATER FAIRFAX COUNTY PUBLIC SAFETY MCI, WORLD WORLD COM. (METROPOLITAN FIBER SYSTEMS) NORTHERN VIRGINIA ELEC. CO-OP (NOVEC) TRANSCO GAS PIPELINE CO. TOWN OF HERNDON PUBLIC WORKS TOWN OF VIENNA WATER SERVICE SPRINT (GLOBAL ONE) UPPER OCCOQUAN SÉRVICE AUTHORITY (UOSA) SMART TRAFFIC SIGNAL SYSTEM PLANTATION PIPE LINE COMPANY VERIZON VIENNA WATER VIRGINIA AMERICAN WATER COMPANY WASHINGTON GAS WILLIAMS (TRANSCO) GAS PIPE LINE CORP.

Underground VDOT Owned/Maintained Utilities EMERGENCY 1-800-241-3624 1. Underground Virginia Department of Transportation (VDOT) Owned and/or Maintained (703-248-5044 24/7AFTER HOURS/EMERGENCIES 703-698-5613 utilities may be present within the project limits. These utilities may include power, 1-800-543-8911 - (703) 631-5363 (METRO) 1-800-835-7191 (24 HRS) communication or other utilities related to street/pedestrian lighting, Intelligent 1-800-926-2728 Traffic System (ITS) devices such as Variable Message Signs (VMS), traffic signals (703) - 378 - 0882and other related facilities. 1-888-264-8240 24/7 1-866-366-4357 2.Fairfax County has attempted to show any known underground VDOT owned or (703) 323-1211 maintained utilities on the plans based upon the best available information at the (703) 698-5600 OR 698-5613 (703)-691-2131 OR 911 time of design. 1-800-624-9675 3. Miss Utility does not and will not mark the location of underground VDOT Owned (703) 852-6700 (703) 335-0500 and/or Maintained utilities within the project area. 1-800-440-8475 (24 HRS) 4. At least 48 business day hours in advance of any excavation, the Contractor shall (703) 435-6860 STATION 185 (703) 435-6846 (703) 255-6381 AFTER 5:00 PM., (703) 255-6385 be responsible for requesting that VDOT mark their underground utilities in the field. 1-800-521-0579 (24 HRS) All requests shall be submitted through the online ?Utility Marking System? by 703-830-2200 (703) - 383 - 2790registering at the following website: 1-800-510-5676 a. <u>http://www.vdotutilitymarking.virginia.gov/Account/Login.aspx?ReturnUrl=%2f</u> 1-800-837-4966 (703) - 255 - 63855. The 48 hour time limit does not begin until 7 AM the business day following receipt 1-800-452-6863 (703) 750-1000 (GAS LEAK (703) 750-4831) of the utility location request by VDOT. State holiday and weekends are not 1-800-440-8475 OR 703-368-3255 OFFICE considered to be business days. Underground VDOT utility location requests received after 4:30 PM shall be considered to be received by VDOT on the next business

marked.

day.



TRANSPORTATION PROJECTS/WORK WITHIN THE RIGHT-OF-WAY UTILITIES

6.No excavation shall commence until the underground VDOT utilities have been

7. The Contractor shall be responsible for repair or replacement of underground VDOT owned/maintained utilities that are damaged due to construction operations at no cost to the County.

Utility Relocation Plans

1. Fairfax County has coordinated the proposed improvements with the owners of all known underground utilities in the project area.

2. Underground Utility Test Hole information, noting the horizontal and vertical location of known underground utilities that are in potential conflict with the project, may be included in the plan set or provided to the Contractor at the Pre-Construction Conference, or upon request following award of the construction contract. 3. Where underground utilities are in conflict with the project, Fairfax County often attempts to relocate conflicting utilities prior to the commencement of construction activities. However, as with all construction activities, extreme care shall be taken by the contractor to ensure utility locations are known prior to excavation. 4. Miss Utility may not have the most recent up to date information concerning underground utilities that may have been recently relocated prior to construction. 5. Fairfax County may provide available plans showing the approximate location of recently relocated underground utilities to the Contactor at the Pre-Construction Conference, or upon request following award of the Construction Contract. Where practicable, the utility relocation plans may be included in the Bid Documents. 6. Any known underground utilities that are in conflict with the project that are scheduled to be relocated during construction shall be as noted on the plans and/or special provisions. The Contractor shall be responsible for coordinating these relocations with the appropriate utility company.

For All excavation work ANYWHERE in Virginia!

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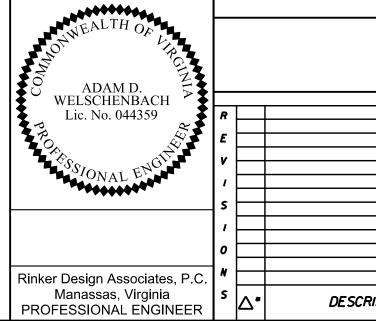
Affected Properties: Old Courthouse Road Pedestrian Access Improvements, Phase I; Project #: FFX 104325

	Property Information			Permanent Perpetual Street Easement		Permanent Storm Drainage Easement		Permanent Sidewalk Easement		Permanent Signal Equipment Easement		Permanent Sight Distance Easement		Permanent Retaining Wall Easement		Temporary Easement		Permanent Uitility Easement				
Tally	Plan Sheet	Plan Parcel *	Tax Map No.	Property Address	Owner	Street Esmt.	SF	Storm Esmt.	SF	SW Esmt.	SF	Sign Esmt.	SF	Sight Esmt.	SF	Ret.Wall Esmt.	SF	G & TCE SF	DVP	(SF) COX (S	F) Verizon (SF)	XO Com (SF) Other (S
	3		029-3-06-0048B 4	403 Old Courthouse Road N.E.	JANET B.ROOT													76	4			
	3		029-3-06-0048A	401 Old Courthouse Road N.E.	THOMAS BACKMAN													247	'4			
	3		029-3-06-0049	307 Old Courthouse Road N.E.	JOSE T.HERNANDEZ JR.LUZ A.HERNANDEZ													219	3			
	4		029-3-06-0059 3	305 Old Courthouse Road N.E.	SANTOSH K.KHANNA,SURINDER KHANNA													132	3			
	4		029-3-06-0058A 3	303 Old Courthouse Road N.E.	MATTHEW R.VERDEROSA													103	5			
	4		029-3-06-0057	301 Old Courthouse Road N.E.	CHRISTOPER M.GAFFNEY, RUYI LI													145	6			
	4		029-3-06-0035	241 Old Courthouse Road N.E.	LIU-HSIUNG CHUANG.PATRICIA L.CHUANG													176	57			
	4,5		029-3-06-0034 2	239 Old Courthouse Road N.E.	PAUL D. PAVNICA, SHARON L. PAVNICA													205	52			
	5,6		029-3-06-0033 2	237 Old Courthouse Road N.E.	PATRICK MILLER.VERONICA MILLER													222	23			
	5,6		029-3-06-0032 2	235 Old Courthouse Road N.E.	WALON N.HICKS													259	97			
	5.6		029-3-06-0031 2	233 Old Courthouse Road N.E.	DONNA SMITH. MOHAMMAD OUGACHE													226	3			

Affected Properties: Old Property Information Tally Plan Sheet Plan Parcel • Tax Map No. Property Address FAIRFAX COUNTY 029-3-01-0028 8530 Old Courthouse Road N.E. 5,6

Right of Way Data

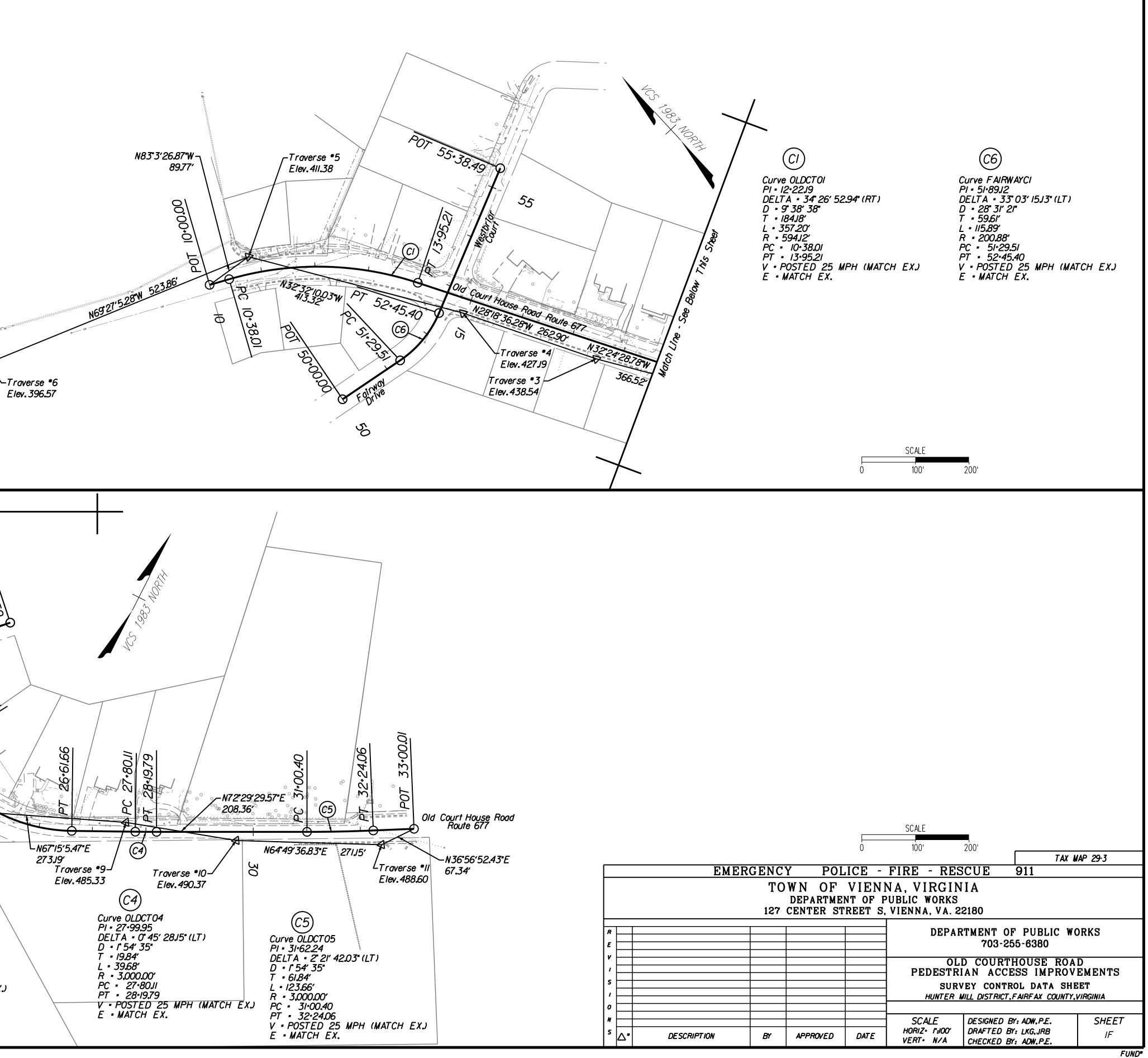
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Owner	Street Esmt.	SF	Storm Esmt.	SF	SW Esmt.	SF	Sign Esmt.	SF	Sight Esmt.	SF	Ret.Wall Esmt.	SF	G & TCE	SF	DVP (SF)	COX (SF)	/erizon (SF)	XO Com (SF)	Other (SF)
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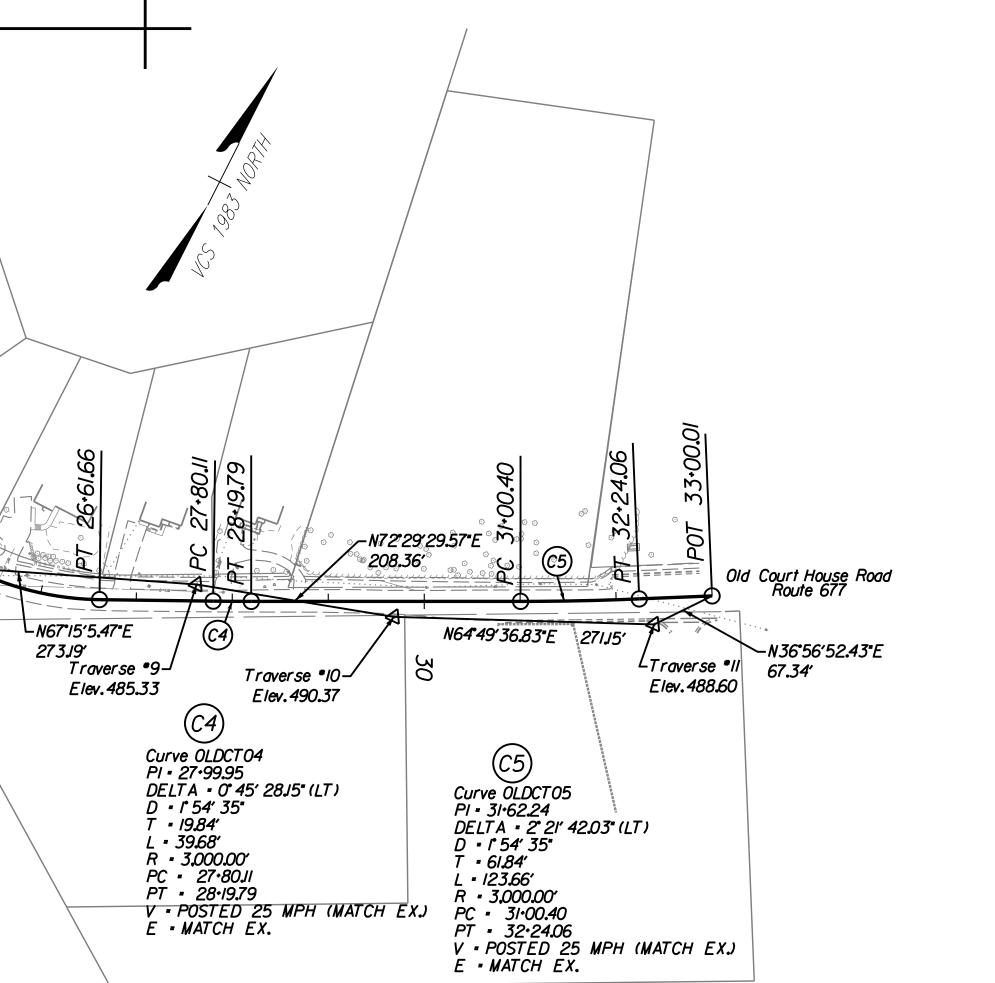


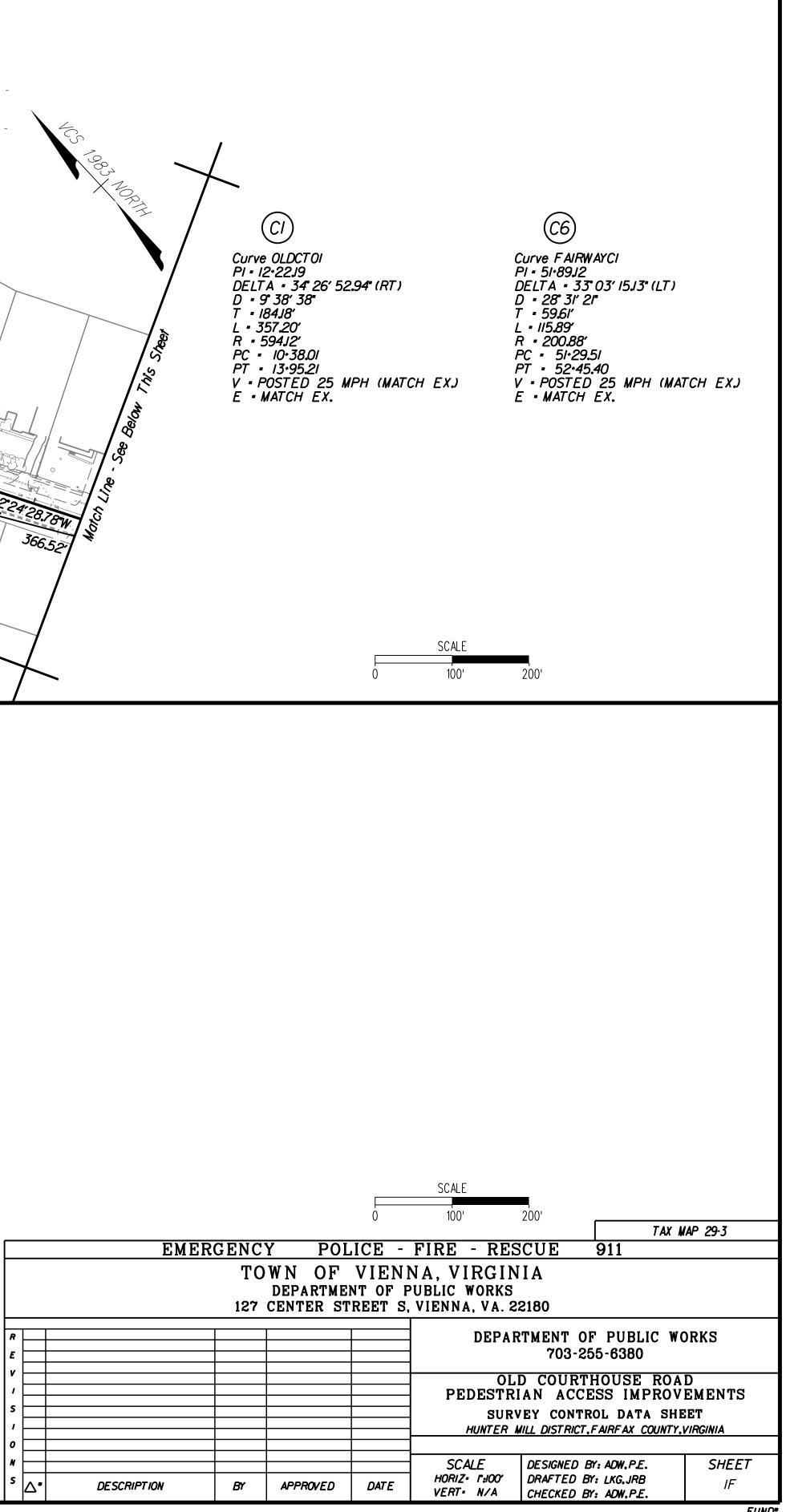
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	Trav 3	7,019,793,8147	ll , 839,839,3552	438.54			Iron Rod w∕ Cap	
	Trav 4	7,020,025.2689	11,839,714,6775	427,19	262.90'	N28°18′ 36 . 28°W	Iron Rod w/ Cap	
					413.32	N32°32′10.03″W	· · · ·	
	Trav 5	7,020,373.7229	II , 839 , 492 , 3789	411.38	523.86'	N69°27′5 .28 °W	Iron Rod w∕ Cap	
	Trav 6	7,020,557.5988	11,839,001,8476	396.57	525,00		Iron Rod w∕ Cap	
	T 7	7,000,050,04/6		705.05	561.47′	N57°31′41 . 78°W		
	Trav 7	7,020,859.0416	II , 838 , 528,I6I6	385.95	-	-	Iron Rod w/ Cap	
	Trav I	7,019,122,3194	II , 839 , 936.3738	439.48			Iron Rod w∕ Cap	
	Trav 8	7,0/9,308.9485	II , 840,379.0762	461.40	480.42'	N67*8′16 . 09*E	Iron Rod w/ Cap	
	1100 0	1,019,000,000		-01.40	273,19'	N67°15′5 . 47°E		
	Trav 9	7,019,414.5846	11,840,631,0178	485.33			Iron Rod w∕ Cap	
	Trav 10	7,019,477,2668	II , 840,829.7278	490.37	208.36'	N72*29′29 . 57*E	Iron Rod w/ Cap	
				150.51	271J5'	N64°49′36.83°E		
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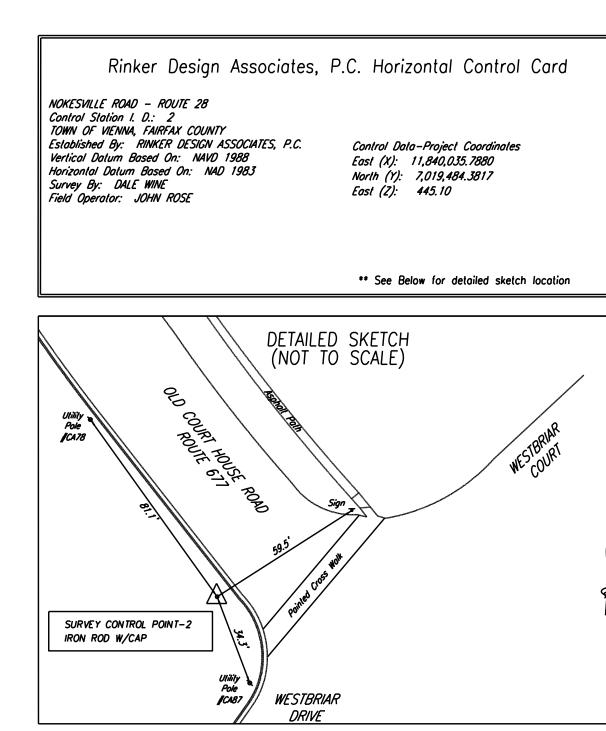


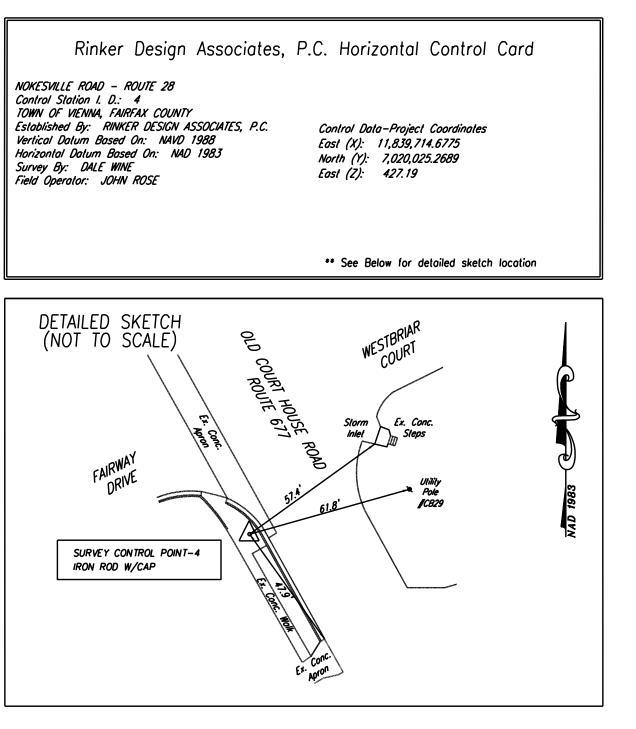


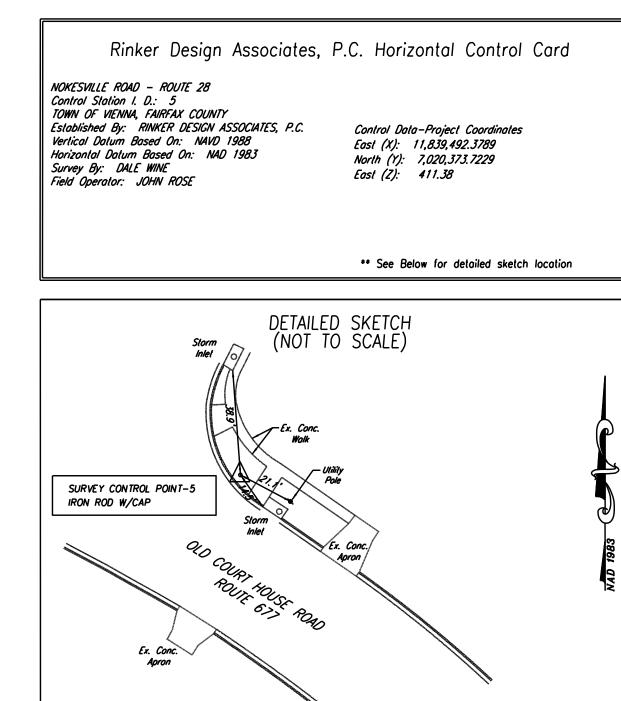


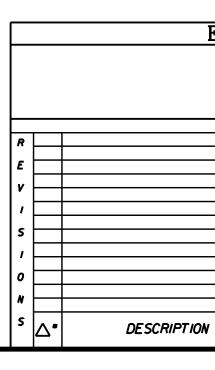
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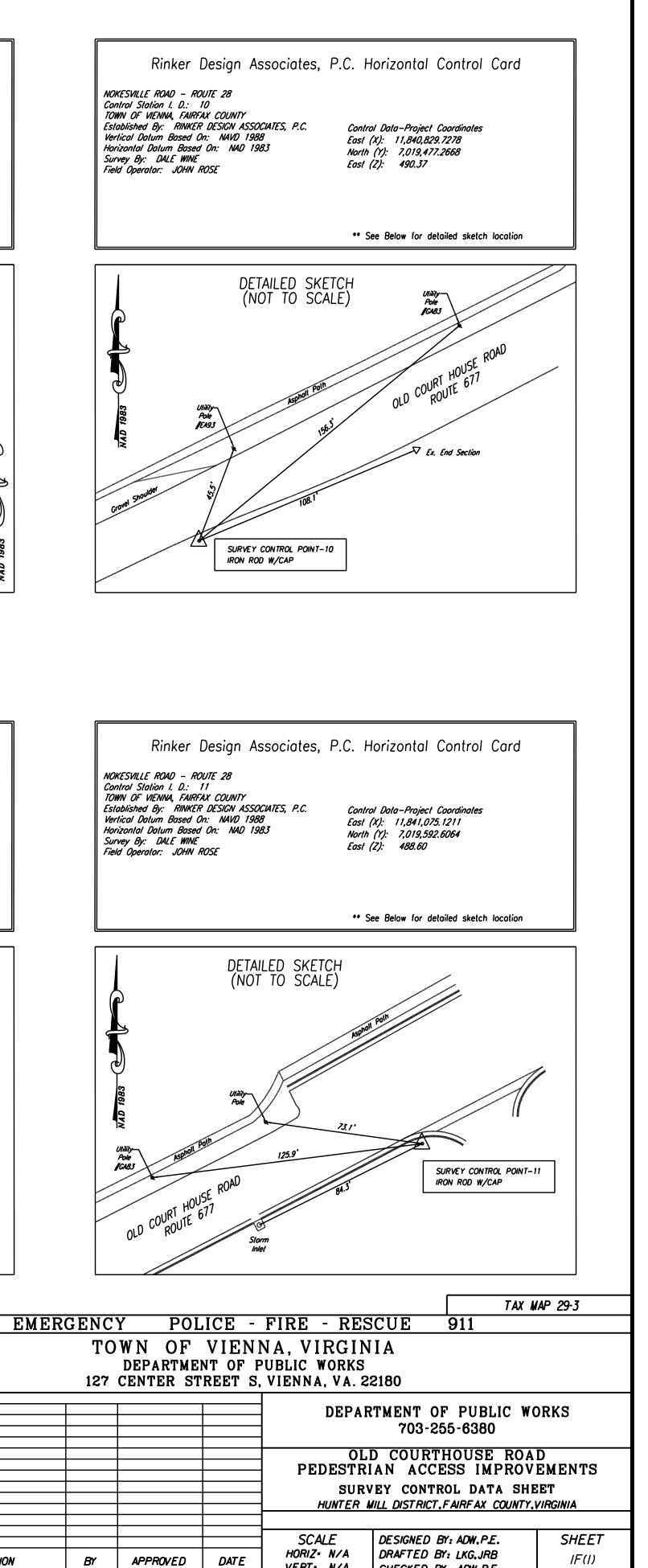
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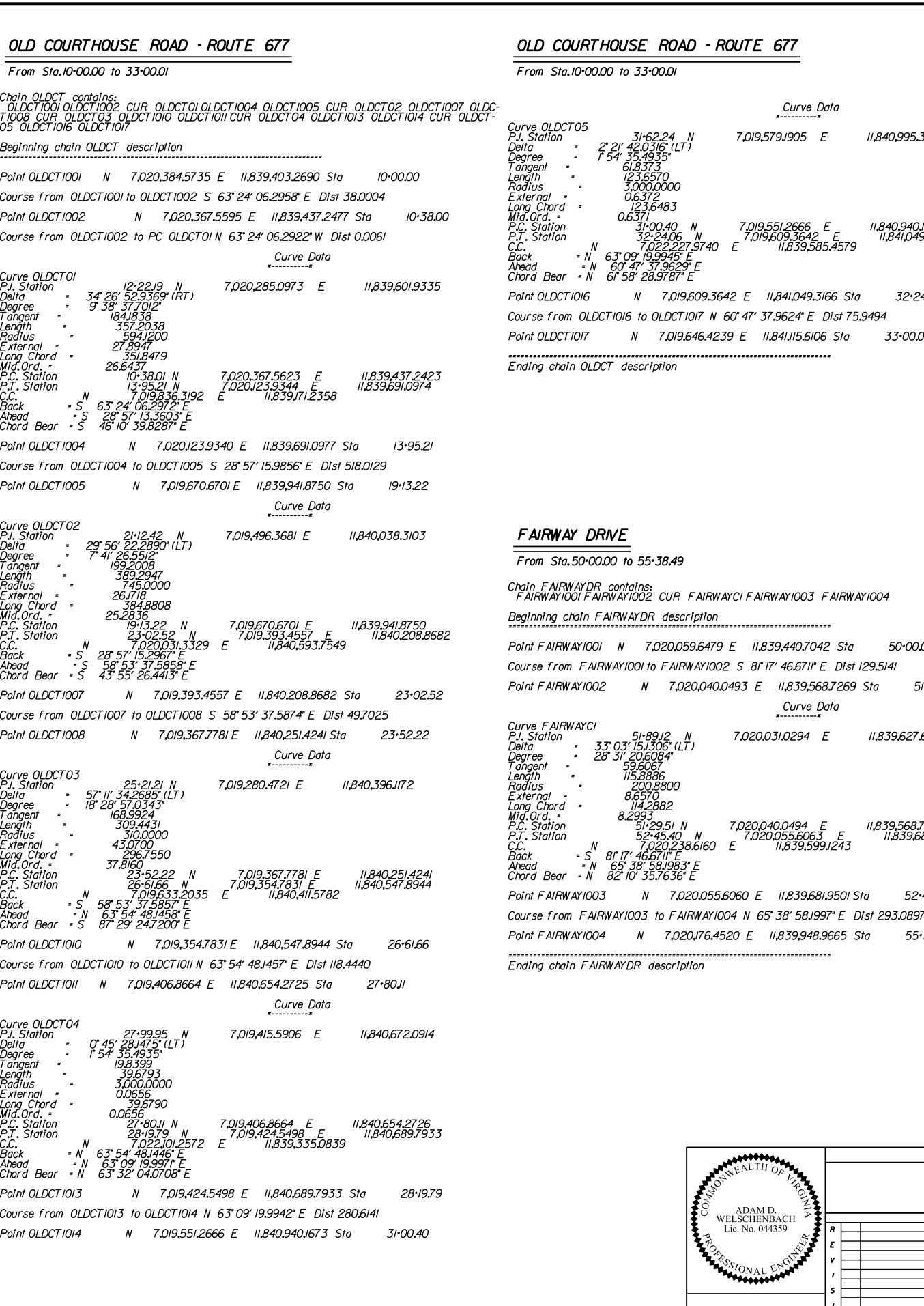
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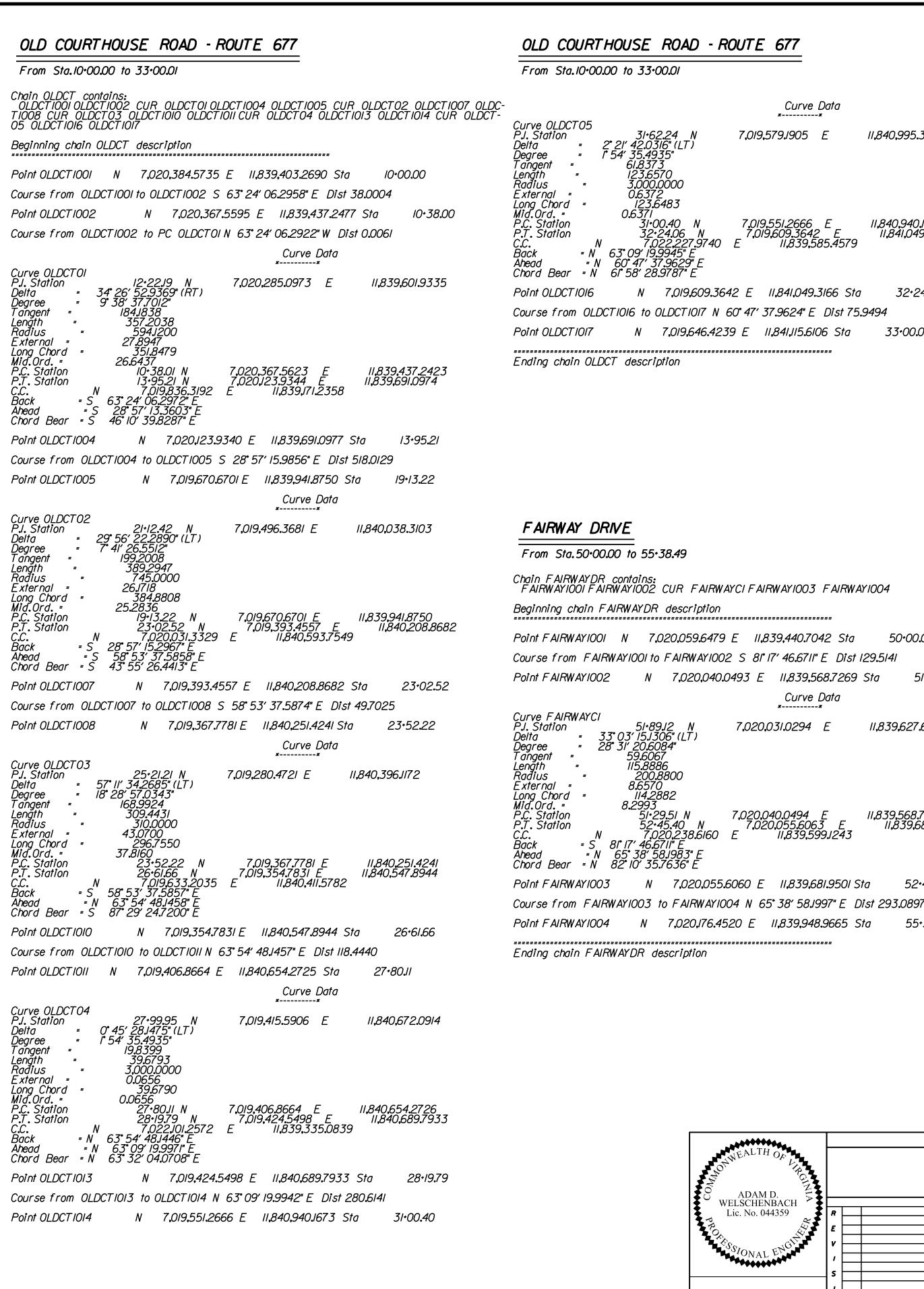
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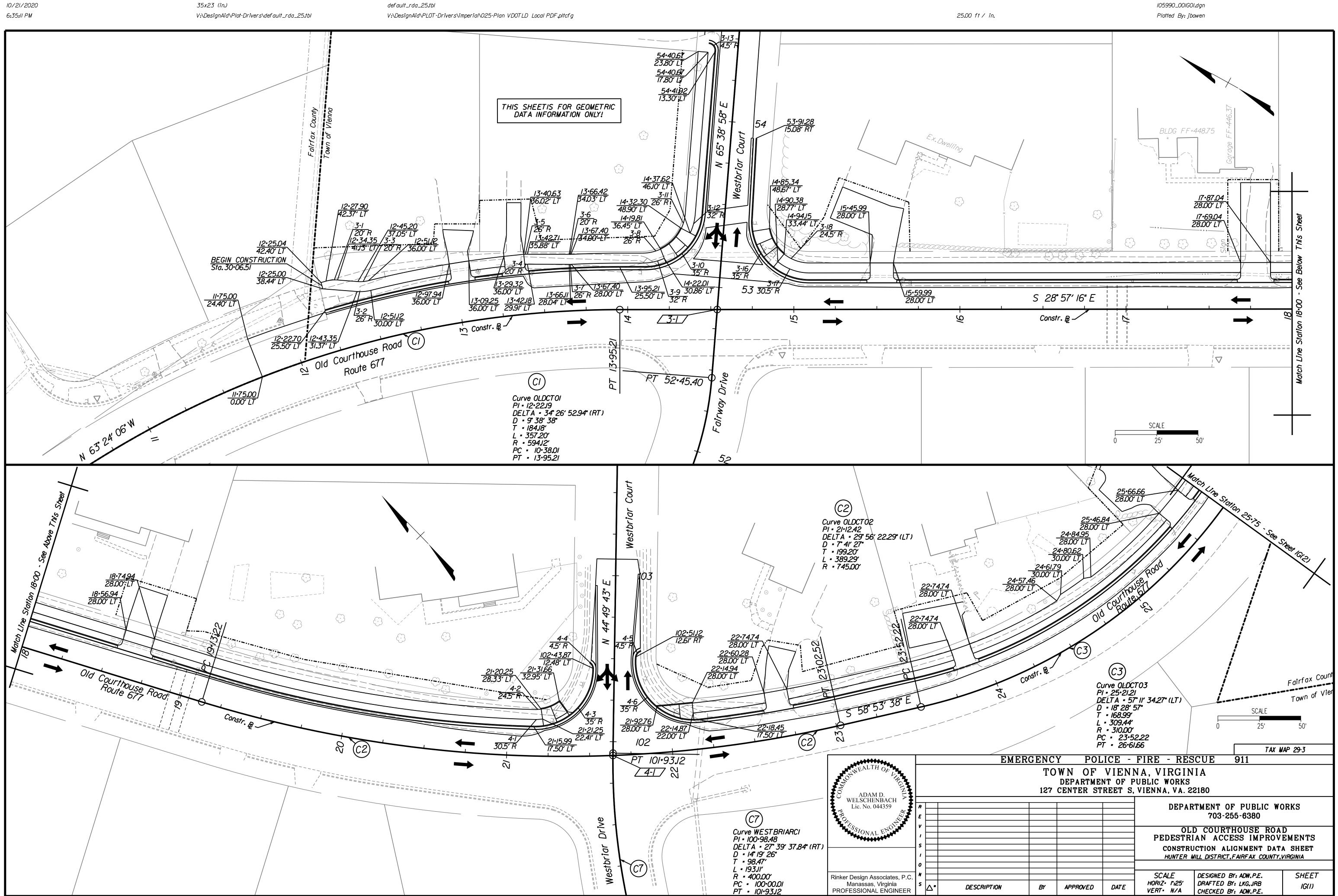


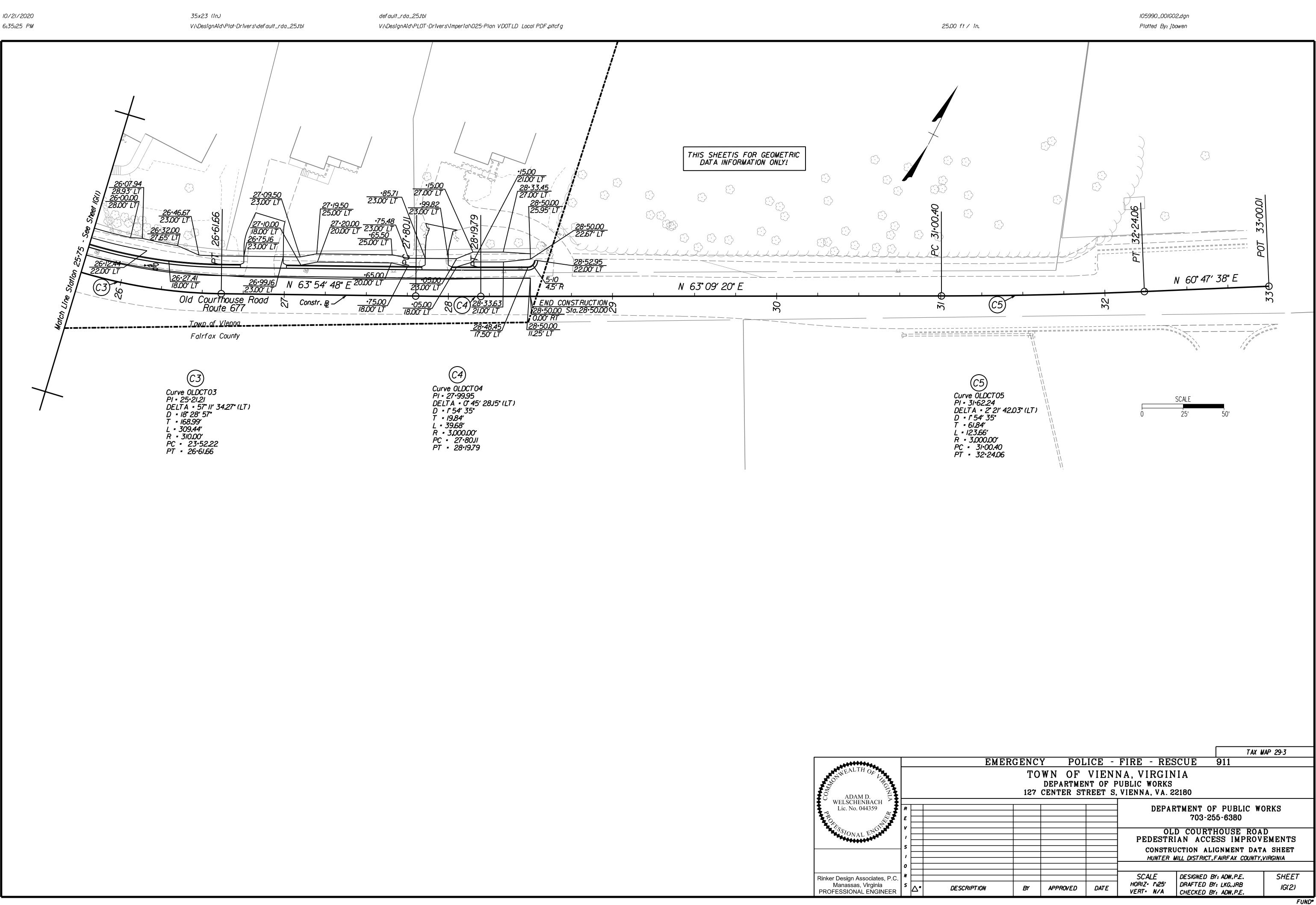


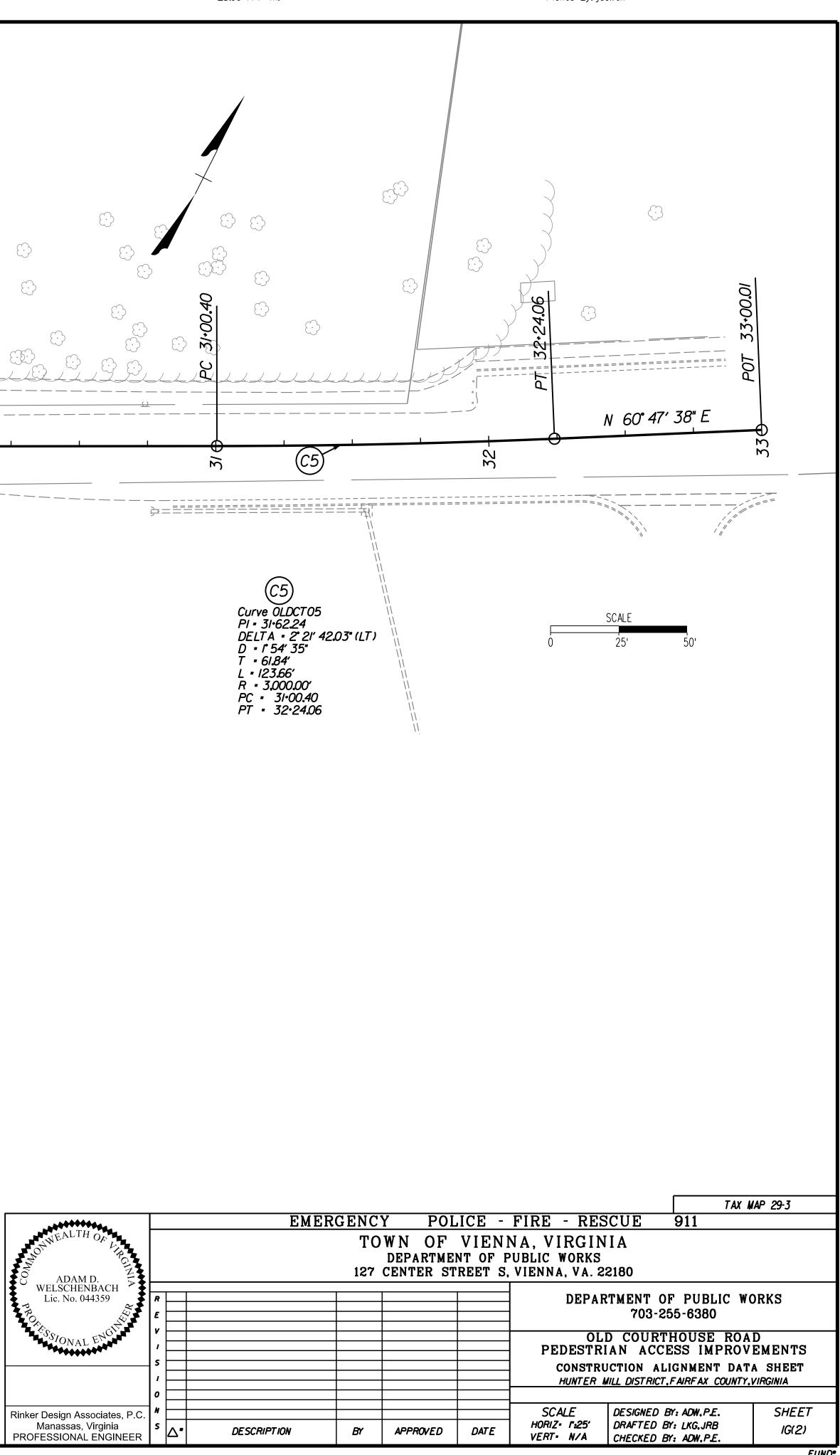
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DESCRIPTION	BY	APPROVED	DATE	HORIZ F:25' VERT N/A	DRAFTED BY: LKG.J. CHECKED BY: ADW.	RB	IG







General Notes:

- TMP/SOC Type A Project Information:
- a
- ldentify the project's TMP Type: This project's TMP/SOC plan has been designed in conformance with a Type A TMP/SOC plan. b
- Identify the work zone location, length, and widths: The project location is as shown on Sheet I. The work zone areas have been delineated as shown on the TMP/SOC plan sheets IK series. The work zone lengths and widths vary by location as shown on the TMP/SOC plan sheets IK series.
- Note the hours the Construction Area will be active: Construction Area shall be considered active when any impact to traffic occurs.(Ist Cone in Road).Construction Area hours have the following limitations: С

	SINGLE LANE CLOSURES									
	MONDAY TO THURSDAY	FRIDAY	SATURDAY	SUNDAY						
DAY TIME	9:30AM TO 3:00PM	9:30AM TO 2:00PM	-	-						
NIGHT TIME	10:00PM TO 5:00AM	10:00PM TO 9:00AM	-	-						

No lane closures will be allowed from noon on the day before a holiday until noon on the workday following the holiday. Holidays include all State and Federal holidays.

- The TMP/SOC plan,during construction,shall be in accordance with Sections 512,701,703 & 704 of the Virginia Department of Transportation Road and Bridge Specifications,dated 2007, the Virginia Work Area Protection Manual,dated June 2011,and the Manual on Uniform Traffic Control Devices (MUTCD),2009 Edition,including the 2011 Virginia Supplement to the MUTCD. d
- Note any existing entrances, existing intersections, or existing pedestrian access points that will be affected by the Construction Area or by the traffic control devices: е

Existing Entrances:

The following existing commercial or private entrances shall remain open for the duration of construction, except as noted. At the following locations are private or commercial entrances which shall have access maintained at all times: Approx.Sta: I3+03 LT, I3+35 LT, I5+55 LT, I7+22 LT, I8+66 LT, 22+67 LT, 23+86 LT, 25+56 LT, 26+86 LT, 27+92 LT

Existing Intersections:

There are multiple unsignalized intersections within the limits of this project, which are to remain operational for the duration of construction. These are the intersections of: Old Courthouse Road @ Westbriar Court (Sta. 14.53.88) Old Courthouse Road @ Westbriar Court (Sta. 21.64.05)

Existing Pedestrian Access Points:

There are existing sidewalks within the project limits.

f

Existing Bus Stops: There are no exisitn bus stops within the project limits.

ldentify the major types of travelers: The roadway carries large diverse types of travelers.In the peak hours however,residential vehicles are the prevailing traveler type for this roadway.

g	The Contractor, at no additional cost to the project and which shall be considered incidental to the cost of the project, shall:	8	Where Group 2 traffic.a minim
	Designate a person assigned to the project who will have the primary responsibility.with sufficient authority.for implementing the TMP/SOC and other safety and mobility aspects of the permit work.This person be in coordination with the Town Construction Inspector during all work.	9	The Contractor staging area.A 600 Mill Street.
	Ensure that personnel assigned to the project are trained in traffic control to a level commensurate with their responsibilities in accordance with VDOT's work zone traffic control training guidelines. All personnel performing flagging operation duties shall have completed/obtained	10	PUBLIC COMMU The Contractor
	flagger certification from VDOT. Any person performing flagger operation duties shall submit a copy of their certification prior to the start of the project, at no additional cost to the project. No flagging operation may be started until all certifications for each person performing the work is submitted for acceptance/approval.	а	Notifying the Pi work plans and
]	Inform the Engineer and Town Construction Inspector of any work requiring lane shifts, lane closures, and/or phase changes a minimum of two working days prior to implementing this activity.	b	Notifying the Pi unscheduled tra
-	Perform reviews of the Construction Area to ensure compliance with contract documents at regularly scheduled intervals at the direction of the Engineer and Town Construction Inspector.	С	Contractor shall the project.
	Contractor shall maintain a copy of the temporary traffic control plan at the work site at all times. Coordinate with Town of Vienna Police Department and Town of Vienna Fire/Rescue Department	//	TRANSPORTATI The Contractor
	for any lane closures and any detours of any nature at no additional cost to the project.	а	Post a list of lo
	Schedule all phases of construction in such a manner that water,sanitary sewer,cable,fiber cable/optic cable,any overhanging utilities,and any underground utilities services will not be interrupted.The Contractor is solely responsible for any interruption in any utility service,and solely	b c	Immediately repo Notify the proje
	responsible for any repairs to the approval of the impacted utility service.	Ū	incidents and e
2	This TMP/SOC plan is intended as a guide.It is not to enumerate every detail which must be considered in the construction of each phase, but only to show the general handling of existing traffic.It shall be the responsibility of the Contractor to present a formal TMP/SOC plan with construction signage to the Engineer and Town Construction Inspector for approval prior to any	d	Within 24 hours shall be complete any future incid
	construction activity that may affect the existing pedestrian or vehicular traffic.		CONTACT NUME
3	Contractor is to maintain at least one lane of traffic on Old Courthouse Road during construction of this project with a minimum clear roadway width in accordance with VDOT standard GS-10 unless otherwise approved by the Engineer.For street intersections,commercial connections,or private entrances,a minimum width no less than existing width shall be maintained at all times,unless approved by the Engineer.		Town Dept.Publ Town Construct Town Construct Emergency Call Non-Emergency
4	All areas excavated below the existing pavement surface and within the clear zone as prescribed in the VWAPM at the conclusion of each workday,shall be backfilled to form an approximate 6:1 wedge against the existing pavement or newly constructed pavement surface for the safety and protection of vehicular traffic.All costs for placing,maintaining and removing 6:1 wedge shall be included in the price bid for	Suggestee	Town Police Town Fire & R d Sequence of Co
	other items in the contract and no additional compensation will be allowed.	General P	hasing Notes:
5	Contractor shall follow the geotechnical recommendations for the project.Materials designated as unsuitable material as detailed in the geotechnical recommendations shall be disposed of offsite and are not to be used for any part of construction.Existing surface,aggregate base, and sub base material which will be demolished or obliterated during construction, and which are suitable for maintenance of traffic, should be utilized prior to the use of commercial material, subject to the approval of the Engineer.	MUTCD = I.The Con work to b	VDOT's current e FHWA's current tractor shall subm performed to b
6	Each phase of construction shall be completed to the installation of intermediate course asphalt prior to the start of the next phase unless otherwise directed by the Engineer.	queues a	y Traffic Control re deemed unacce
7	Contractor shall ensure positive drainage for the duration of the project.Contractor shall add any additional temporary measures necessary to facilitate proper, positive drainage	For the d	o the start of con luration of constr
	for the duration of construction.	attenuator	ntractor shall be r rs,flagging station FMP/SOC General
		PHASE I	V:(No Graphical P
		Summary:	The Contractor
			ntractor shall impl m mill & overlay (5.
		Operation	ontractor shall imp s on a Multi-Lane s on a Multi-Lane s.
		3. Contra construct	ctor is to maintai ion.
	ADAM D. WELSCHENBACH Lic. No. 044359		

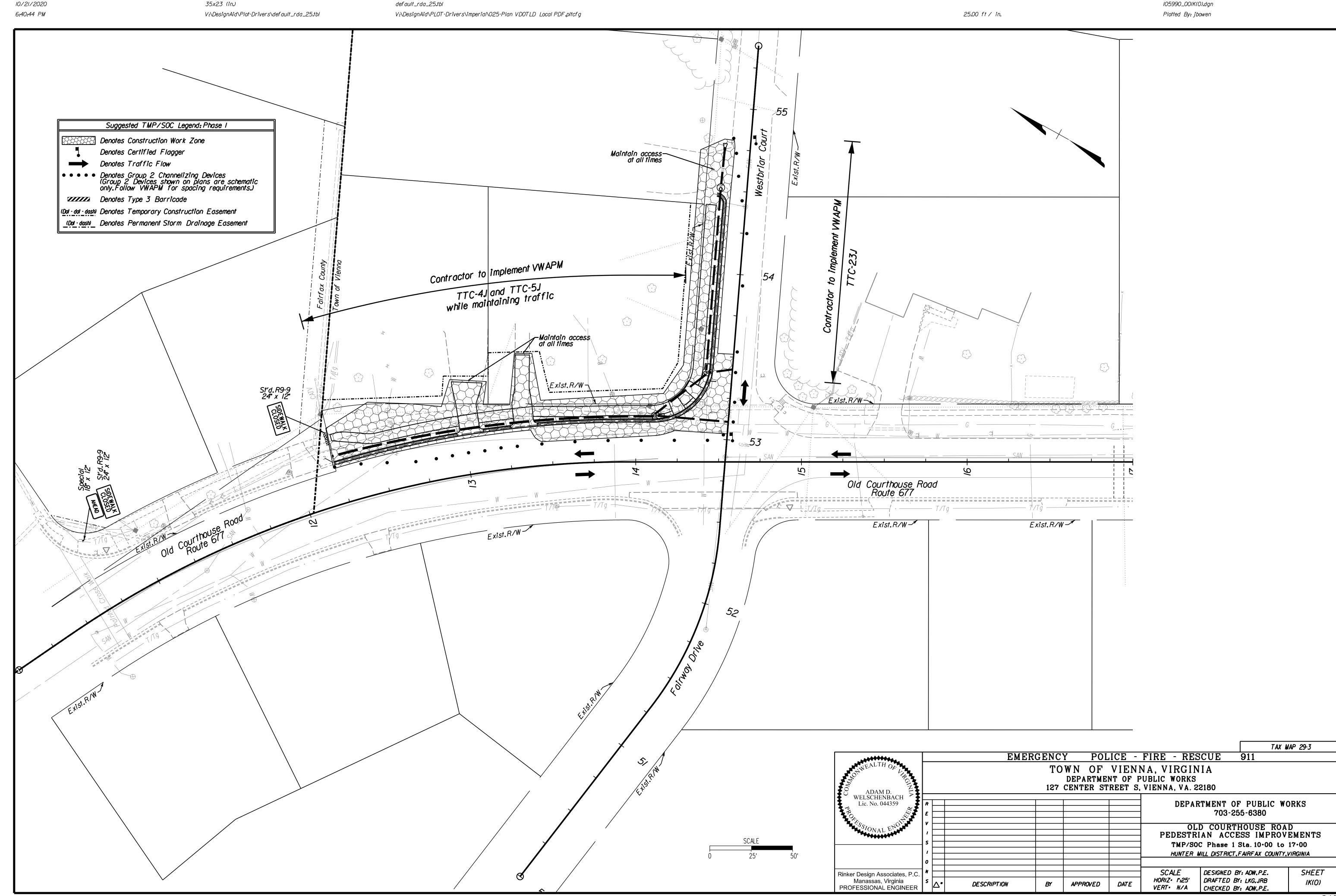
Rinker Design Associates, P.C.

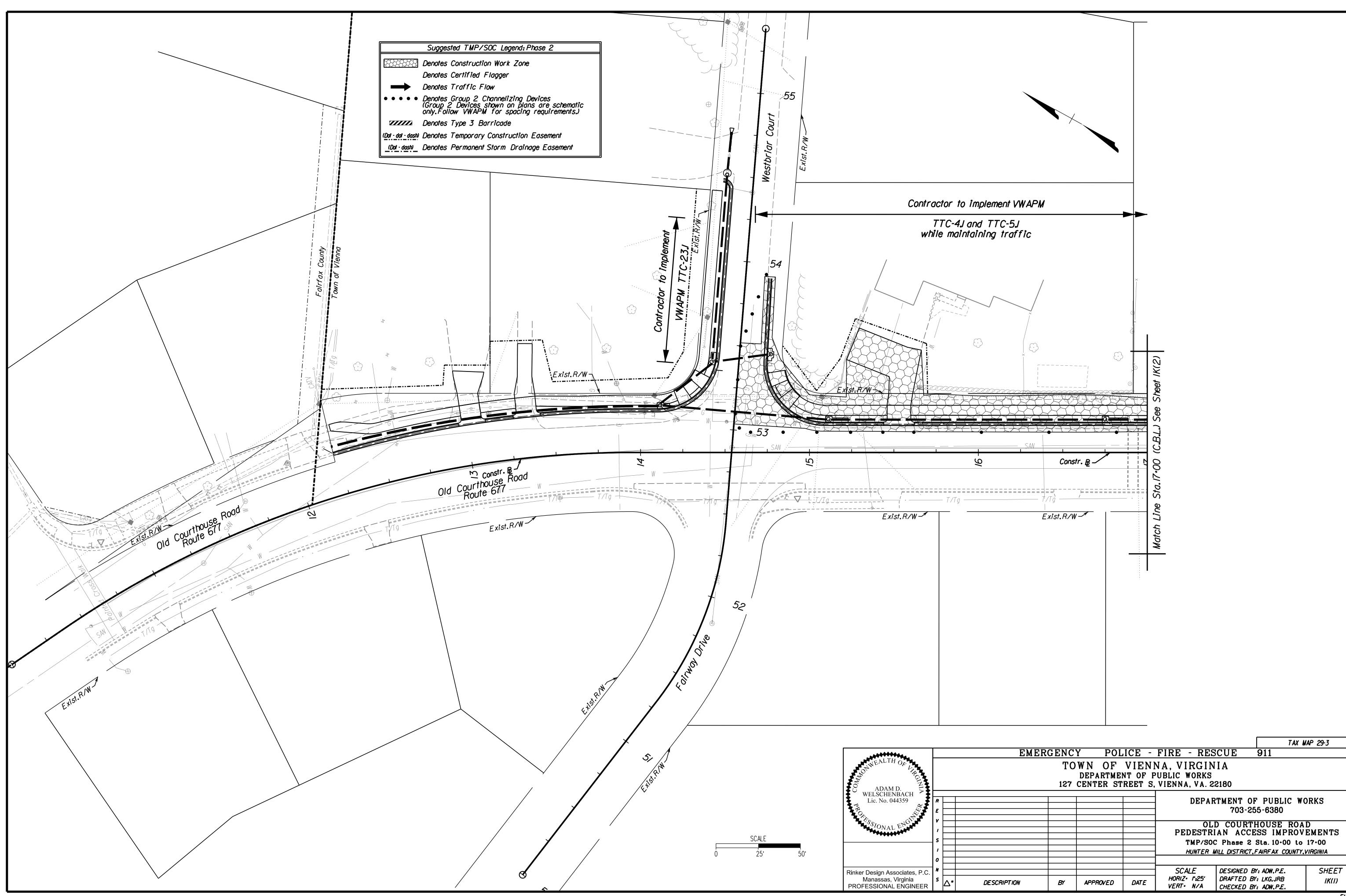
Manassas, Virginia PROFESSIONAL ENGINEER

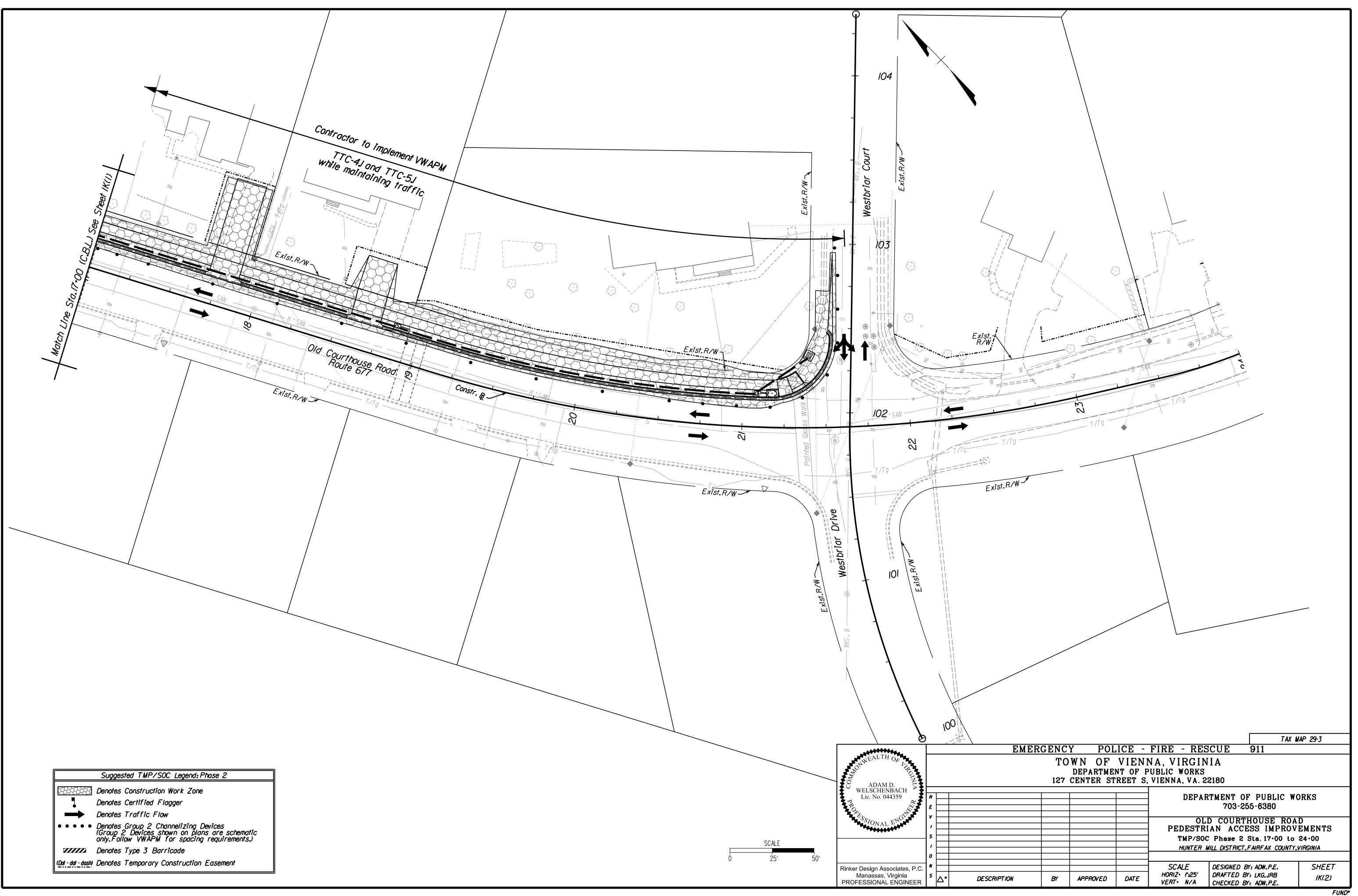
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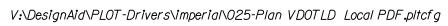
				e the Construction Area and APM is to be maintained.				
				location(s) of the construction o use is located at the Town Property at				
IMUNICATIONS for shall be res		for:						
e Project Manag and traffic del		Construction	Inspector	two weeks in advance of any scheduled				
Project Manager,Construction Inspector,and Town of Vienna of any traffic delays.								
nall attend any and all meetings requested by the Town of Vienna at no additional cost								
ATION OPERA for shall be res		for implement	ting and p	providing the following:				
f local emergen	ncy resp	onse agencies	inside the	e project's construction office/trailer.				
report any traft	fic incic	lents that may	occur in	the work zone.				
oject's Constru d expected tra			rown of V	lienna of any				
				work zone,a review of the traffic controls duce the frequency and severity of				
UMBERS								
Public Works Di fuction Inspecto fuction Manager	ors	Mike (TBD TBD 911	Gallagher,(703) 255-6380				
ncy Numbers:		(703) 2	255-6366					
Rescue		(703) 9	938-2242					
Construction:								
ent edition of th ent edition Man	ual on ັU	Iniform Traff	ic Control	Devices				
o be approved trol Plan Notes	by the l shown	Engineer prior on this sheet.	to the co. The Town	scribes the necessary traffic control measures for the mmencement of any work activities as indicated on the reserves the right to remove lane closures if traffic to the project.				
				t limit signage in accordance with VWAPM TTC-53.0. Dage remains in compliance if the project limits change.				
	ecessar	y temporary bo		able Changeable Message Sign,truck mounted impact on the plans submitted to the Engineer in accordance				
al Plans)								
or shall genera	lly consi	ruct the remai	'ning elem	ents of the project.				
				in accordance with VWAPM riping.and signage installation				
ane Roadway" a	and/or T	TTC-58.0 "End	l of Day S	ning for Partial Paving Signing for Full Paving ay and final surfacing				
ntain acc e ss to	all entr	ances and str	·eet connec	ctions at all times during				
EVEDC	TENC			TAX MAP 29-3				
EMERC				<u>FIRE - RESCUE 911</u> NA, VIRGINIA				
		DEPARTMEN	NT OF P	VA, VIRGINIA PUBLIC WORKS VIENNA, VA. 22180				
				DEPARTMENT OF PUBLIC WORKS 703-255-6380				
				OLD COURTHOUSE ROAD				
				PEDESTRIAN ACCESS IMPROVEMENTS TMP/SOC GENERAL NOTES				
				HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA				

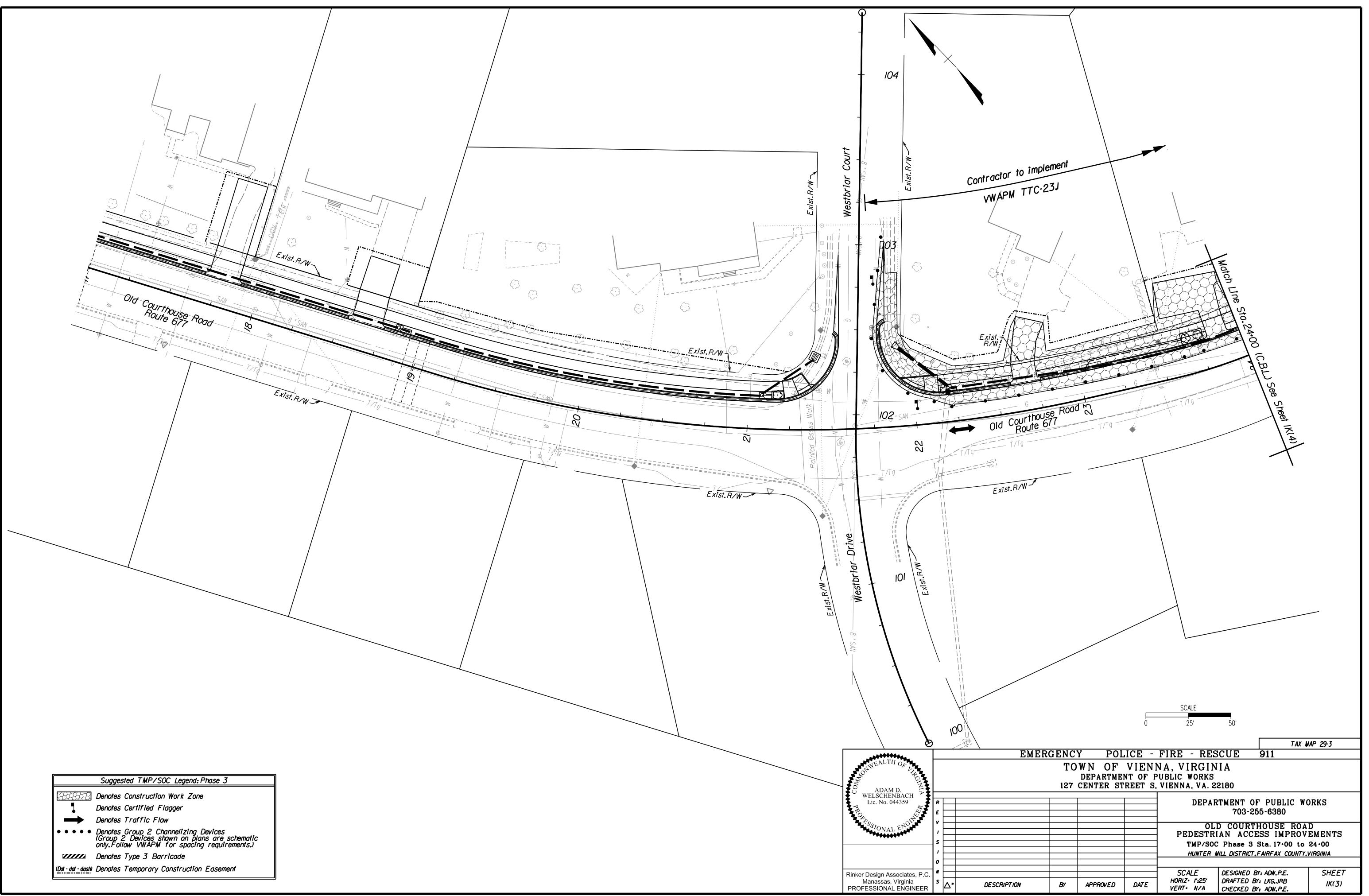
				SCALE	DESIGNED BY: ADW.P.E.	SHEET
DESCRIPTION	BY	APPROVED	DATE	HORIZ• F:25 ' VERT• N/A	DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	IJ

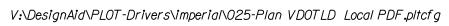


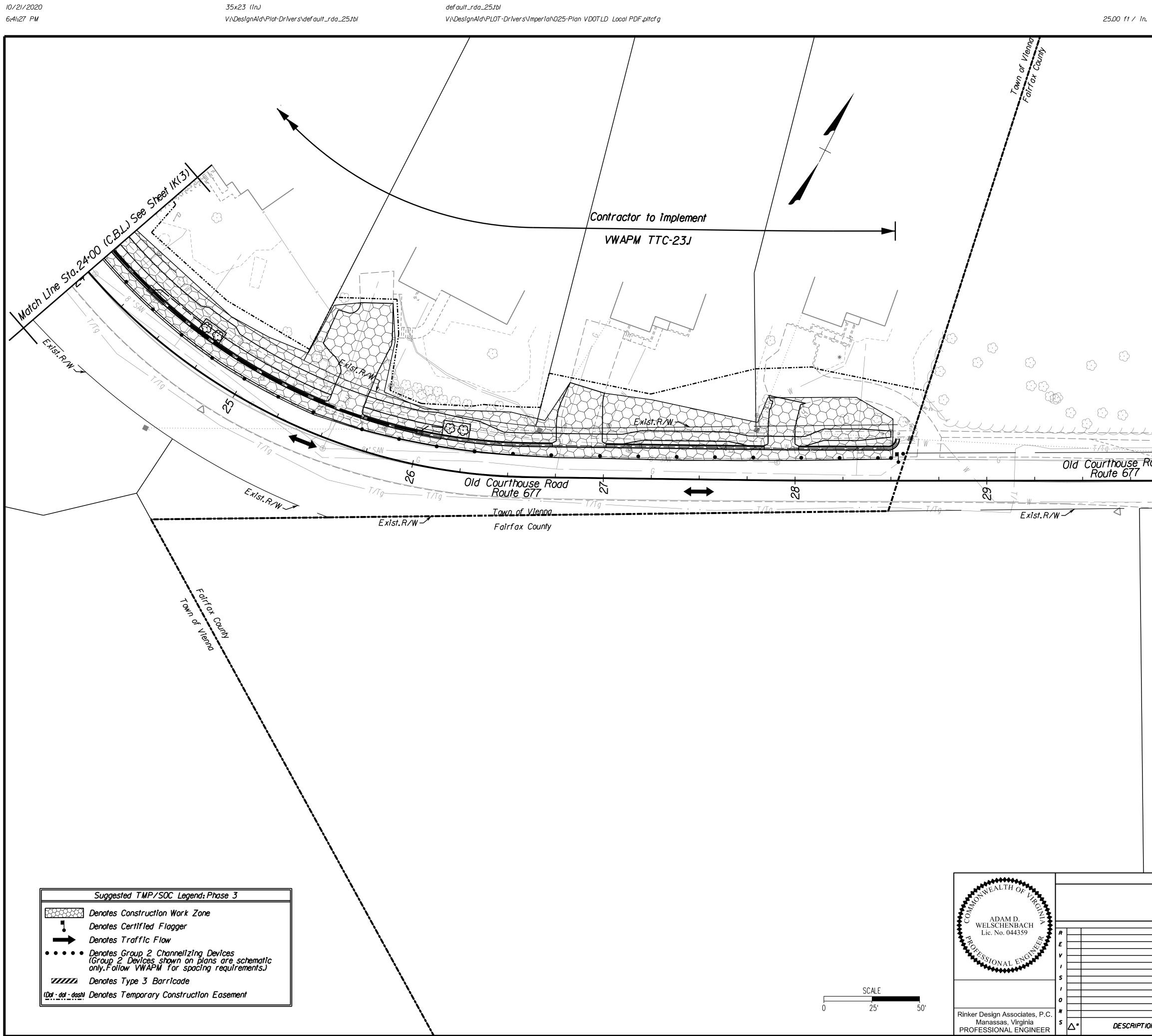












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RIPTION	BY	APPROVED	DATE	SCALE HORIZ= 1°=25' VERT= N/A	DESIGNED BY DRAFTED BY CHECKED BY	: LKG.JRB	SHEET IK(4)

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Erosion and Sediment Control Narrative

Project Description: This is a pedestrian access improvement project along Old Courthouse Road NE between Pine Valley Drive and Gosnell Road in the Town of Vienna, Virginia. The project proposes to add curb and gutter, sidewalk, curb ramps with pedestrian crossings at Westbriar Court. Additionally, all drainage will be collected and conveyed via a new proposed closed storm sewer system. The project is located in the Wolftrap Creek watershed management area which is within the greater Difficult Run watershed. The land disturbance area for this phase of the project is 1.08 ac.

Existing Site Conditions: The project site is along Old Courthouse Road NE between Pine Valley Drive and Gosnell Road. Vegetation within the project site consists of landscaped lawns and some large trees. Storm runoff is collected by roadside ditches and conveyed to four outfalls via existing closed storm sewer systems.

Ad jacent Areas: Areas ad jacent to the project are mostly residential or commercial in nature.

Off-site Areas: There will be minimal impacts to adjacent parcels associated with the construction of this project. All necessary right of way, right of entry agreements, easements, and provisions will be acquired prior to the start of construction. The Contractor shall be responsible for the locations of acceptable borrow and/or disposal sites, and these shall be in accordance with Town of Vienna requirements or as directed by the Town.

Soils: See soils map located on this sheet.

Critical Areas: There are no critical areas within the project site.

Erosion and Sediment Control Measures: Water quality and sediment/erosion control are of extreme importance. Care must be taken to avoid discharge of sediment into the existing storm water system. In order to best control impacts on this watershed, all vegetative and structural sediment control practices shall be constructed and maintained according to minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook. Strict compliance with this program and standards is required. We are therefore specifying a plan to minimize impacts on the ad jacent properties.

At the time of land disturbing activities within the Town right-of-way, the Contractor shall have a representative with Erosion and Sediment Control Contractor Certification (ESCCC) at the project site. The Town and Contractor are responsible for complying with applicable Local, State, and Federal Environmental Laws and Regulations, including acquiring clearances/authorizations from appropriate regulatory agencies.

Land Disturbing/Construction Sequence - Phase I I. The Contractor shall install the silt fence and inlet protection as shown on the Phase I Erosion & Sediment Control plan. 2. After the silt fence and inlet protection have been installed, the Contractor shall

obtain the site inspector's approval of these controls. 3. After the site inspector's approval of the initial controls, clear and grub the site as

Land Disturbing/Construction Sequence - Phase 2

I. Fine grade the site.

necessary.

2. Install curb & gutter, sidewalk, and entrance base course and concrete pavement. 3. Install all permanent sod and fertilize all grassed areas.

4. Clean site of all trash and debris.

5. Have the inspector inspect all areas to determine if they are adequately stabilized.

Maintenance Program: The Contractor shall make a visual inspection of all mechanical controls and newly stabilized areas (i.e. seeded, mulched, or sodded areas) on a daily basis and after each rainfall event to insure that all controls are functioning properly. The following items will be checked in particular: inlet protection will be checked regularly for sediment buildup which will prevent drainage, and if the gravel is clogged by sediment, it shall be removed and cleaned or replaced; the silt fence barrier will be checked regularly for undermining or deterioration of the fabric, and sediment shall be removed when the level of sediment deposition reaches halfway to the top of the barrier; and the seeded areas will be checked regularly to ensure that a good stand is maintained, and areas shall be fertilized and reseeded as needed. Any damaged controls shall be repaired by the end of the work day, including reseeding and mulching if necessary. The Contractor may install additional measures should he or she deem if necessary at the inspector's approval. All erosion & sediment controls shall be removed within seven (7) days after the project is stabilized.

Structural Practices:

I. Silt Fence Barrier (3.05) - Silt fence barriers will be installed downslope of areas with minimal grades to filter sediment-laden runoff from sheet flow as indicated in the Erosion and Sediment control plans.

2. Storm Drain Inlet Protection (3.07) - All storm sewer inlets shall be protected during construction. Sediment-laden water shall be filtered before entering the storm sewer inlets.

3. Culvert Inlet Protection (3.08) - All culverts shall be protected during construction. Sediment-laden water shall be filtered before entering the culvert. 4. Storm Drain Outlet Protection (3.18) - All storm sewer outlets shall be protected during construction.

5. Temporary Seeding (3.31) - All denuded areas which will be left dormant for extended periods of time shall be seeded with fast germinating temporary vegetation immediately following grading. Selection of the seed mixture will depend on the time of year it is applied.

6. Permanent Seeding (3.32) - Perennial vegetative cover shall be established on disturbed areas by planting seed to reduce erosion and decrease sediment yield and to permanently stabilize disturbed areas. Selection of the seed mixture will depend on the time of year it is applied. The planting soil shall be applied in accordance with Std. 3.30.

7. Permanent Stabilization - Permanent stabilization shall be done in accordance with the VESCH and all Town of Vienna seeding standards.

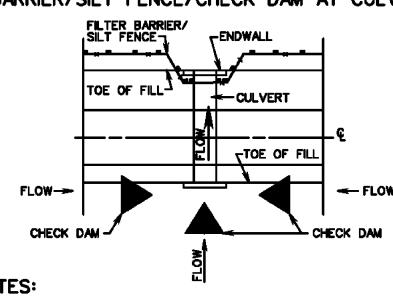
Stormwater Runoff Considerations: See sheet 2K series for Storm Computations and Outfall Analysis for this project.

Dust Control: Contractor shall be responsible to control dust throughout the entire construction phase by the application of water and/or approved adhesives per Std. 3.39 of the Virginia Erosion and Sediment Control Handbook.

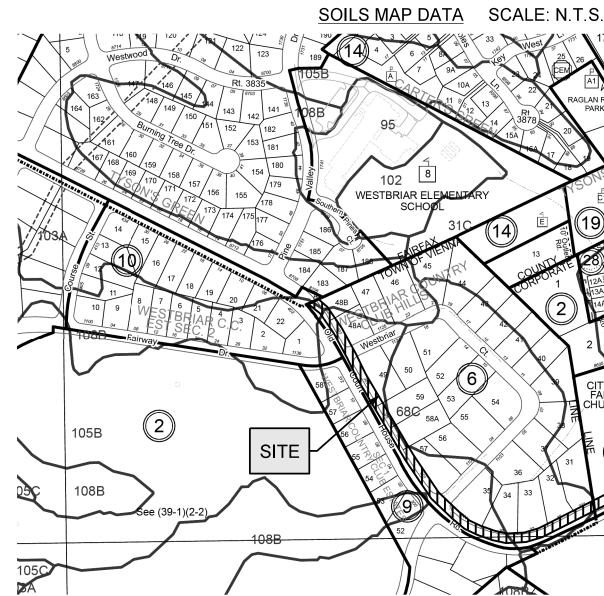




TYPICAL DETAIL FOR INSTALLATION OF TEMPORARY FILTER BARRIER/SILT FENCE/CHECK DAM AT CULVERT



NOTES:



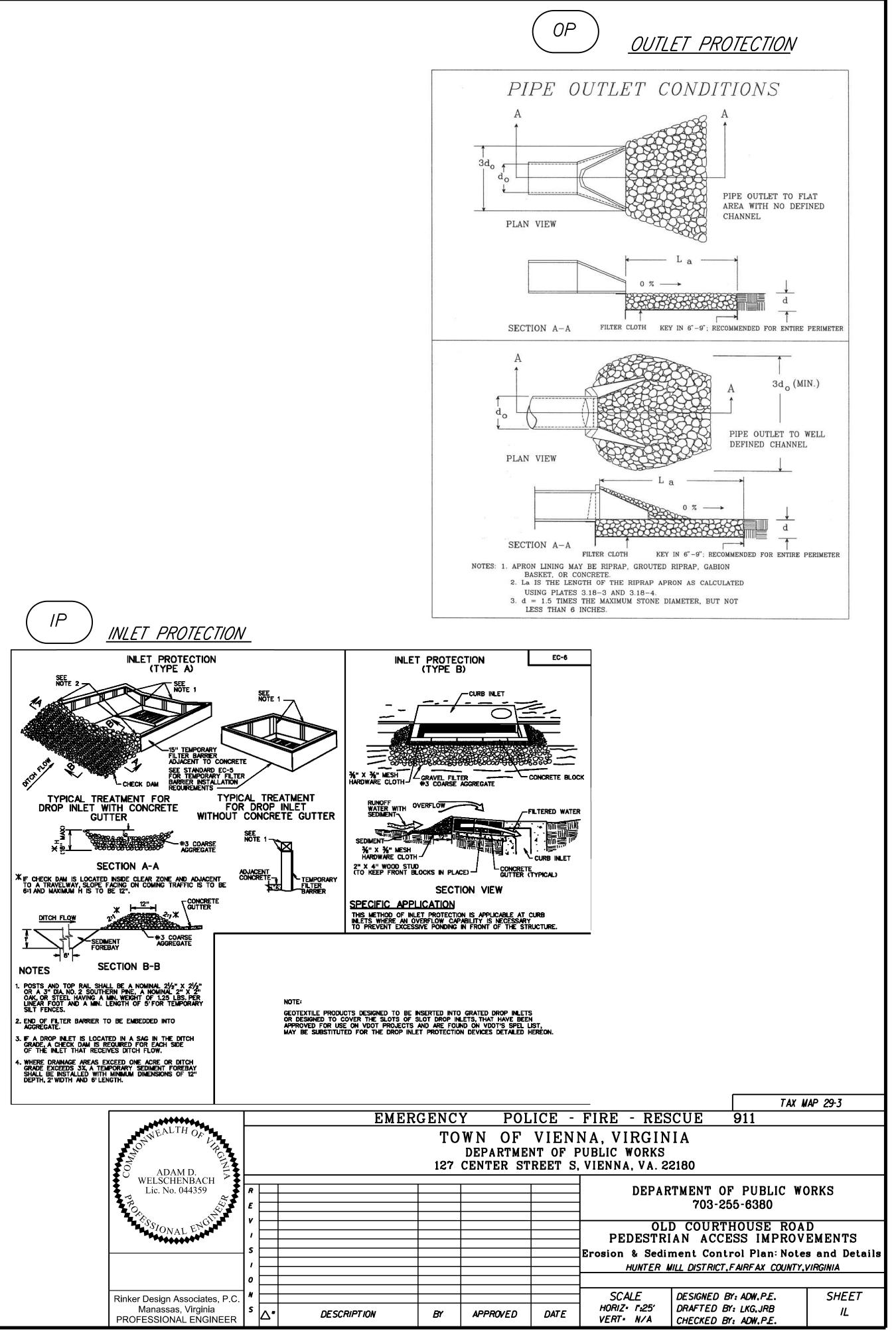
SOILS MA	NP SOURCE: ⊠COUNTY MAP; □P	RIVATE SOILS SO	CIENTIST (FOF	R UNMAPPED	SITES)			
SOIL ID NUMBERS	SOIL SERIES NAME	FOUNDATION SUPPORT	SOIL DRAINAGE	EROSION POTENTIAL	PROBLEM CLASS			
31C	DANRIPPLE GRAVELLY LOAM MARGINAL - b,w WARGINAL - MEDIUM II							
68C	KINGSTOWNE DANRIPPLE COMPLEX	MARGINAL - w, b	MARGINAL - w, s	MEDIUM	IVB			
103A	WHEATON CODORUS COMPLEX	POOR - f, w, b	POOR - f, w, s	LOW	IVA			
105B	WHEATON GLENELG COMPLEXGOODGOODHIGH							
108B	WHEATON SYMERDUCK COMPLEXMARGINAL - w, bPOOR - w, sMEDIUMIVB							
IS THE SITE YES 🗌 NO	LOCATED WITHIN NATURALLY C	CCURING ASBES	STOS SOILS?					
THE OR REGARI OCCUP/ DEPARI	THAT MAY CONTAIN NATURALLY ANGE SOILS TAX MAP GRIDS ON DIND THESE SOILS OR FILL ORIG ATIONAL SAFETY AND HEALTH R IMENT OF LABOR AND INDUSTRY B. ENVIRONMENTAL PROTECTION	THE COUNTY WI INATING FROM T EGULATIONS EN AND SPECIAL G	EBSITE. SPEC HESE SOILS A FORCED BY T	IAL PRECAUT RE REQUIREI HE VIRGINIA	IONS D BY			
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WESTBRIAR ELEMENTARY

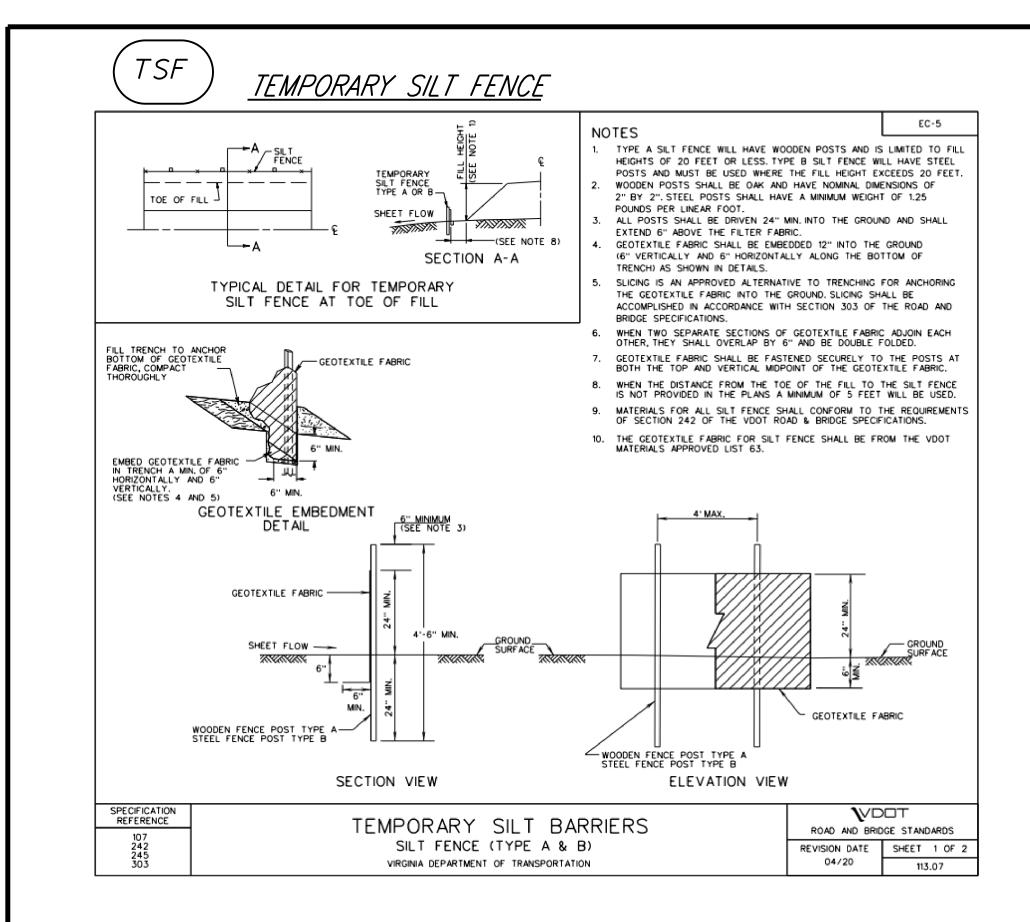
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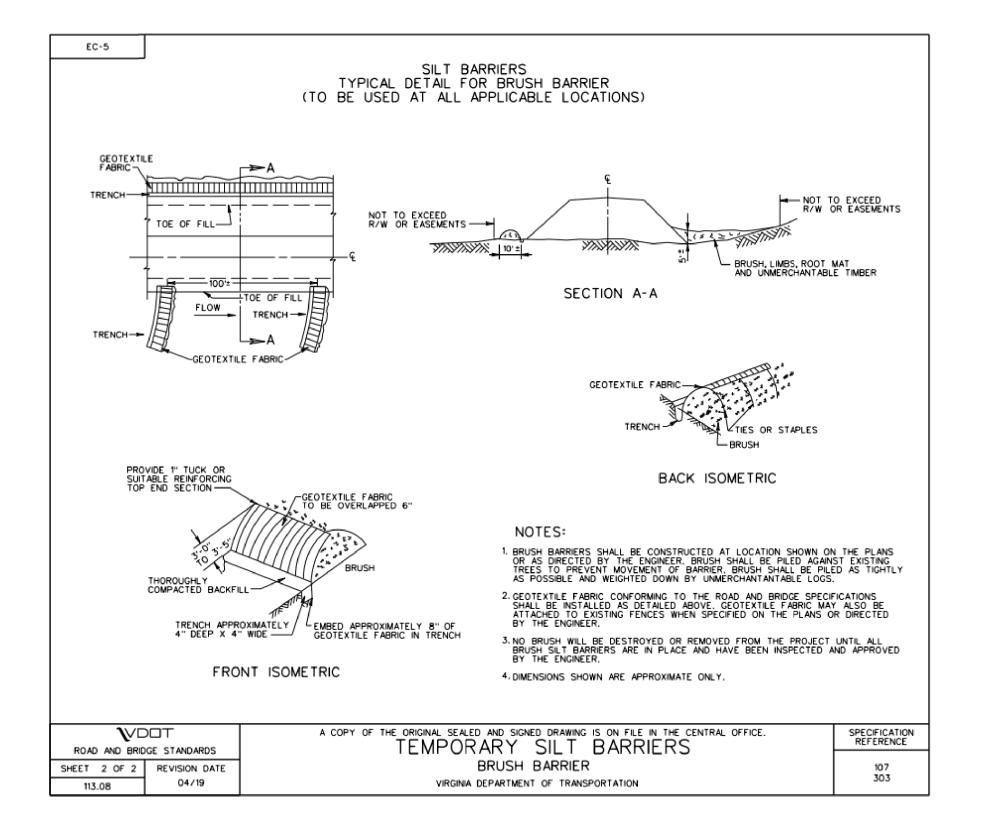
GREENSTONE. NATURALLY-OCCURRING ASBESTOS MINERALS, PREDOMINANTLY ACTINOLITE AND TREMOLITE, ARE KNOWN TO OCCUR IN THIS FORMATION. EXCAVATIONS IN BEDROCK OR EARTH MOVING ACTIVITIES WITHIN THIS FORMATION MAY EXPOSE THESE MINERALS TO THE ATMOSPHERE, ALLOWING THE FIBERS TO BECOME AIRBORNE.

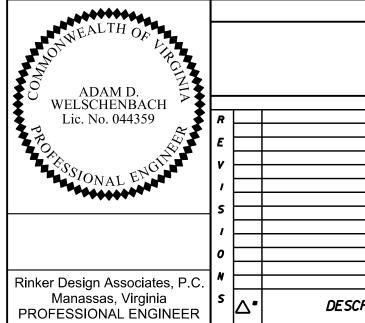


* INSTALLATION DETAIL ONLY - ROCK CHECK DAMS, FILTER BARRIER, AND SILT FENCE TO BE PAID FOR IN ACCORDANCE WITH THE ROAD AND BRIDGE SPECIFICATIONS.

1. IF ANY PORTION OF FILL IS GREATER THAN 5', SILT FENCE IS REQUIRED. IF FILL HEIGHT IS LESS THAN 5', FILTER BARRIER IS REQUIRED. 2. ROCK CHECK DAM IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE ROAD AND BRIDGE SPECIFICATIONS, AND STANDARD EC-4. 3. FILTER BARRIER/SILT FENCE IS TO BE INSTALLED IN ACCORDANCE WITH THE ROAD AND BRIDGE SPECIFICATIONS, AND STANDARD EC-5.

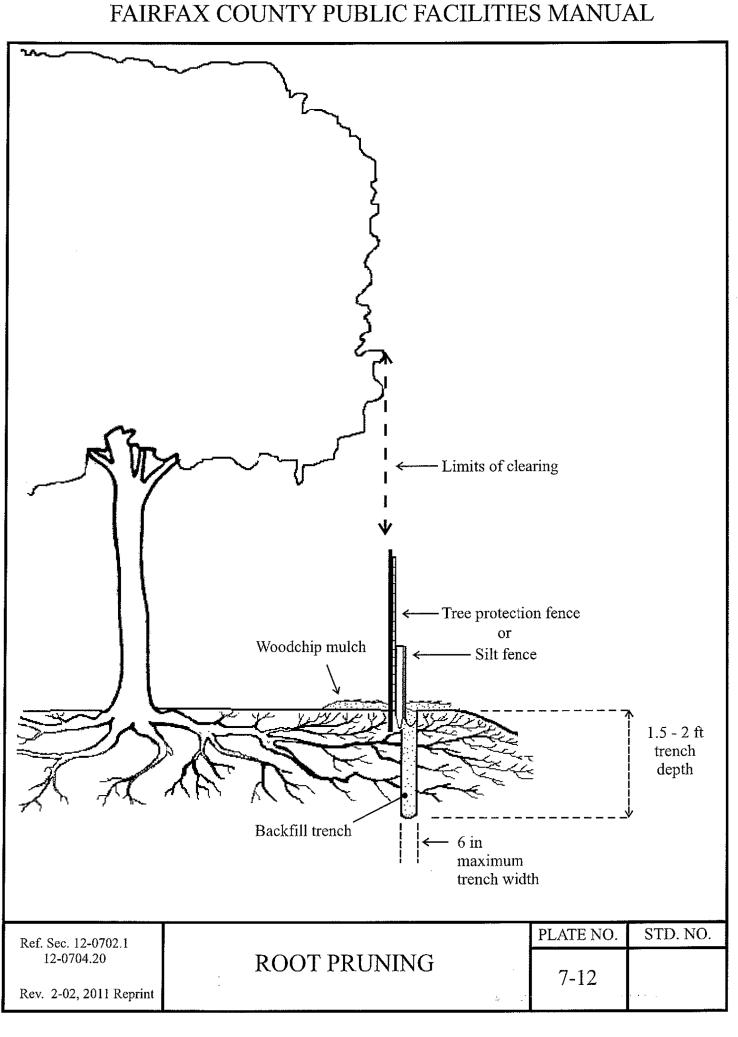


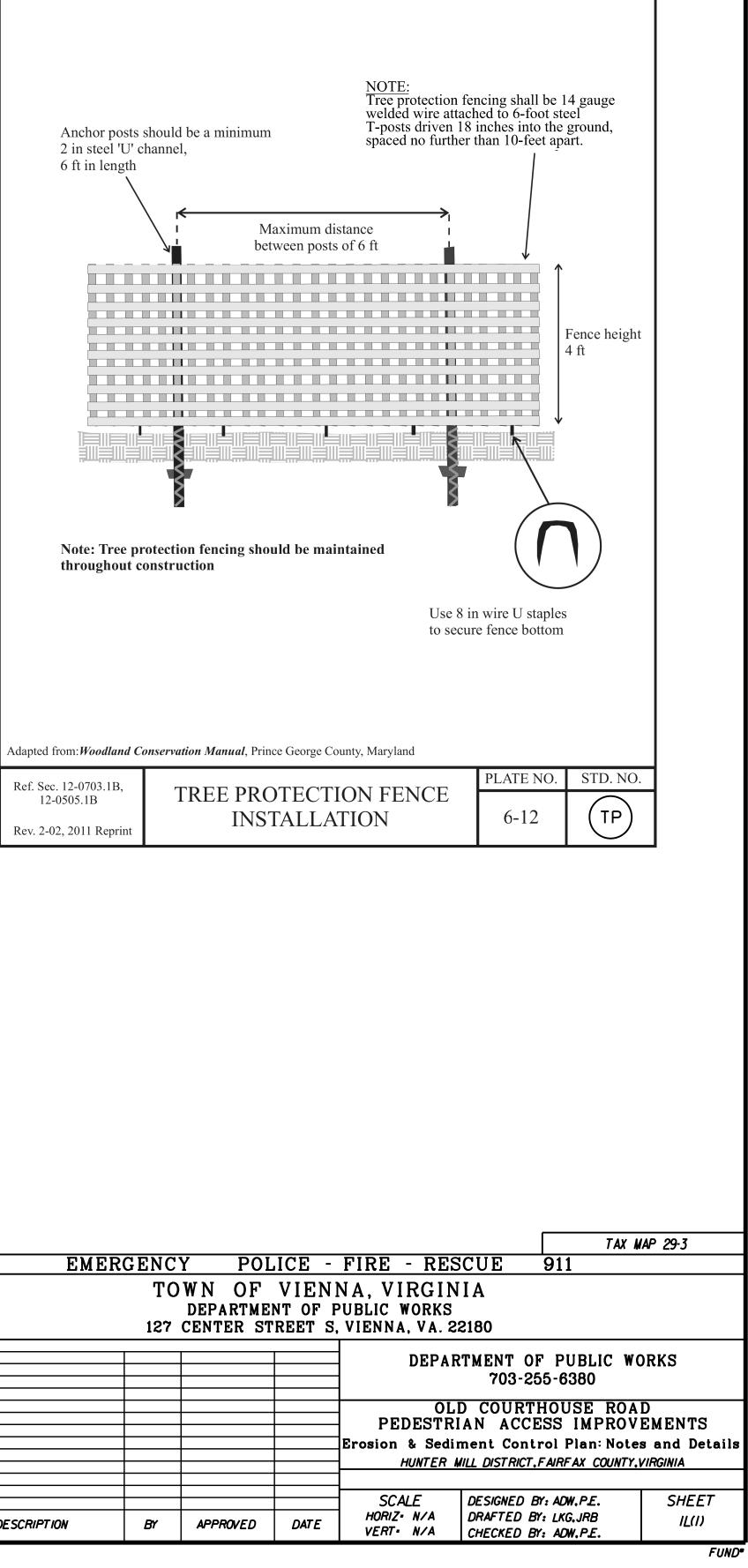




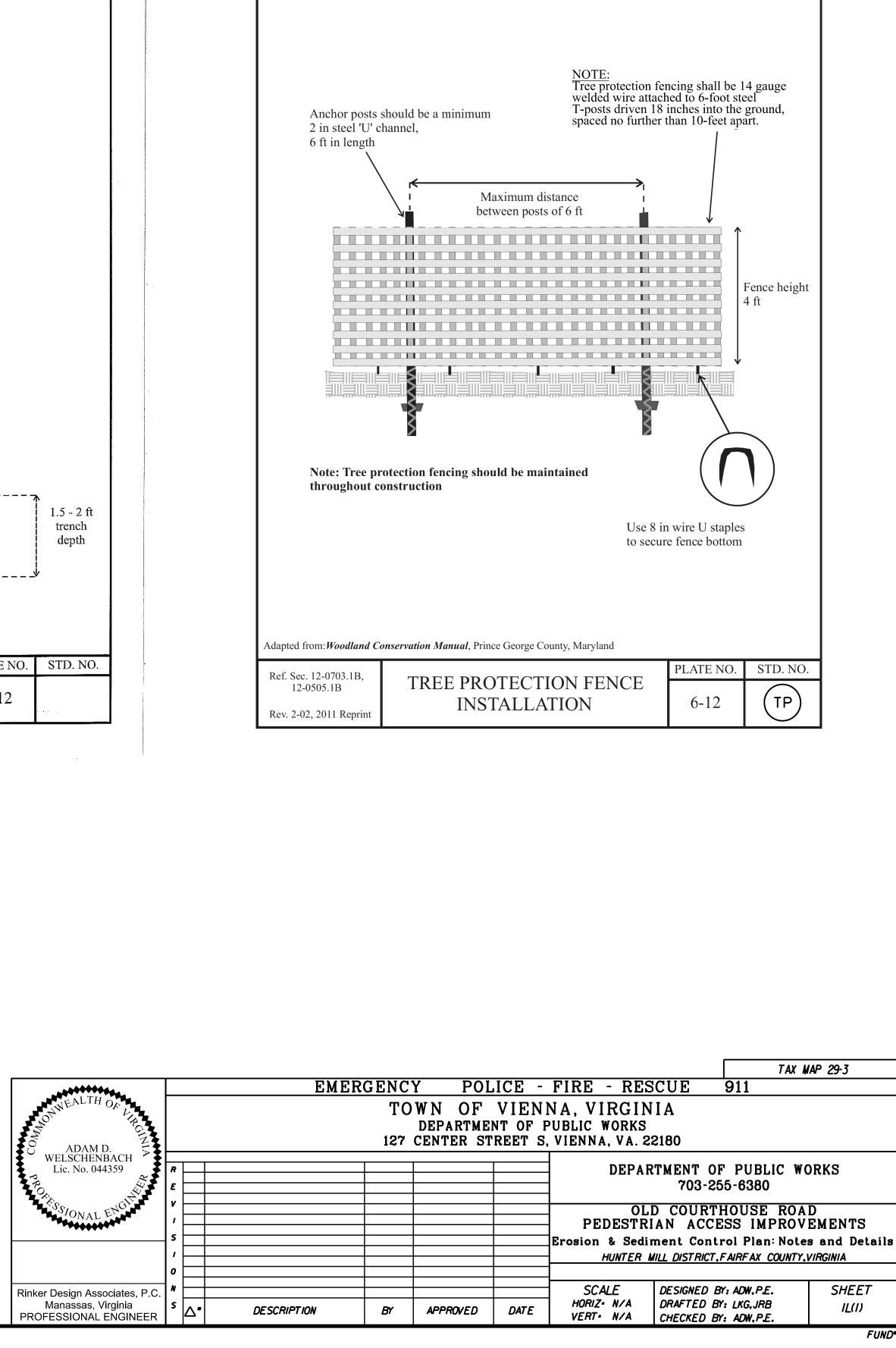
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				DEPAR		F PUBLIC WO 5-6380	DRKS			
				OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS E&S CONTROL DETAILS HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA						
RIPTION	BY	APPROVED	DATE	SCALE Horiz• r:25' Vert• n/a	DESIGNED B DRAFTED BY CHECKED BY	: LKG,JRB	SHEET IL(Oa)			

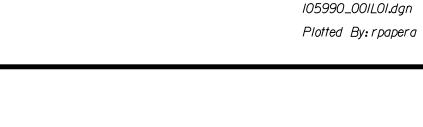
Erosion & Sediment Control Notes & Details





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FAIRFAX COUNTY PUBLIC FACILITIES MANUAL

4VAC50-30-40.<u>Minimum Standards</u>.(MS-I9)

A VESCP must be consistent with the following criteria, techniques and methods:

- I. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 14 days. Permanent stabilization shall be applied to areas that are to be left dormant for more than one year.
- 2. During construction of the project, soil stock piles and borrow areas shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as borrow areas and soil intentionally transported from the project site.
- 3.A permanent vegetative cover shall be established on denuded areas not otherwise permanently stabilized. Permanent vegetation shall not be considered established until a ground cover is achieved that is uniform, mature enough to survive and will inhibit erosion.
- 4. Sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment shall be constructed as a first step in any land-disturbing activity and shall be made functional before upslope land disturbance takes place.
- 5. Stabilization measures shall be applied to earthen structures such as dams.dikes and diversions immediately after installation.
- 6. Sediment traps and sediment basins shall be designed and constructed based upon the total drainage area to be served by the trap or basin. a. The minimum storage capacity of a sediment trap shall be 134 cubic yards per acre of drainage area and the trap shall only control drainage areas less than three acres.
- b. Surface runoff from disturbed areas that is comprised of flow from drainage areas greater than or equal to three acres shall be controlled by a sediment basin. The minimum storage capacity of a sediment basin shall be 134 cubic yards per acre of drainage area. The outfall system shall, at a minimum, maintain the structural integrity of the basin during a 25-year storm of 24-hour duration. Runoff coefficients used in runoff calculations shall correspond to a bare earth condition or those conditions expected to exist while the sediment basin is utilized.
- 7. Cut and fill slopes shall be designed and constructed in a manner that will minimize erosion. Slopes that are found to be eroding excessively within one year of permanent stabilization shall be provided with additional slope stabilizing measures until the problem is corrected.
- 8. Concentrated runoff shall not flow down cut or fill slopes unless contained within an adequate temporary or permanent channel, flume or slope drain structure.
- 9.Whenever water seeps from a slope face, adequate drainage or other protection shall be provided.
- IO. All storm sewer inlets that are made operable during construction shall be protected so that sediment-laden water cannot enter the conveyance system without first being filtered or otherwise treated to remove sediment.
- II. Before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel.
- 12. When work in a live watercourse is performed, precautions shall be taken to minimize encroachment, control sediment transport and stabilize the work area to the greatest extent possible during construction. Non-erodible material shall be used for the construction of causeways and cofferdams. Earthen fill may be used for these structures if armored by non-erodible cover materials.
- 13. When a live watercourse must be crossed by construction vehicles more than twice in any six-month period, a temporary vehicular stream crossing constructed of non-erodible material shall be provided.
- 14. All applicable federal, state and local chapters pertaining to working in or crossing live watercourses shall be met.
- 15. The bed and banks of a watercourse shall be stabilized immediately after work in the watercourse is completed.
- 16. Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria: a. No more than 500 linear feet of trench may be opened at one time.
- b. Excavated material shall be placed on the uphill side of trenches.
- c. Effluent from dewatering operations shall be filtered or passed through an approved sediment trapping device, or both, and discharged in a manner that does not adversely affect flowing streams or off-site property.
- d.Material used for backfilling trenches shall be properly compacted in order to minimize erosion and promote stabilization. e. Restabilization shall be accomplished in accordance with this
- chapter.
- f. Applicable safety chapters shall be complied with.

Frosion & Sediment Control Notes & Details

- land-disturbing activities.
- further erosion and sedimentation.
- natural or man-made channels:
- the outfall of the pipe or pipe system shall be performed.
- manner:
- contributing drainage area of the project in question; or 2)
- of channel bed or banks.
- pipe or system.
- channels or pipes are not adequate, the applicant shall:
- channel the bed or banks; or
- storm is contained within the appurtenances;
- man-made channel; or
- downstream erosion.
- d.The applicant shall provide evidence of permission to make the improvements.
- e. All hydrologic analyses shall be based on the existing watershed project.
- the facility to the receiving channel.
- h. All on-site channels must be verified to be adequate.
- pipe or pipe system, or to a detention facility. in a residential, commercial or industrial development shall not be all engineering calculations.

17. Where construction vehicle access routes intersect paved or public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a paved or public road surface, the road surface shall be cleaned thoroughly at the end of each day. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner. This provision shall apply to individual development lots as well as to larger

18. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization or after the temporary measures are no longer needed, unless otherwise authorized by the VESCP authority. Trapped sediment and the disturbed soil areas resulting from the disposition of temporary measures shall be permanently stabilized to prevent

19. Properties and waterways downstream from development sites shall be protected from sediment deposition, erosion and damage due to increases in volume, velocity and peak flow rate of stormwater runoff for the stated frequency storm of 24-hour duration in accordance with the following standards and criteria. Stream restoration and relocation projects that incorporate natural channel design concepts are not man-made channels and shall be exempt from any flow rate capacity and velocity requirements for

a. Concentrated stormwater runoff leaving a development site shall be discharged directly into an adequate natural or man-made receiving channel, pipe or storm sewer system. For those sites where runoff is discharged into a pipe or pipe system, downstream stability analyses at b. Adequacy of all channels and pipes shall be verified in the following

I) The applicant shall demonstrate that the total drainage area to the point of analysis within the channel is one hundred times greater than the

a)Natural channels shall be analyzed by the use of a two-year storm to verify that stormwater will not overtop channel banks nor cause erosion

b) All previously constructed man-made channels shall be analyzed by the use of a ten-year storm to verify that stormwater will not overtop its banks and by the use of a two-year storm to demonstrate that stormwater will not cause erosion of channel bed or banks; and c) Pipes and storm sewer systems shall be analyzed by the use of a ten-year storm to verify that stormwater will be contained within the

c. If existing natural receiving channels or previously constructed man-made I) Improve the channels to a condition where a ten-year storm will not overtop the banks and a two-year storm will not cause erosion to

2) Improve the pipe or pipe system to a condition where the ten-year

3) Develop a site design that will not cause the pre-development peak runoff rate from a two-year storm to increase when runoff outfalls into a natural channel or will not cause the pre-development peak runoff rate from a ten-year storm to increase when runoff outfalls into a

4) Provide a combination of channel improvement, stormwater detention or other measures which is satisfactory to the VESCP authority to prevent

characteristics and the ultimate development condition of the subject

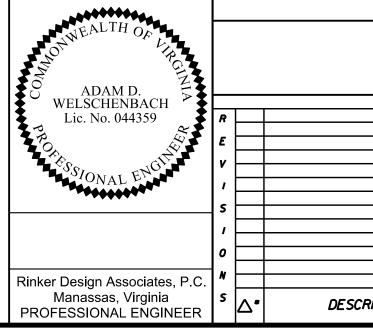
f. If the applicant chooses an option that includes stormwater detention, he shall obtain approval from the VESCP of a plan for maintenance of the detention facilities. The plan shall set forth the maintenance requirements of the facility and the person responsible for performing the maintenance. g. Outfall from a detention facility shall be discharged to a receiving channel, and energy dissipators shall be placed at the outfall of all detention facilities as necessary to provide a stabilized transition from

i. Increased volumes of sheet flows that may cause erosion or sedimentation on adjacent property shall be diverted to a stable outlet, adequate channel,

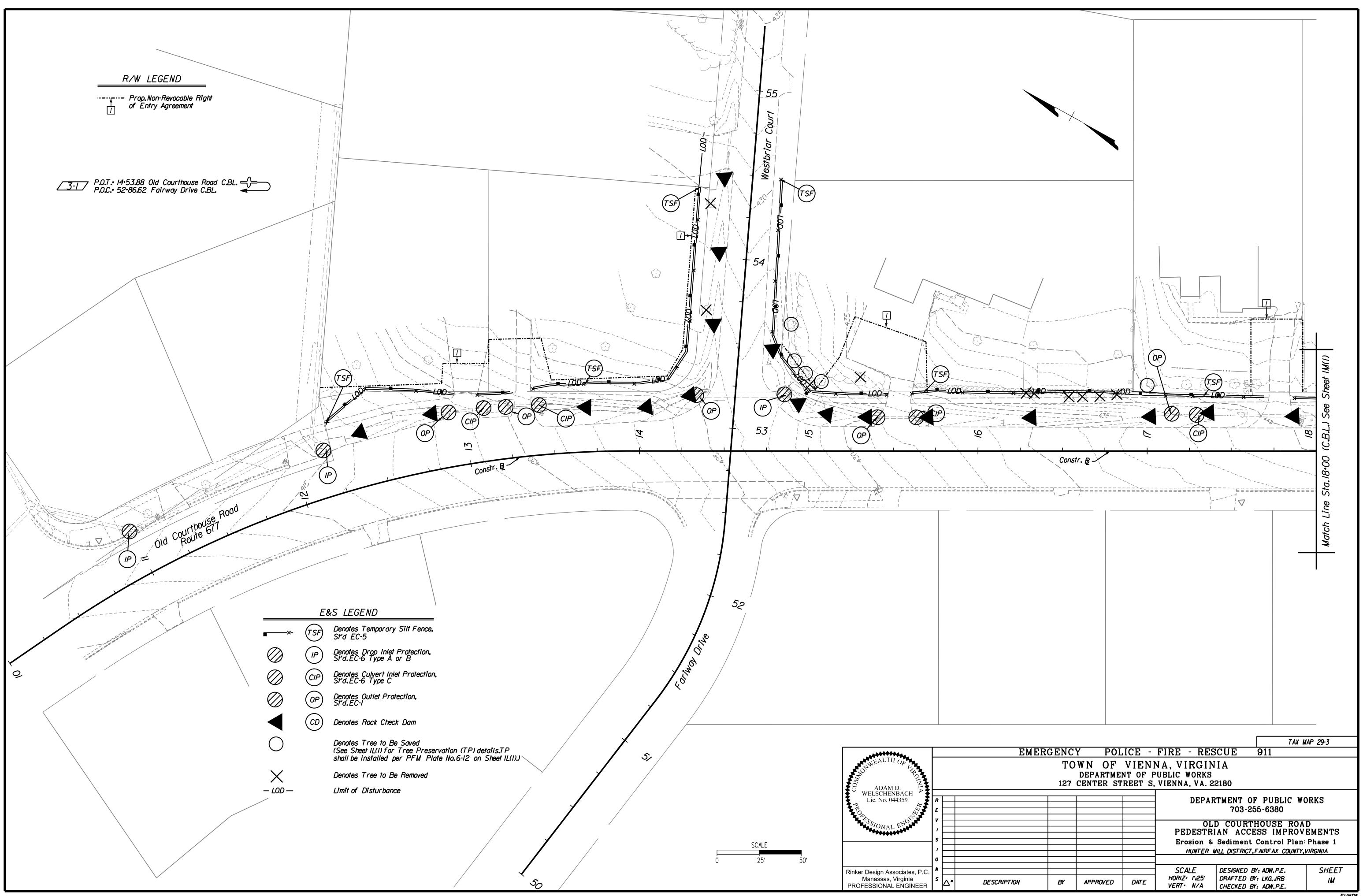
j. In applying these stormwater management criteria, individual lots or parcels

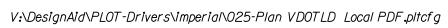
considered to be separate development projects. Instead, the development, as a whole, shall be considered to be a single development project. Hydrologic parameters that reflect the ultimate development condition shall be used in

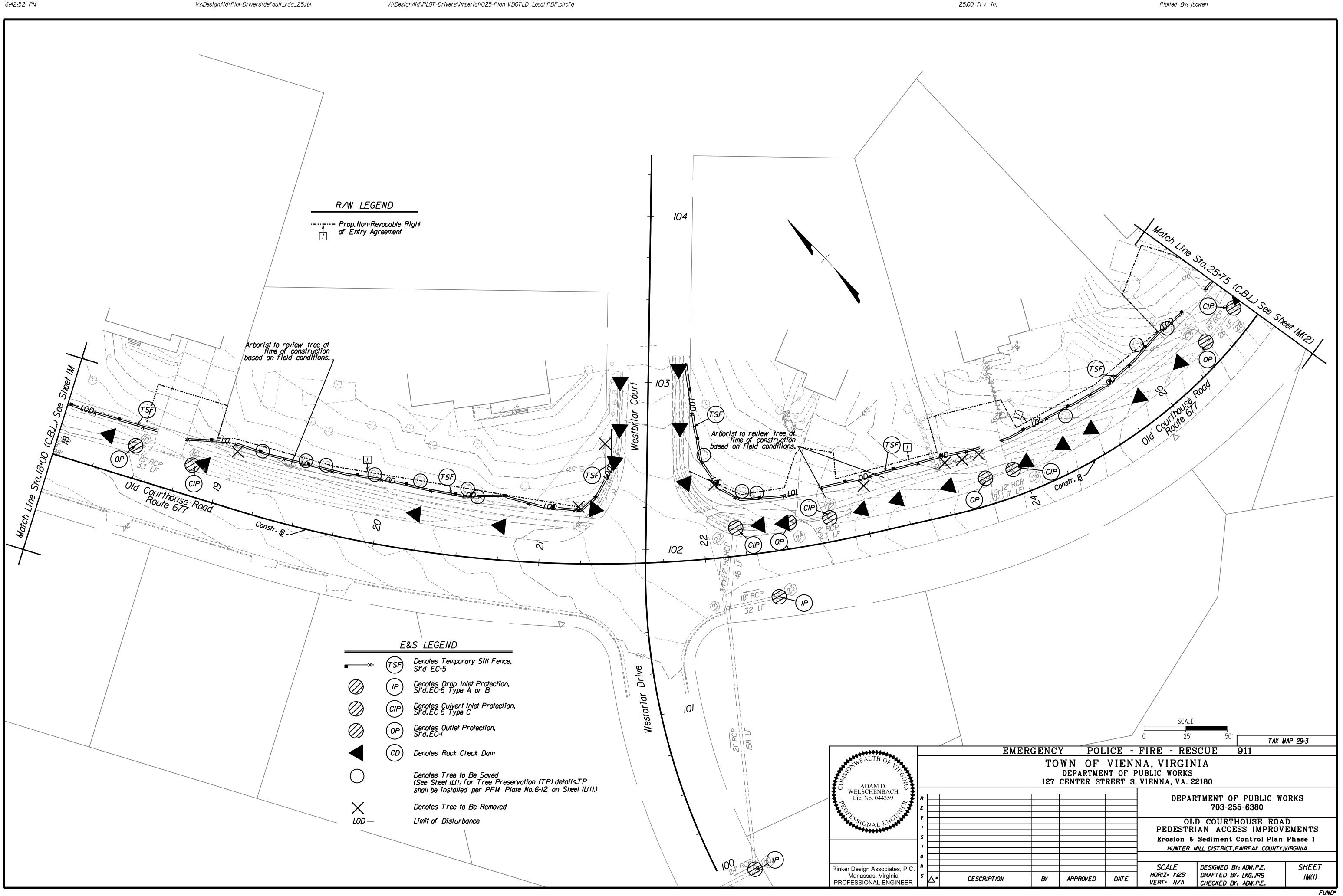
- k. All measures used to protect properties and waterways shall be employed in a manner which minimizes impacts on the physical, chemical and biological integrity of rivers, streams and other waters of the state.
- I. Any plan approved prior to July 1,2014, that provides for stormwater management that addresses any flow rate capacity and velocity requirements for natural or man-made channels shall satisfy the flow rate capacity and velocity requirements for natural or man-made channels if the practices are designed to (i) detain the water quality volume and to release it over 48 hours; (ii) detain and release over a 24-hour period the expected rainfall resulting from the one year, 24-hour storm; and (iii) reduce the allowable peak flow rate resulting from the 1.5,2, and 10-year, 24-hour storms to a level that is less than or equal to the peak flow rate from the site assuming it was in a good forested condition, achieved through multiplication of the forested peak flow rate by a reduction factor that is equal to the runoff volume from the site when it was in a good forested condition divided by the runoff volume from the site in its proposed condition, and shall be exempt from any flow rate capacity and velocity requirements for natural or man-made channels as defined in any regulations promulgated pursuant to IOJ-562 or 10.1-570 of the Act.
- m.For plans approved on and after July 1,2014, the flow rate capacity and velocity requirements of IO.I-56I A of the Act and this subsection shall be satisfied by compliance with water quantity requirements in the Stormwater Management Act (10,1-603,2 et seq. of the Code of Virginia) and attendant regulations, unless such land-disturbing activities are in accordance with 4VAC50-60-48 of the Virginia Stormwater Management Program (VSMP) Permit Regulations.
- n.Compliance with the water quantity minimum standards set out in 4VAC50-60-66 of the Virginia Stormwater Management Program (VSMP) Permit Regulations shall be deemed to satisfy the requirements of Minimum Standard 19.



EMER	GENC			FIRE - RES		(MAP 29-3
	ТО	WN OF DEPARTME	VIEN NT OF F	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 2	IIA	
				DEPAR	RTMENT OF PUBLIC 703-255-6380	WORKS
				PEDESTR Erosion & Sedi	D COURTHOUSE RO IAN ACCESS IMPRO ment Control Plan: No WILL DISTRICT, FAIRFAX COUNT	VEMENTS tes and Details
RIPTION	ΒΥ	APPROVED	DATE	SCALE HORIZ• N/A VERT• N/A	DESIGNED BY: ADW, P.E. DRAFTED BY: LKG, JRB CHECKED BY: ADW, P.E.	SHEET IL(2)



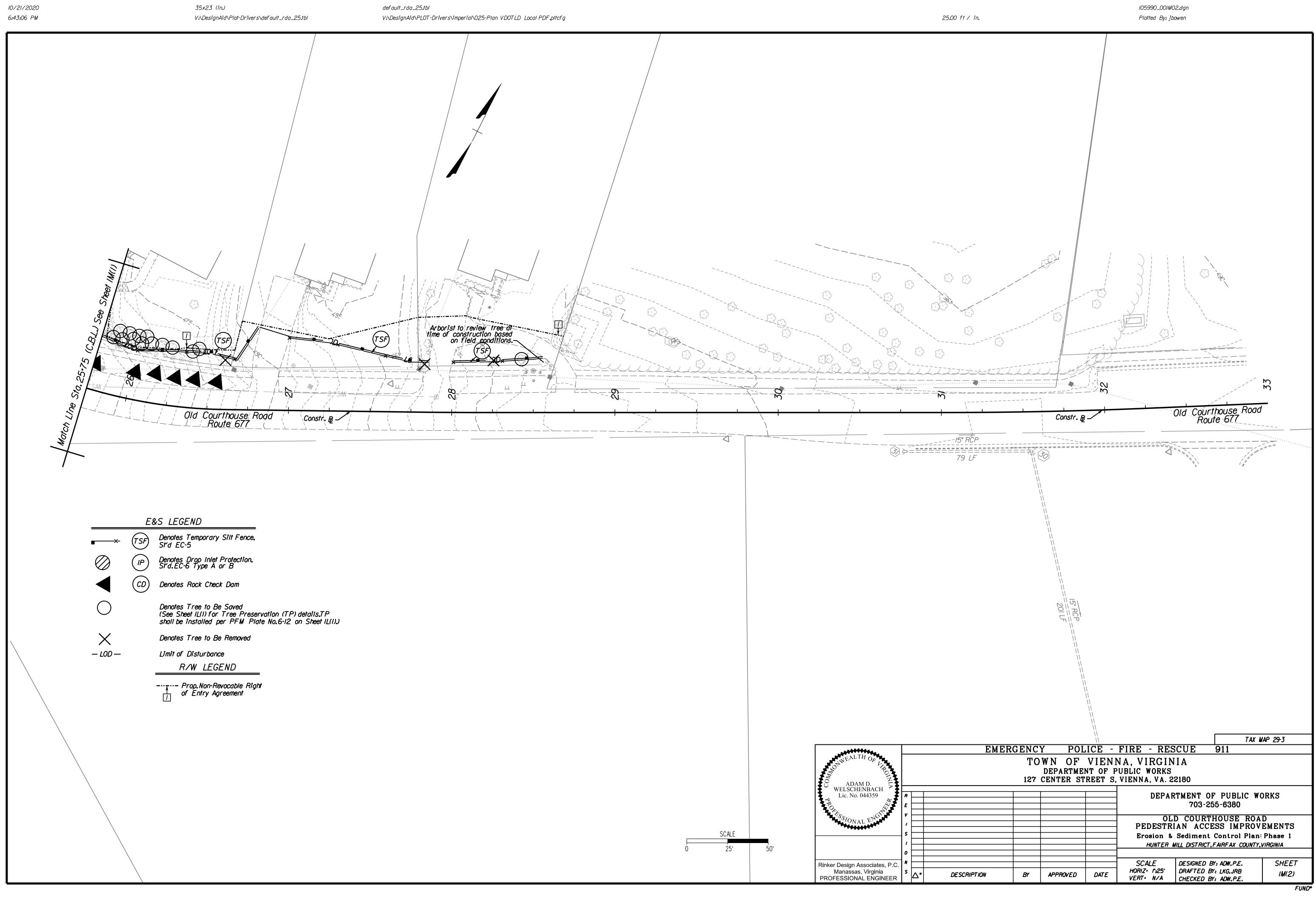


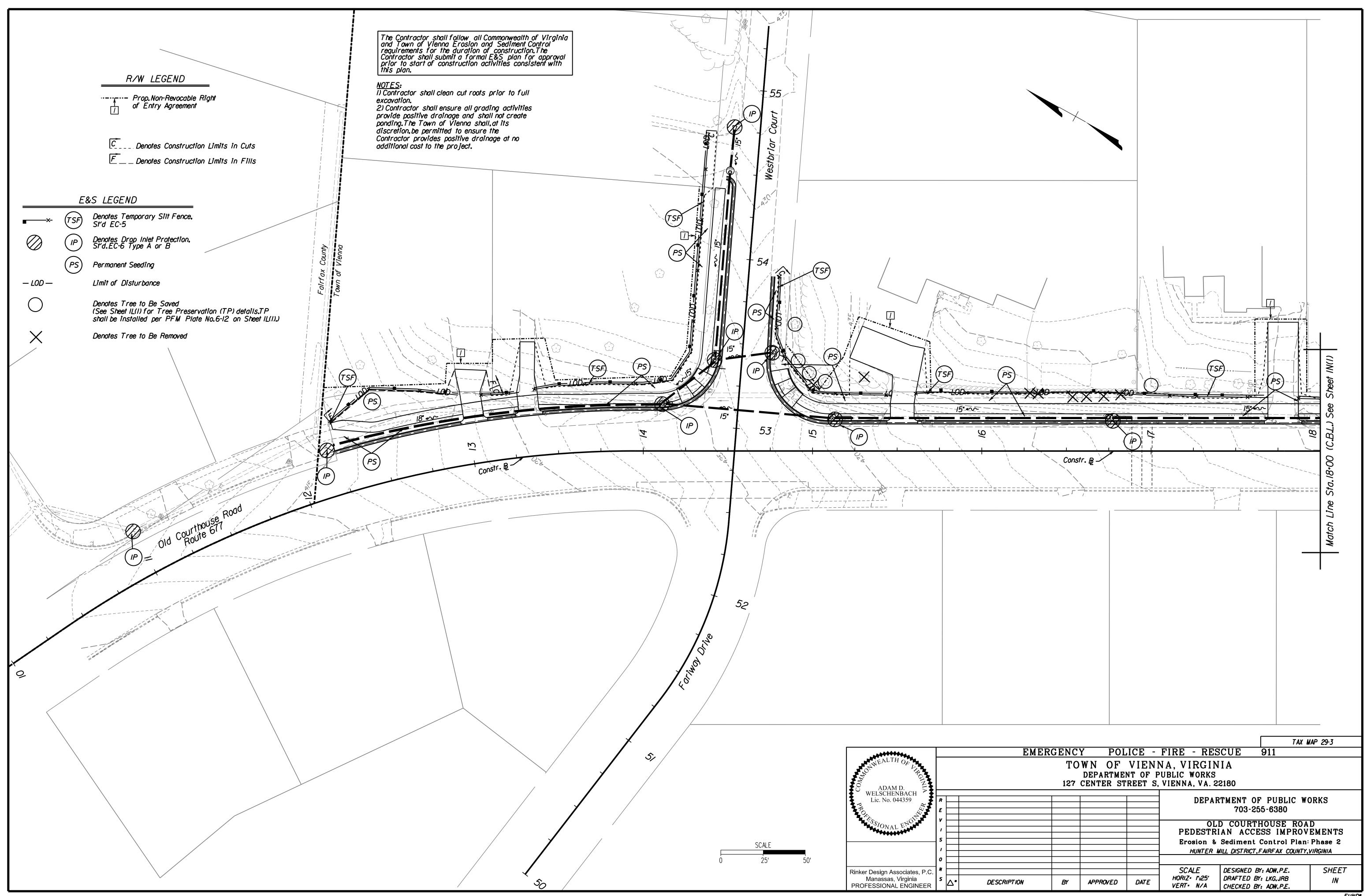


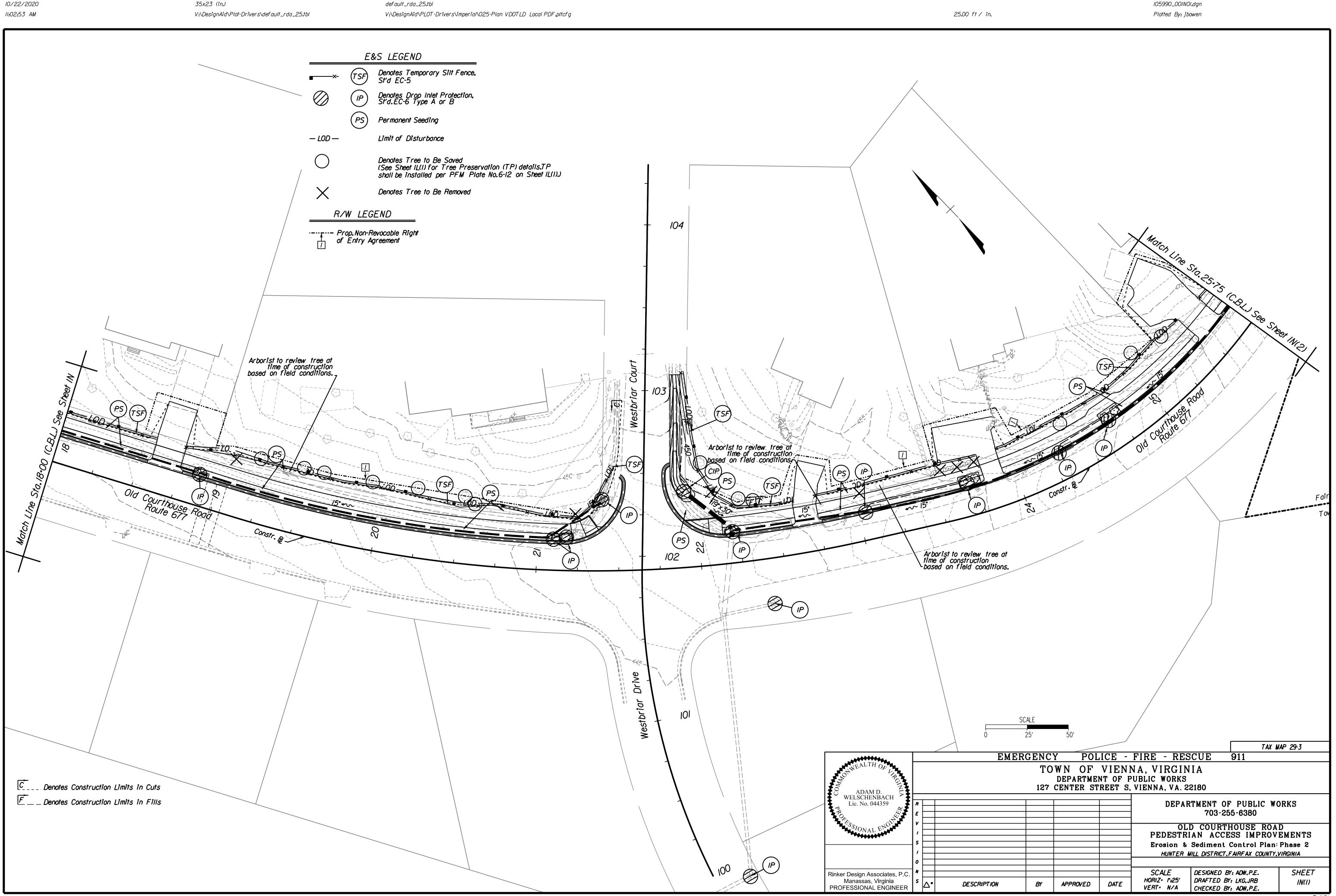
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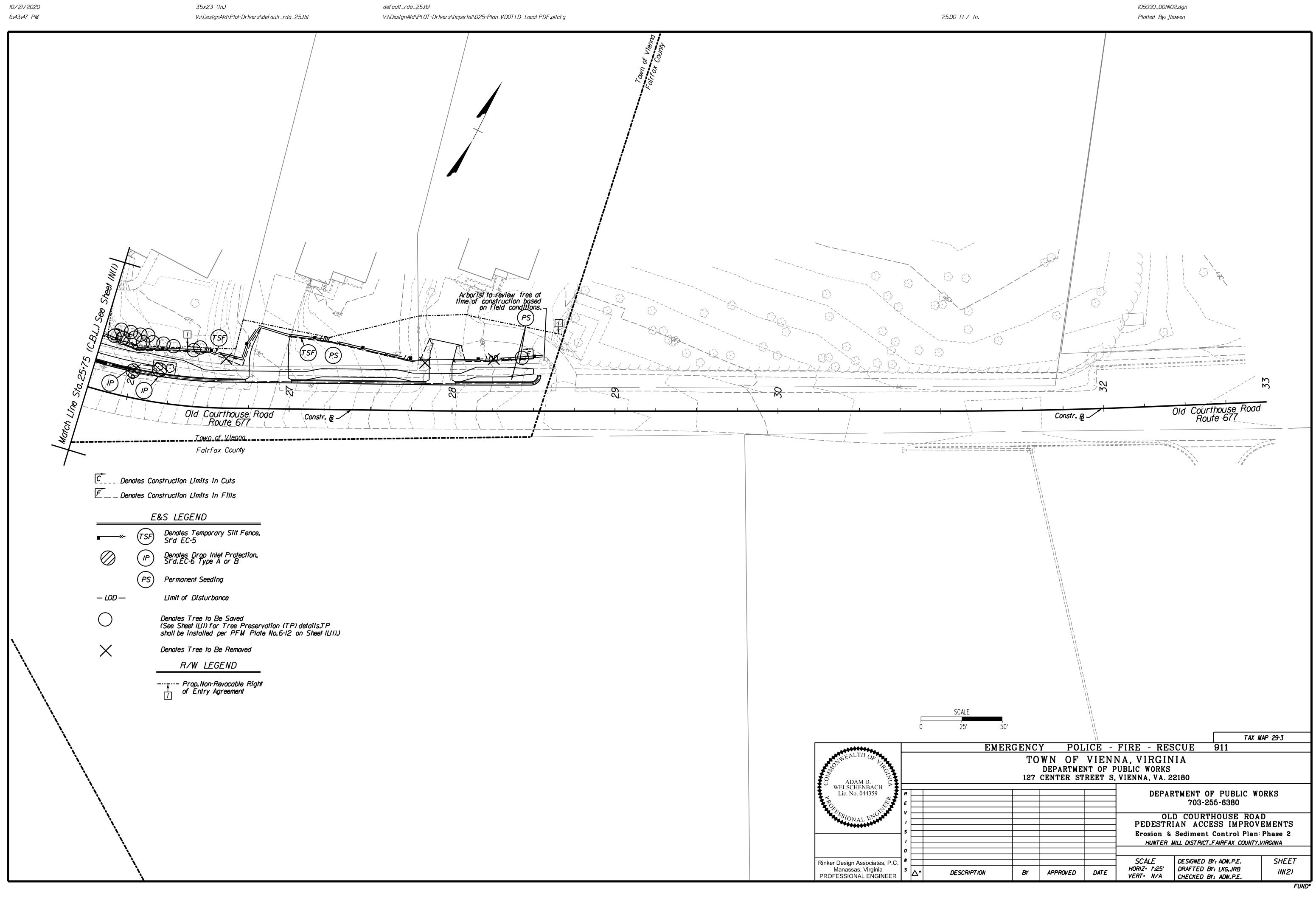
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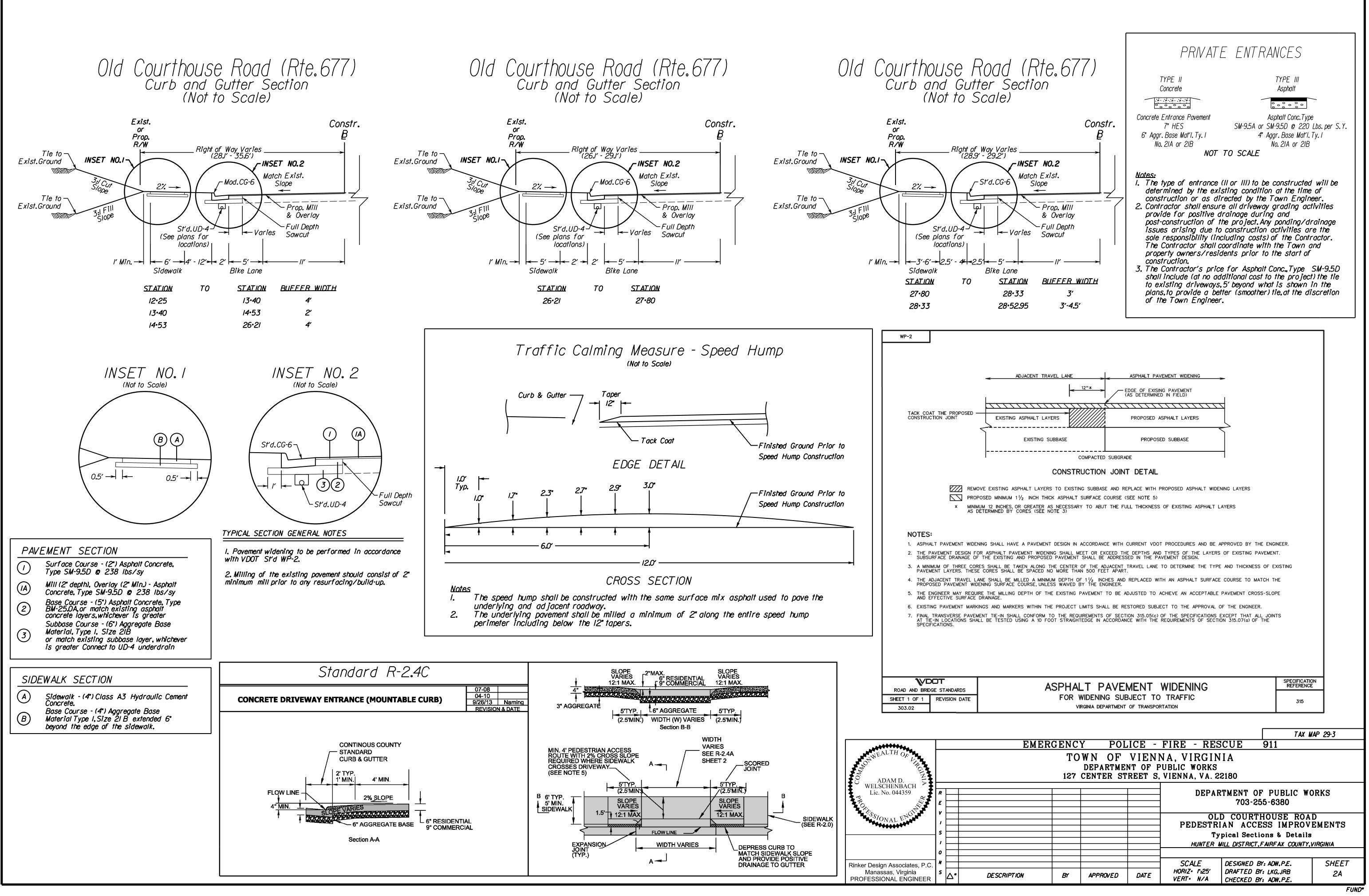
10/21/2020











F	C	
F)	

			RMATION								
PROJECT INFORMATION											
PROJECT NAME		Old Courthouse Road Pedestrian Access Improvement									
COUNTY PROJECT NUMBER		FFX 104325									
VDOT UPC NUMBER (IF APPLICABLE)	PE – xxxxxx RW – xxxxxx CN – xxxxxx										
PROJECT LIMITS / LENGTH	North C	County Line to	Battery	y Park	St / 0 . 40 m	<i>i</i> .					
FUNDING SOURCE		Locality	,								
DATE OF FUNDING OBLIGATION	2016										
LATITUDE/ LONGITUDE		LAT	38° 55′ 0" N		LONG	77°14′38"W	/				
6TH ORDER HUC		020700	0081004 (PL)	<u>22 Dif</u>	ficult R	ในก)					
TYPE OF DEVELOPMENT: (SELECT ALL TH	IAT APPLY	<i>'</i>)									
NEW DEVELOPMENT											
LINEAR DEVELOPMENT											
STORMWATER MANAGEMENT TECHNICAL C		SED:									
VSMP TECHNICAL CRITERIA	IIB / FFX	CO STOP	RMWATER MAN	AGEMEN	IT ORDIN	ANCE ARTICL	E 4				
	-										
		NO APPROVAL DATE:									
TYPE(S): Detention Exception			LDS NUM								
· ·	TEC										
CONSTRUCTION SITE ESTIMA		109			17 0 45	-	(SF)				
TOTAL SITE AREA		1.08		(AC)	(AC) 47,045 (AC) 47,045						
TOTAL DISTURBED AREA		1.08	0.47			1	(SF)				
TOTAL SITE IMPERVIOUS AREA		PRE	0.43	(AC)	POST	0.63	(AC)				
VPDES PERMIT REQUIREMENTS (CHECK O	NE):										
DISTURBED AREA < 1 (AC) ; VPDE	S PERMIT	NOT REC	QUIRED.								
\square DISTURBED AREA ≥ 1 (AC); VPDE	S PERMIT	IS REQU	RED.								
RECEIVING WATERS											
RECEIVING STREAM		Wolftra	p Creek								
WATERSHED		Difficult Run									
DESCRIPTION OF IMPAIRED WATERS SUBJ TMDLs, IF APPLICABLE	ECT TO	Not App	licable								
NOTE:											
	E THE L	ATEST	REVISION OF	DRAI	NAGE	COMPUTATIC	ONS				
I.) FOR ADDITIONAL DETAILS SE		OL PLA	N SHEETS 1	L-1M(1).						
I.) FOR ADDITIONAL DETAILS SE BOOK AND EROSION AND SEDIMEN	CONTRO			•	•						
BOOK AND EROSION AND SEDIMEN			AND THE S	TORM	WATER	POLLUTION					
300K AND EROSION AND SEDIMEN ⁻ 2.) THIS IS A COUNTY ADMINIST	ERED PR	OJECT									
300K AND EROSION AND SEDIMEN 2.) THIS IS A COUNTY ADMINIST PREVENTION PLANS (SWPPP) IS PR	ERED PR REPARED	OJECT	RFAX COUN								
300K AND EROSION AND SEDIMEN ⁻ 2.) THIS IS A COUNTY ADMINIST	ERED PR REPARED	OJECT	RFAX COUN								

OWNER/REPRI	ESENTATIVE CONTACT
NAME:	MICHAEL GALLAGHER, DEPUTY DIRECTOR OF PUBLIC WORKS TOWN OF VIENNA
PHONE NUMBER: EMAIL ADDRESS:	703–255–6380 MGALLAGHER@VIENNAVA.GOV
ADDRESS:	DEPARTMENT OF PUBLIC WORKS 127 CENTER ST. S VIENNA, VA 22180

ITY POLYETHYLEN	E (HDPE) USE					VPDES PERM COUNTY STORM	IWATEI
ES (PROPOSED O	NLY)		GRANDFA	THERING CRITE			
FACILITY TYPE	PURPOSE	AREA TREATED (ACRES)	LATITUDE (DECIMAL DEGREE)	LONGITUDE (DECIMAL DEGREE)	WATERSHED	RECEIVING WATERS	
o SWM Facility Proposed					Difficult Run		+
F	ITY POLYETHYLEN MEETS: TIME LIMI ES (PROPOSED O FACILITY TYPE	MEETS: TIME LIMITS ON APPLIC ES (PROPOSED ONLY) FACILITY TYPE PURPOSE	ITY POLYETHYLENE (HDPE) USED ON THIS MEETS: TIME LIMITS ON APPLICABILITY OF ES (PROPOSED ONLY) FACILITY TYPE PURPOSE AREA TREATED (ACRES)	ITY POLYETHYLENE (HDPE) USED ON THIS PROJECT MEETS: TIME LIMITS ON APPLICABILITY OF APPROVED ES (PROPOSED ONLY) GRANDFA FACILITY TYPE PURPOSE AREA TREATED (ACRES) LATITUDE (DECIMAL DEGREE) 0 SWM Facility	ITY POLYETHYLENE (HDPE) USED ON THIS PROJECT YES N MEETS: TIME LIMITS ON APPLICABILITY OF APPROVED DESIGN CRITE ES (PROPOSED ONLY) GRANDFATHERING CRITE FACILITY TYPE PURPOSE AREA TREATED (ACRES) LATITUDE (DECIMAL DEGREE) LONGITUDE (DECIMAL DEGREE)	ITY POLYETHYLENE (HDPE) USED ON THIS PROJECT YES NO NO MEETS: TIME LIMITS ON APPLICABILITY OF APPROVED DESIGN CRITERIA GRANDFATHERING GRANDFATHERING CR	ITY POLYETHYLENE (HDPE) USED ON THIS PROJECT YES NO NO VPDES PERMICABILITY OF APPROVED DESIGN CRITERIA MEETS: TIME LIMITS ON APPLICABILITY OF APPROVED DESIGN CRITERIA SWM FACILITIES DESIGNES SWM FACILITIES DESIGNES ES (PROPOSED ONLY) GRANDFATHERING CRITERIA SWM FACILITIES DESIGNES FACILITY TYPE PURPOSE AREA TREATED (ACRES) LATITUDE (DECIMAL DEGREE) WATERSHED RECEIVING WATERSHED 0 SWM Facility Image: Composition of the state of the

ROJECT DATA SHEET

TABLE 1. WATER QUALITY ANALYSIS PER VSMP TECHNICAL CRITERIA IIB / FFX CO STORMWATER MANAGEMENT ORDINANCE ARTICLE 4 **

RECEIVING WATERS	OUTFAL	L	TOT DISTUI ARI	RBED	PRE	DEVELOPMENT	LAND USE	POST	DEVELOPMENT	LAND USE	PHOSPHORUS REMOVAL REQUIRED *	ON-SITE PHOSPHORUS REMOVAL PROVIDED *
	ID	LOCATION	(AC)	(SF)	FORESTED (AC)	TURF (AC)	IMPERVIOUS AREA (AC)	FORESTED (AC)	TURF (AC)	IMPERVIOUS AREA (AC)	(LB/YR)	(LB/YR)
Wolftrap Creek	1,2		1.08	47,045	0.00	0.65	0.43	0.00	0.45	0.63	0.59	0.63

<u>NOTE:</u>

* PHOSPHORUS REMOVAL TO BE PROVIDED BY THE PURCHASE OF OFFSITE NUTRIENT CREDITS. PLEASE SEE TABLE 2 BELOW FOR MORE INFORMATION. ** TABLE HEADING SHOULD BE REVISED IF TECHNICAL CRITERIA 5 IS USED FOR GRANDFATHERED PROJECTS.

TABLE 2.

OFFSITE	COMPLIANCE	FOR	WATER	QUALITY	(NUTRIENT CREDITS	3)
			••••		(· /

NUTRIENT CREDIT BANK NAME	4TH ORDER HUC	NUTRIENT CREDIT TO BE ACQUIRED (LB/YR)	PURCHASE LETTER (MM/DD/YY) (3)
N/A	02070008	0.00	N/A

<u>-NOTE:</u> -3. ADDITIONAL INFORMATION WILL BE DOCUMENTED IN THIS TABLE UPON PURCHASE OF NUTRIENT CREDITS. PLEASE SEE LEDGER BELOW FOR

-EVIDENCE OF NUTRIENT CREDIT AVAILABILITY (RESERVATION)

SPACE BELOW RESERVED FOR EVIDENCE OF NUTRIENT CREDIT AVAILABILITY, WHEN NECESSARY

								Total Quanti	ty & Balance Re	mainig									
				1	This sprea	dsheet appl	ies ONLY to Projec	ts which d:	raw Nutrient	Credits fro	m FCDOT's	s Bulk Purc	hase.						
						Drav	v Down Qu	antity f	or Indiv	idual P	Projects	5							
											-	I	T		1				
Tracking #	Proj. #	Project Name	UPC # any)	Funding Source	Fund Number	Proejct Location 8 Digit HUC	Watershed Name	Phosphorous Removal Required (lbs/vr)	Cost/Project	TP Transferred (LB)	TP Balance (LB)	TN Retired (LB)	TN Balance (LB)	DEQ Permit #	Purchase Agreement Date	Date I Requested	Date of Credit Transfer	Comments	
2-1	2G40-088-012	Old Courthouse Road Pedestrian Improvem		C&I - Bike & Pedestrian Program	400-C40011	0207008	Middle Potomac – Catoctin (Difficult Run)		\$1,958.60	0.14	14.86	1.40	148.51	N/A	2/27/2017	3/6/2017	Pending		
																			SEE SHEET 2J(1) FOR LOCALIT
																			APPROVAL / ACCEPTANCE OI SWM STRATEGY
																			SWW STRATEGT
requ e limi		YES 🛛 NO					CERTIFIED E & PLAN REVIEWER												
REQU		YES 🛛 NO													EMER	GENCY	P	OLICE	- FIRE - RESCUE 911
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ECHNI	CAL CRIT	TERIA 5 🗆 (OLD)				NWW O	RCIN		2 M	RCIN						DEPARTI	MENT OF	F PUBLIC WORKS S, VIENNA, VA. 22180
\/ALIIIE	LENGTH /		BLDG.				ADAM WELSCHEN Lic. No. 04	D. IBACH 14359		ADAN WELSCHE Lic. No. (M D. ENBACH 044359								DEPARTMENT OF PUBLIC WORKS
CODE	AREA OF	/ UNIT NO. OF F (FT/ SERVED 7 SF) DISCON	TOP NECT)				ROFE			OF	A. C.								703-255-6380
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	+							• ·	┥┝─		· ▼ ▼ *	s =							PROJECT DATA SHEET
vided	by use of	Filterra tree	boxes				1		1 1			I ′ ⊢				i t		1	HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA
		Filterra tree	boxes.									0							
		<i>Filterra tree</i>	boxes.				Rinker Design A Manassas, V		Rink	er Design A Manassas	ssociates, P			DESCRIPTIC			APPROVEL	D DATI	SCALE DESIGNED BY: ADW.P.E. SH

Stormwater Detention Exception Request Approval





DEC 1 4 2018

Vanessa Aguayo, Project Manager Fairfax County Department of Transportation 4050 Legato Road, Suite 400 Fairfax, Virginia 22033-2895

Reference: Stormwater Detention Exception #8833-WSWD-001-1

Dear Ms. Aguayo:

Subject:

The referenced stormwater detention exception request has been received and reviewed for consistency with the Stormwater Management Ordinance (SWMO) Fairfax County Code section 124-6-1. Based on the justifications provided, the Director has determined that:

- i. The exception is the minimum necessary to afford relief;
- similar circumstances;
- imposed or self-created; and

condition:

ensure capacity adequacy for the 10-year storm event.

This exception approval in no way relieves you of any other County drainage requirements including adequacy of outfall and pro-rata share payments. Compliance with the SWMO, the Chesapeake Bay Preservation Ordinance, proffers and development conditions are also required.

Conditional Analysis: •Detailed analysis has been provided on Sheet 2K(4).

County of Fairfax, Virginia

To protect and enrich the quality of life for the people, neighborhoods and diverse communities of Fairfax County

Old Courthouse Road Pedestrian Access; 8833-FDOT-001-1; Tax Map No 029-3-((01))-0024A & 0028; Hunter Mill District

ii. Granting the exception will not confer any special privileges that are denied in other

iii. Exception requests are not based upon conditions or circumstances that are self-

iv. Reasonable and appropriate conditions shall be imposed as necessary upon any exception granted so that the intent of the Act and this Chapter are preserved.

Therefore, your request to grant a partial exception of the stormwater detention requirement of the SWMO (124-4-4-D), is hereby approved on December 4, 2018, subject to the following

Detailed outfall analysis for the existing closed conduit system shall be provided to

Department of Land Development Services 12055 Government Center Parkway, Suite 659 Fairfax, Virginia 22035-5503 Phone 703-324-1780 • TTY 711 • FAX 703-653-6678 www.fairfaxcounty.gov



Vanessa Aguayo, Project Manager 8833-WSWD-001-1 Page 2 of 2

This exception shall automatically expire, without notice, 24 months after the date of this letter, unless the subject plan has been approved.

Please ensure that a copy of this letter is made a part of the submitted plan.

If further assistance is desired, please contact Yosif Ibrahim, Senior Engineer III, Site Development and Inspections Division (SDID), at 703-324-1720 or yosif.ibrahim@fairfaxcounty.gov

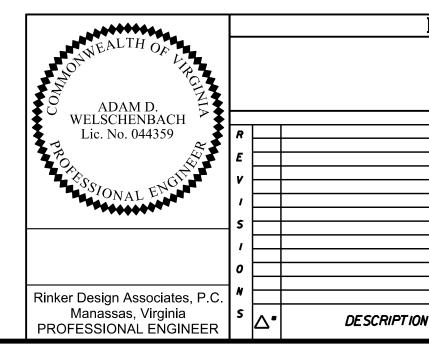
Sincerely,

· · · · ·

Spahabo Shahab Baig, P.E. Chief, North Branch

SDID Land Development Services (LDS)

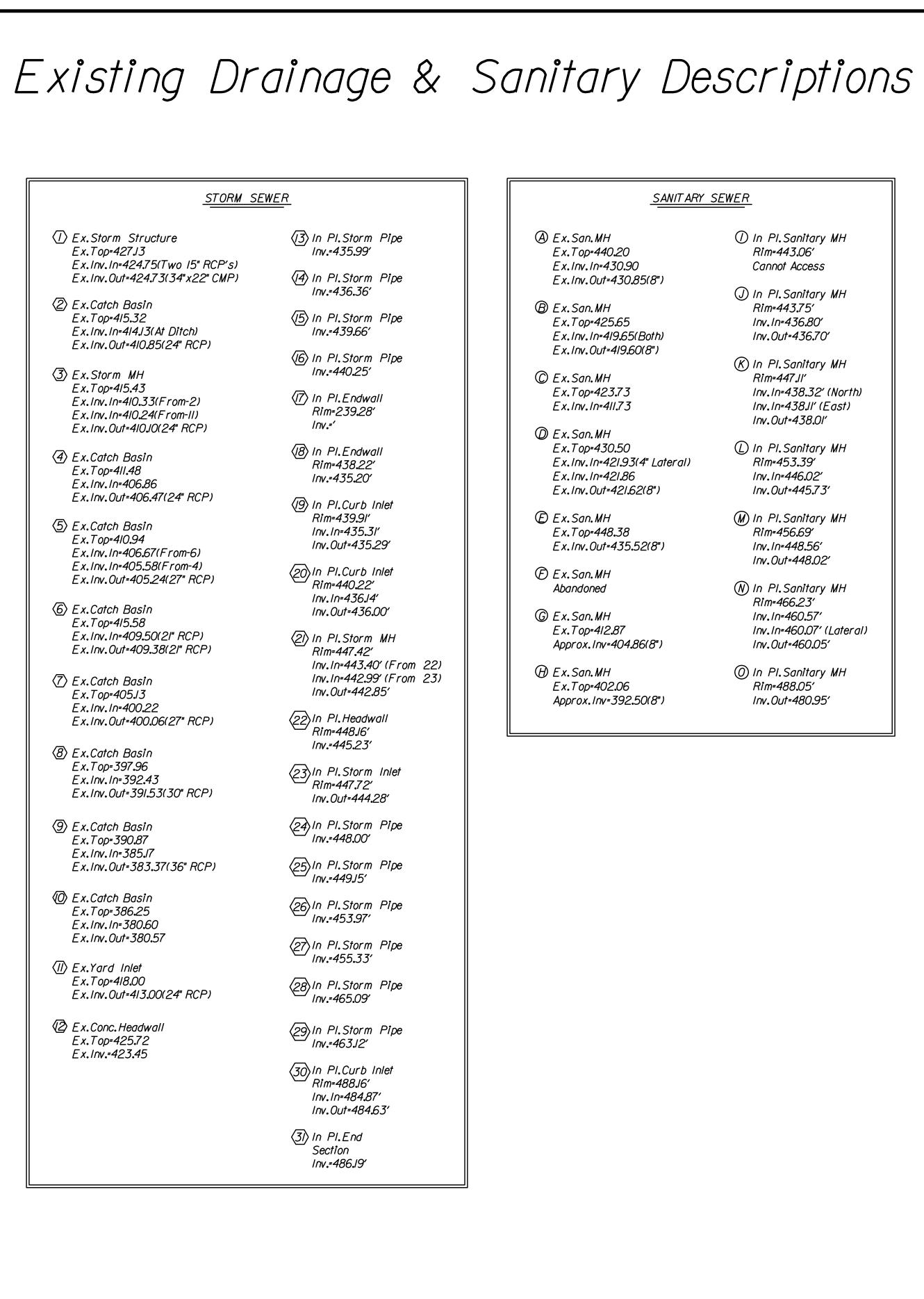
cc: Shannon Curtis, Chief, Watershed Assessment Branch, Stormwater Planning Division, Department of Public Works and Environmental Services Yosif Ibrahim, Senior Engineer III, SDID, LDS Waiver File





VI	enna /irginia –				Departmen	nt of Public V	Norks
					Micha	ael J. Gallagher Di	r, P.E. irector
	Vincinia Da			May 1, 2017			
	<u> </u>		-				
		ty Approval/Acc strian Enhanceme	-	WM Strategy (Joint	Town/County Projec	t – Old Courtl	house
3	system is o _] "General V	perated under a s	eparate peri ter Manager	nit from the State of nent Program (VSM	wn's limits and the T Virginia per requirem P) Permit for Dischar	nents of 4VAC	50-60,
	approximat facilities at sidewalk, s segments/J LF) within As part of driveway of designed to	tely 350' outside the other end of t storm sewer an ohases to be cons the Town of Vier the project's imp entrance improv	of the Town's r the Town's r d pedestria structed at th na. Phase 2 rovements a ements will	wn's northern limit, northern limits. The on facility improven he same time. Phase is all the improvem a six (6) foot concret be constructed. T	orth side of Old Cou through the Town entire project is appro- nents. The project is 1 is all the improven ents (approx. 350 LF) e sidewalk, storm sev he proposed improv ial properties and min	and tie to ex ximately 1900 s broken into nents (approx within the Co wer, and resid zements have	LF of LF of two 1550 punty. lential been
	Boxes) to be are designe	e constructed wit ed to handle imp	hin the proje pervious are	ect's limits for Phase	by the proposed BM 1. The Town confirms as required by Town rately.	s that these fac	cilities
	serves as co	oncurrence that t	he project's l		n & Design Hydrauli within the Town, as		
			Ŭ	*	389 or <u>Michael.Gallag</u>	<u>her@viennava</u>	a.gov.
-	Sincerely, Mudde Michael J. C	Gallagher, P.E.	L				
						TAX M	AP 29-3
MER				FIRE - RE			_, ,
		DEPARTME	NT OF P	NA, VIRGI PUBLIC WORKS VIENNA, VA. 3	}		
				DEPA	RTMENT OF P 703-255-6		ORKS
				PEDESTE SWM Detention	LD COURTHOU RIAN ACCESS on Exception a	IMPROV	EMENTS Ly Appro
					MILL DISTRICT, FAIR		
	BY	APPROVED	DATE	SCALE HORIZ= 1:25' VERT= N/A	DESIGNED BY: AD DRAFTED BY: LKG CHECKED BY: AD	G, JRB	SHEE 2J(I)
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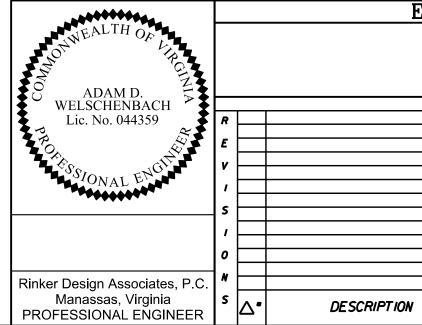
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- ① In Pl.Sanitary MH Rim=443.06' Cannot Access
- ⑦ In PI.Sanitary MH Rim=443.75' Inv.In=436.80' Inv.Out=436.70'
- (K) In Pl.Sanitary MH Rim=447,11′ Inv. In=438.32' (North) Inv.In=438.II' (East) Inv.Out=438.01'
- () In Pl.Sanitary MH Rim=453.39' Inv.In=446.02' Inv.Out=445.73'
- M In Pl.Sanitary MH Inv.In=448.56' Inv.Out=448.02'
- (N) In Pl.Sanitary MH Rim=466.23′ Inv.In=460.57' Inv.In=460.07′ (Lateral) Inv.Out=460.05'
- () In PI.Sanitary MH Rim=488.05′ Inv.Out=480.95'

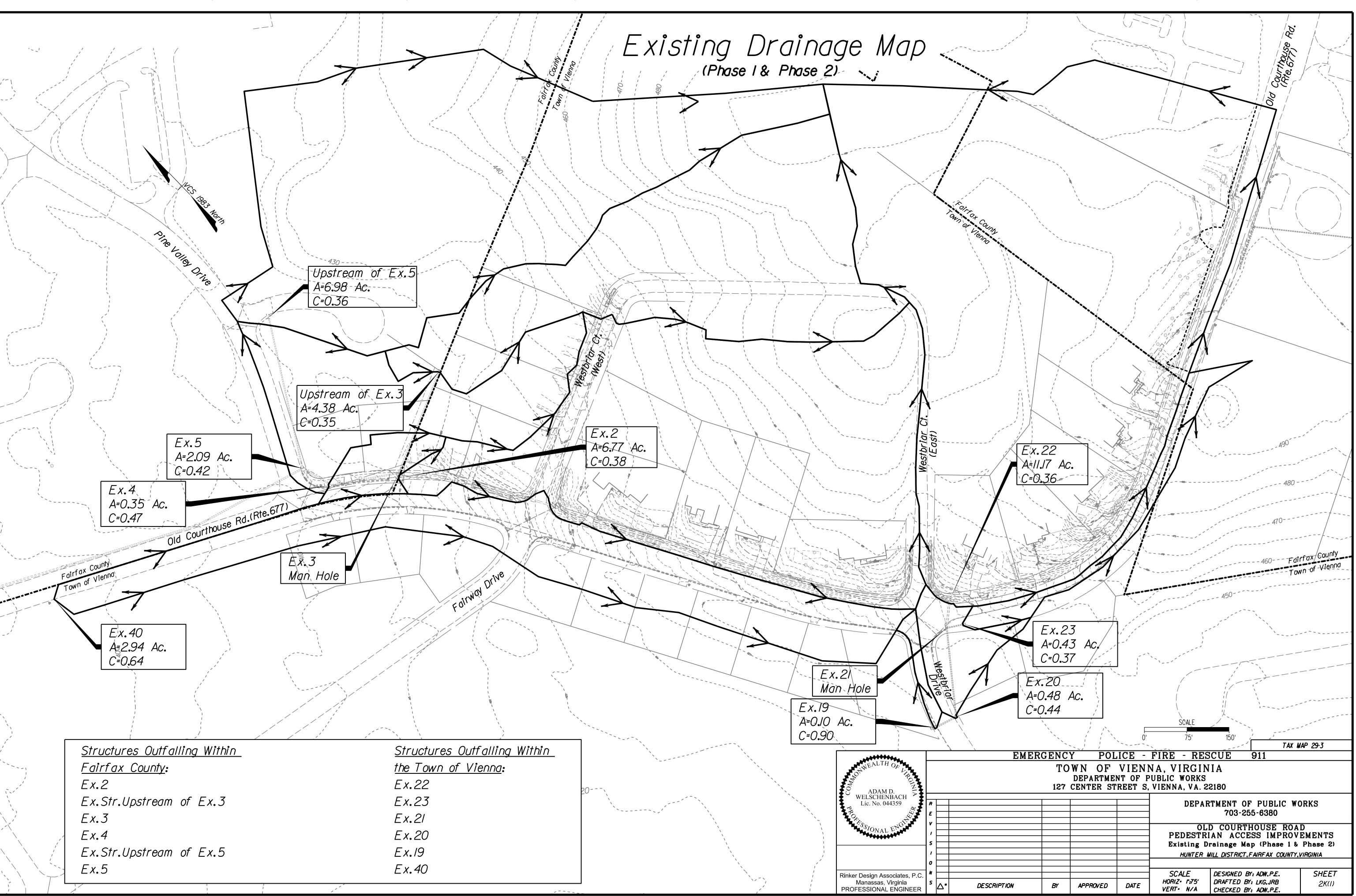
PHASE I

Sheet 3					
3-1	I St'd.DI-3BB Req'd. L=6' H=8.93' Inv.=415.08 Top=424.01 St'd.IS-I Req'd. I/2" Steel Plate Req'd. Connect UD-4 to DI	<i>4-2</i> A	I Filterra Tree Box (6' x 4') Inv.=442.59 Top=446J3 (1) 3' x 3' Tree Grate 6' - 4" SDR-35 PVC Outfall Pipe to Str.4-2 See Details,2L Series	5-I to 4-8	II9' - 15" Conc. Pipe Req'd.(3' Cover) (352' Radius with open joints - using 8' pipe joint lengths) Joints are to be opened a maximum of 25% of the spigot or tongue length. Inv.(in) 453.87 Inv.(out) 447.68
3-1 to Ex.2	196' - 18" Conc.Pipe Req'd.(7' Cover) (720' Radius with open joints - using 8' pipe joint lengths) Joints are to be opened a maximum of 25% of the spigot or tongue lengt Inv.(in) 415.08 Inv.(out) 411.05	4-2 to 4-1	215' - 15" Conc.Pipe Req'd.(3' Cover) (750' Radius with open joints - using 8' pipe joint lengths) Joints are to be opened a maximum of 25% of the spigot or tongue length. Inv.(in) 441,87 Inv.(out) 438.19	5-2 5-2A	Inv.[in] 453.87 Inv.[out] 447.68 I St'd.DI-2B Req'd. L=12' H=4.0' Inv.=466.69 Top=470.69 Connect UD-4 to DI Connect 6" SDR-35 PVC to DI I Filterra Tree Box (13' x 7') Inv.=469.39_ Top=472.93
3-2	I St'd.DI-3B Req'd. L=6' H=5.35' Inv.=421.16 Top=426.51 St'd.IS-I Req'd.	4-4	I St'd.DI-7 Req'd.Type III Grate Req'd. H=3.5' Inv.=442.85 Top=446.35		Inv.=469.39 Top=472.93 (2) 4' x 4' Tree Grate 13' - 6" SDR-35 PVC Outfall Pipe to Str.5-2 See Details.2L Series
3-2 to 3-1	Connect UD-4 to DI	4-4 to 4-2	36' - 15" Conc.Pipe Req'd.(3' Cover) Inv.(in) 442.85 Inv.(out) 441.97	5-2 to 5-1	(280' Radius with open joints
	38' - 15" Conc.Pipe Req'd.(4' Cover) Inv(in) 421J6 Inv(out) 419.78	4-6	St'd.ES-IA 9"x30" Req'd. Inv.447.00		- using 8' pipe joint lengths) Joints are to be opened a maximum of 25% of the spigot or tongue length. Inv,(in) 466.69 Inv,(out) 453.97
3-3	3.3 Lin.Ft.St'd.MH-I or 2 Req'd. I St'd.MH-I Frame & Cover Req'd. Inv.426.20 Top=430.20 St'd.IS-I Req'd.	4-6 to 4-7	32' - 19"x30" Elliptical Conc.Pipe Req'd.(1.5' Cover) Inv.(in) 447.00 Inv.(out) 445.40		
3-3 to 3-2	102' - 15" Conc. Pipe Req'd.(3' Cover) Inv.(in) 426.20 Inv.(out) 422.76	4-7	I St'd.DI-3C Req'd. L=8' H=3.6' Inv.=445.30 Top=448.92 I St'd.Monolithic Box Req'd. Less Than Minimum Height		PHASE 2
3-4	I St'd.ES-I I5" Req'd. Inv.429.50		See Detail Sheet 2K(8) St'd.IS-I Req'd.	5-3	I St'd.DI-3B Req'd. L=4' H=3.0' Inv.=485.30 Top=488.30
3-4 to 3-3	23' - 15" Conc.Pipe Req'd.(2' Cover) Inv.(in) 429.50 Inv.(out) 426.30	4-7 to Ex.21	Connect UD-4 to DI Existing Pipe To Be Extended with		l St'd.Monolithic Box Req'd. Less Than Minimum Height See Detail Sheet 2K(8)
3-5	I St'd.DI-3C Req'd. L=6' H=4.0' Inv.=423.J3 Top=427.J3		3' - 34" x 22" Conc.Pipe Req'd.(2' Cover) Inv.(in) 445.30 Inv.(out) 443.40	Sheet 6	Connect UD-4 to DI
3-5 to 3-2	Connect UD-4 to DI 32' - 15" Conc. Pipe Req'd.(2' Cover)	4-8	I St'd.DI-2B Req'd. L=IO' H=4.O' Inv.=447.58 Top=451.58 St'd.IS-I Req'd.	5-3 to 6-3	14' - 15" Conc. Pipe Req'd.(2' Cover) Inv.(in) 485.30 Inv.(out) 485.20
3-6	Inv (in) 423,13 Inv (out) 422.76		Connect UD-4 to DI Connect 6" SDR-35 PVC to DI	6-1	I St'd.DI-3C Req'd. L=6' H=2.59' Inv.=485.60 Top=488.19 I St'd.Monolithic Box Req'd.
50	I St'd.DI-3B Req'd. L=12' H=7.0' Inv.=422.68 Top=429.68 St'd.IS-I Req'd. Connect UD-4 to DI	<i>4-8</i> A	I Filterra Tree Box (12' x 6') Inv.=451.37 Top=454.91 (2) 4' x 4' Tree Grate	6.1 to 5-7	Less Than Minimum Height See Detail Sheet 2K(8) Connect UD-4 to DI
3-6 to 3-1	99' - 15" Conc.Pipe Req'd.(5' Cover) Inv.(in) 422.68 Inv.(out) 419.78		60' - 6" SDR-35 PVC Outfall Pipe to Str.4-8 See Details,2L Series	6-1 to 5-3 6-2	38' - 15" Conc. Pipe Req'd.(2' Cover) Inv.(in) 485.60 Inv.(out) 485.40 I St'd DI-38 Reg'd
3-7	I St'd.DI-3B Req'd. L=14' H=7.0' Inv.=430.08 Top=437.08 St'd.IS-I Req'd.	4-8 to 4-7 Sheet 5	79' - 15" Conc. Pipe Req'd.(2' Cover) Inv.(in) 447.58 Inv.(out) 445.44	U-Z	I St'd.DI-3B Req'd. L=4' H=2.5' Inv.=485.85 Top=488.35 I St'd.Monolithic Box Req'd. Less Than Minimum Height See Detail Sheet 2K(8) Connect UD-4 to DI
3-7 to 3-6	Connect UD-4 to DI I6I' - I5" Conc.Pipe Req'd.(3' Cover) Inv.(in) 430.08 Inv.(out) 425.78	5-1	I St'd.DI-2B Req'd. L=IO' H=4.O' Inv.=453.87 Top=457.87 St'd.IS-I Req'd. Connect UD-4 to DI Connect 6" SDR=35 PVC to DI	6-2 to 6-1 6-3	
Sheet 4 4-1	I St'd.DI-3B Req'd. L=8' H=4,0' Inv.=438.09 Top=442.09 St'd.IS-I Req'd. Connect UD-4 to DI	5-IA	Connect 6" SDR-35 PVC to DI I Filterra Tree Box (I2' x 6') Inv.=456.58 Top=460.I2 (2) 4' x 4' Tree Grate 33' - 6" SDR-35 PVC Outfall Pipe to Str.5-I		IJT CY.of EC-I Class Al Req'd. Type A Installation
4-1 to 3-7	207' - 15" Conc.Pipe Req'd.(3' Cover) Inv.(in) 438.09 Inv.(out) 433.18		See Details, 2L Series		
4-2	I St'd.DI-3B Req'd. L=6' H=4,0' Inv.=441,87 Top=445,87 St'd.IS-I Req'd. Connect UD-4 to DI Connect 4" SDR-35 PVC to DI				
	ſ	I	EMERGENCY PO	DLICE - F	TAX MAP 29-3 IRE - RESCUE 911
		NONWEALTH OF LIP	TOWN OF DEPARTM	F VIENNA IENT OF PUE	A, VIRGINIA BLIC WORKS
		ADAM D. WELSCHENBACH Lic. No. 044359	127 CENTER :		IENNA, VA. 22180 DEPARTMENT OF PUBLIC WORKS 703-255-6380
		CSSIONAL ENGINE	v		OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS Existing Drainage & Sanitary Descriptions, Proposed Drainage Descriptions HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA
		Rinker Design Associates, P.C.	S DESCRIPTION BY APPROVED	DATE	SCALEDESIGNED BY: ADW.P.E.SHEETHORIZ- N/ADRAFTED BY: LKG.JRB2KVERT- N/ACHECKED BY: ADW.P.E.2K



ge Descriptions

FUND*



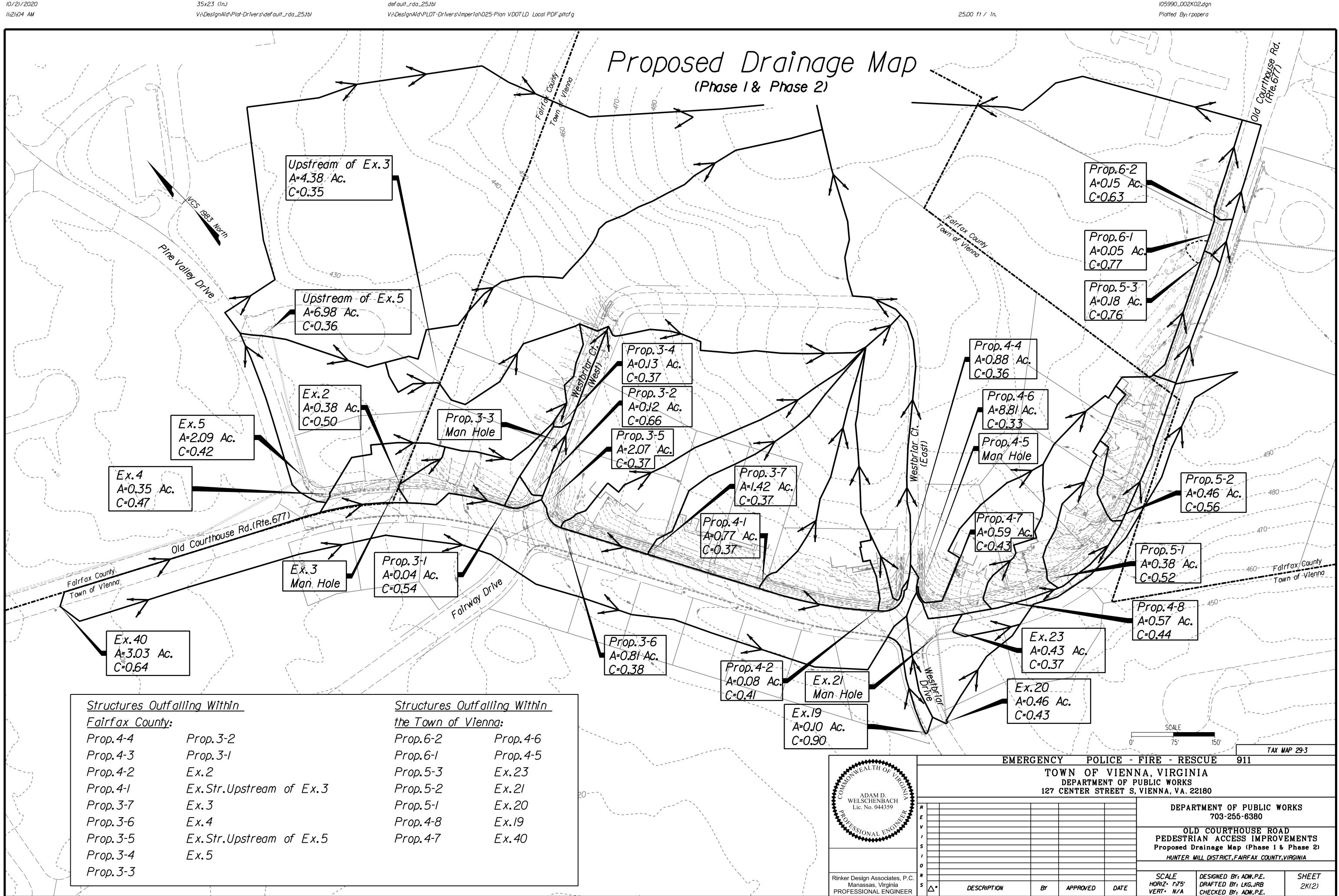
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35x23 (în.)

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10/21/2020

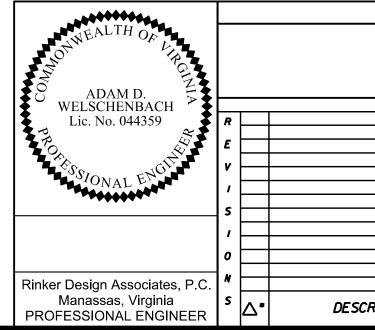
11**:**20**:**57 AM



<i>Number</i>	Inlet Inlet	Lenath (F1)		Station	Draiange Area (Ac)	J	CA	Sum CA		l (In/Hr)	a Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow		5 Gutter Slope (Ft/Ft)	Sx Cross Slope (Ft/Ft)	T (Spread) (Ft)	W (Gutter Width) (Ft)	W/T	Sw (Gutter Slope) (F1/F1)	Sw/Sx	Eo (App.9C-8)	0	S'W	Se	Computed	L, Specified	11/ I			u intercepted (CFS)	0b Carryover (CFS)	Depth at Curb (In)	Allowable Ponding Depth (F1)	Height of Curb Opening h (Ft)	ng Inlei u/p	Depth of Inlet	T Spread & SAG (F1)	Remarks		PRE-DEV INLET CC form ld-204 inlet computat route: Old Courth) M P U T	ATIC	NS
Ex	2 <u>YI-I</u>	4	1 12	2+18	6.77	0.38	2.57	7																																	DESIGNED BY:	ΙZ	DATE	5/06/2016
								2.5	57 4	.00	10.29	0.00) 10.2	29 0.	0400			4.00									4		1.	00 1	0.29	0.00	0.6					Bł	Ht .=0.3 5	56'	CHECKED BY:			ENGLISH

	Inlet																													Sag Inlei	's Only			POST -D	EVEI	LOPM	ΞNΊ	ר
Number	Type	Length (Ft)	Station	Draiange Area (Ac)	U	CA	Sum CA	I (In/Hr)	a incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (F1/F1)	Sx Cross Slope (Ft/Ft)	T (Spread)(Ft)	W (Gutter Width) (F1)	1/M	Sw (Gutter Slope) (F1/F1) Sw/Sx	Eo (App.9C-8)	Ø	S'w	Se	Computed Length (F1)	L, Specified Length (Ft)	רירד		0 Intercepted (CFS)	Ob Carryover (CFS)	Depth at Curb (In)	Allowable Ponding Depth (Ft)	Height of Lurb Opening h (Ft) d/h	Depth at Inlet (In)	T Spread @ SAG (F1)	Remarks	INLET FORM LD-204 INLET COMPUT ROUTE: Old Co	ATIONS			
3-1	DI-3BE	8 6	14.02.6	7 0.04	0.54	0.02																												DESIGNED BY:	JZ	DA	TE:	9/05/2017
							0.02	4.00	0.09	0.00	0.09	0.0100	0.0208	I . 16	2.00 1.7	197 ()	0.0833 4.0048	1	3.5	0,1458	0.1666	1.978	6	3.033	1.00	0.09	0.00	1.2						CHECKED BY:	NVD	UN	ITS:	ENGLISH
3-2	DI-3B	6	53•45.	<u>1 0,12</u>	0.66	0.08														<u> </u>														_				
7.5	01.70		E Z A Z E	4 105	0.77	070	0.08	4.00	0.32	0.00	0.32	0.0280	0.0520	1.58	1.50 0.9	95120	0.0833 1.6019		2.06	0,1146	0.1666	4,608	6	1.302	1.00	0.32	0.00	1.5	+		_	-	Deels (1 t	-				
5-5	DI-SC	6	53•47.5	4 1.95	0.37	0.72	072	4.00	289	0.00		0.0280	0.050	458	150 07	2780	0.0833 1.6333		2.08										+		-		Back/Lt. Back/Lt.					
				0,12	0.37	0.04	0.72	7.00	2.09	0.00		0.0200	0.00		1.000.0	2100	1.0000 1.00000		2.00														Ahead/Rt.	-				
							0.04	4.00	0.18	0.00																							Ahead/Rt.	-				
												0.0280											6			3.06		3.4	0.413 0	.4583 0.90	11 4.9	5.61	Weir Flow	1				
3-6	DI-3B	12	15+11.20	0.81	0.38	0.31																																
				_		_	0.3/	4.00	1.23	0.00	1.23	0.0720	0.0632	2.36	1 . 50 0.6	3690	0.0833 1.3180	0.947	1.86	0,1034	0,1611	11.052	12	1.086	1.00	1.23	0.00	2J			_			_				
3-7	DI-3B	14	16•75	1.42	0.37	0.53		4.00	2,10	0.00	2,10	0.0328	30.0200	6.56	1.50 0.2	287 0	0.0833 4.1650	0.645	2.64	0.1466	0 <i>J</i> 145	13.403	5 14	1.045	1.00	2.10	0.00	2.7						-				
4-1	DI-3B	8	<i>18•85₊0</i>	0 0.77	0.37	0.28											0077 (7776)		0.00	<u></u>		7.00									_			4				
							0.29	4.00]4	0.00]4	0.0116	0.0176	6.//	1.50 0.2	2214 ()	0.0833 4.7330	0.649	2.68	0,149	0,1143	7.601	8	1.052	1.00	,]4	0.00	2.6						-				
4-2	DI-3B	6	21.08,1	3 0,11	0.41	0.05		+		+	+	+				-+											+		+			+		-				
							0.05	4.00	0.18	0.00	0.18	0.0256	0.0688	1.28	1.50 1.1	765 0	0.0833 1.2108	1	1.76	0.0978	0.1666	3.541	6	1.694	1.00	0.18	0.00	1.3						1				
4-4	DI-7	2	02.33.	54 0.88	0.36	0.32																												4				
							0.32	4.00	1.27	0.00	1.27	0.0619			2.00								2		1.00	1.27	0.00	0.3					BHt.=0.139'	4				
<i>Ex2</i>	<u> </u>	4	12•18.20	0.38	0.5	0.19			076		076														<u> </u>	076			+									
							0.19	4	0.76	0	0.76	0.01			4								4			0.76	0	0.3	+			+	BHt.=0.062'					
L																																						

Storm Computations for Outfalls in Fairfax County



					TAX N	IAP 29-3
EMER	GENC	Y POL	ICE -	FIRE - RES	CUE 911	
		DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 22		
				DEPAR	TMENT OF PUBLIC W(703-255-6380	DRKS
				PEDESTRI Storm	D COURTHOUSE ROA AN ACCESS IMPROV Computations for Out in Fairfax County IILL DISTRICT.FAIRFAX COUNTY.	EMENTS falls
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(3)

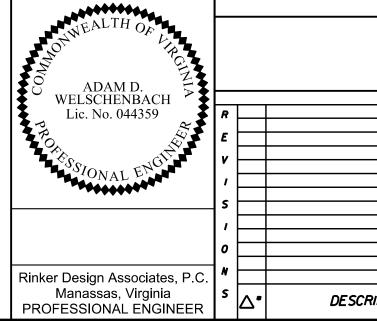
PRE-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

S'	ORM SEWER DES	IGN COMP	UTATIO	NS																	DESIG	NED E	3Y: <i>JZ</i>	,		DATE	:: I2	2/13/2018
S	ORM FREQUENCY	· [1	l0-Year]																	CHEC	KED B	Y: NV	'D		UNIT	S: <i>E</i>	NGLISH
Γ		From F	Point	To Po	nint	Drain		C	A	Total	Rain	Rui			levations	Length		Size	Shape	Number Capaci	yFriction	า	Norn	nal Flow			Flow	
	Pipe No.		Ct -		Ct -	Area "A"	Runoff Coeff.		Accum- ulated	Inlet Time	Fall			Upper End	Lower End	of Pipe		Dia.or Span/Rise		of Pipes	Slope		f Area of Flow, An	Hm	Vn	En	Time	Remarks
		Reference	Sta.	Reference	Sta.	"A" (Acre)	"C"	"""		(Minutes)	(In/Hr)	(CFS)	(CFS)		2/10	(Ft)	(Ft/Ft)	(In)	1	(CFS	(Ft/Ft)	(<i>F</i> †)	(SqFt)	(Ft)	(Ft/Sec.	;) (Ft)	(Sec)	
	Ex2toEx3	Ex2	12+18.20	Ex3	12•14,61	6.77	0.38	2.57	2.57	5.00	6.77	0.00	17.42	410 . 85	410.33	8	0.0650	24	Circular	1 57.68	0.0062	2 0.75	1.08	0.41	<i>16,</i>	4.77	0	
	Ex3toEx4	Ex3	12•14,61	Ex4	11•00.78	0.00		0.00	4,]]	16.25	4. 45	0.00	24.34	410 . 10	406.86	120	0.0270	24	Circular	I 37,17	0.0120	I . 18	1.93	0.55	12.6	3.65	10	
	Ex4toEx5	Ex4	11•00.78	Ex5	10•60.11	0.35	0.47	0,16	4.27	16.41	4.43	0.00	24.34	406.47	405.58	60	0.0148	24	Circular	1 27.55	0.0120	1.46	2.46	0.60	9. 9	2.98	6	
	Ex5toEx7	Ex5	10•60.11	<i>Ex</i> 7		2.09	0.42	0.88	6.5/	16.51	4.42	0.00	33.58	405,24	400.22	209	0.0240	27	Circular	/ 48.00	0.0122	1.39	2.57	0.63	13.1	4.04	16	

POST-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

STORM	SEWER DES	IGN COM	PUTATIO	NS																		DESIG	NED B	Y: <i>JZ</i>	•		DATE	;: /	12/13/2
STORM	FREQUENCY		10-Year]																		CHECK	KED BY	t: NV	D		UNIT	S: /	ENGLISH
		From	Point	To Po	pint	Drain		C	A	Total	Rain	Rui	nof f	Invert El	levations	Length	Slope	Size	Shape	Wumber	Capacity	Friction	,	Norn	nal Flow			Flow	Τ
	Pipe No.	Reference	Sta.	Reference	Sta.	Area "A" (Acre)	Runoff Coeff. "C"	Incre- ment	Accum- ulated	Inlet Time (Minutes)	Fall	Lateral (CFS)	Total Q (CFS)	Upper End	Lower End	of Pipe (Ft)	9	Dia.or Span/Rise	•	of Pipes		Slope (Ft/Ft)	Depth of Flow.dn	Area of Flow, An (SqFt)	Hm (Ft)	Vn (Ft/Sec.	En (Ft)	Time (Sec)	
4	4-4to4-2	4-4	102+33.54	4-2	21.08.13	0.88	0.36	0.32	0.32	5.00	6.76	0.00	2.14	442.85	441.97	26.00	p.03385	15	Circular	1	11.89	0.00110	0.36	0.29	0.21	7.34	1.20	3.54	
	4-2to4-1	<i>4-2</i>	21.08.13	4-1	18•85.00	0.08	0.41	0.03	0.35	5.06	6.74	0.00	2.36	441,87	438 . 19	215.00	0.01712	15	Circular	/	8.45	0.00/40	0.45	0 . 40	0.25	5.90	0.99	36.42	
	4-Ito3-7	4-1	18•85.00	3-7	16•75	0.77	0.37	0.29	0.63	5.67	6.55	0.00	4.23	438.09	433 . 18	207.00	0.02372	15	Circular	1	9.95	0.00450	0.57	0.54	0.29	7.78	1.51	26.62	
	3-7to3-6	3-7	<i>l6</i> •75	3-6	15+11.20	1.42	0.37	0.53	1,16	6,11	6.41	0.00	7.59	430.08	425.78	161.00	0.02671	15	Circular	/	10.56	0.01440	0.79	0 . 81	0.36	9.36	<i>2</i> , <i>1</i> 5	17.20	
	3-6to3-1	3-6	15•11.20	3-1	14•02.67	0.81	0.38	0.31	1.47	6.40	6.33	0.00	9.54	422.68	419.78	105.00	0.02762	<i>I</i> 5	Circular	1	10.74	0.02280	0.92	0 . 97	0.38	9.89	2.44	10.62	
	3-ItoEx2	3-1	14.02.67	Ex2	12+18.20	0.04	0.55	0.02	2.38	6.57	6.28	0.00	14.97	4/5.08	411.05	190.00	0.02121	18	Circular	1	15.30	0.02120	1.20	1.52	0.46	9 . 87	2.71	19.26	
E	x2toEx3	Ex2	12•18.20	Ex3	12•14.61	0.38	0.50	0,19	2.57	6.90	6.20	0.00	<i>16</i> , <i>1</i> 5	410.85	410.33	8.00	0.06500	24	Circular	1	57.68	0.00530	0.72	1.03	0.40	15.75	4.57	0.5/	
E	x3toEx4	Ex3	12•14,61	Ex4	11+00.78	0.00		0.00	4,	16.25	4.47	0.00	18.41	<i>410.10</i>	406.86	120.00	0.02700	24	Circular	1	37 .]7	0.00690	0.99	1.56	0.50	11 . 80	<i>3.</i> /6	<i>10,1</i> 7	
E	x4toEx5	Ex4	11+00.78	Ex5	10•60.11	0.35	0.47	0 . /7	4. 27	16.42	4.45	0.00	19,15	406.47	405.58	60.00	0.01483	24	Circular	1	27.55	0.00750	1.23	2.02	0.56	9.47	2.62	6.33	
E	x5toEx7	Ex5	10•60,11	<i>Ex</i> 7		2.09	0.42	0.88	7.66	16.53	4.43	0.00	33.97	405.24	400,22	209.00	p.02402	27	Circular	1	48.00	0.01250	1.40	2.60	0.64	13.09	4.06	15.97	
																									<u> </u>		<u> </u>		
	3-4to3-3	3-4	54•76	3-3	54•5/	0,13	0.37	0.05	0.05	5.00	6.77	0.00	0.33	429.50	426.30	23.00	0,13913	15	Circular	1	24.10	0.00000	0.0	0.05	0.07	6.91	0.84	3.33	
	3-3to3-2	3-3	54•51	3-2	53•45 . //	0.00		0.00	0.05	5.05	6.75	0.00	0.33	426.20	422.76	102.00	p.03373	15	Circular	1	11.86	0.00000	0.14	0.08	0.09	4.22	0.42	24.16	
	3-2to3-1	3-2	53 · 45,11	3-1	14•02.67	0J2	0.66	0.08	0.89	5.46	6.62	0.00	5 . 91	421 , 16	419.78	48.00	0.02875	<i>I</i> 5	Circular	1	10.95	0.00870	0.65	0.65	0.32	9.10	1.94	5.28	
	3-5to3-2	3-5	53•47.54	3-2	53•45,11	2.07	0.37	0.77	0.77	5.00	6.77	0.00	5,19	423,13	422.76	30.00	0.01233	15	Circular	1	7,17	0.00670	0.79	0.82	0.36	6.37	1.42	4.71	

Storm Computations for Outfalls in Fairfax County



105990_002K04**.**dgn Plotted By:rpapera

12/13/2018 ISH

Remarks	
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	_
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						TAX MAP 29-3	
EME	RGENC	Y POL	ICE -	FIRE - RE	SCUE 911		
		DEPARTME	NT OF H	NA, VIRGII Public works , vienna, va. :			
				DEPA	RTMENT OF PUE 703-255-638		
				PEDESTF Storn	D COURTHOUS NAN ACCESS IN n Computations in Fairfax Cou MILL DISTRICT, FAIRFAX	MPROVEMENTS for Outfalls inty	
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.F DRAFTED BY: LKG.JI CHECKED BY: ADW.F	RB 2K(4)	
			•		•	FUND)*

PRE-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

OKM LD	047																									
HYDRAUL	IC GRADE L	INE AN	ALYSIS																	DE	SIGNED	BY:	JZ	D	DATE: /2	2/13/2018
NCIDENCI	E PROBABIL	ITY [10-Year	• <u> </u>																CH	ECKED	BY:	NVD	L	JNITS: E	NGLISH
INLET		INVERT	DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	CTION L	055					Ad j.Ht	Inlet			Inlet	Top of MH	
OR	ST A.	EL.	OF FLOW	WATER	PIPE	DISCH.	PIPE	SLOPE,Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION	5174	OUTFLOW	OUTFLOW	SURF ACE	Do	Oo	Lo	(FT/FT)	Hf	Vo	Но	Vî	Vi*2/2g	0.35*MAX.	Angle	к	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(II)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
Ex7																								402.02		
Ex5	10•60 .]]	400.22	2.25	402.02	27	33.58	209	0.0123	2.568	13,1	0.662	9. 9	1.5	0.533	58 . 0	0.54	0.83	2.02	3.88	2.02	YES	1.01	3.58	406.63	410.44	0 . K .
Ex4	II+00 . 78	405.58	2.00	<i>407,18</i>	24	24.34	60	0.0121	0.726	9 . 9	0.380	12.6	2.5	0.865	41.0	0.43	1.06	2.30	0.73	2.30	YES	I . 15	1.88	409.06	410.98	<i>O.K.</i>
Ex3	12+14.61	406.86	2.00	409.06	24	24.34	120	0.0121	1.452	12.6	0.618	<i>I6</i> , <i>I</i>	4.0	1.405	75 . 0	0.62	2.50	4. 52	0.00	4.52	YES	2.26	3.71	412.77	4/5.43	0.K.
Ex2	12•18.20	410.33	2.00	412.77	24	17.42	8	0.0062	0.050	16 . 1	1.004	0.0	0.0	0.000	0.0	0.00	0.00	1.00	17.42	1.30	YES	0.65	0.70	413.47	414,13	0.K.

POST-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

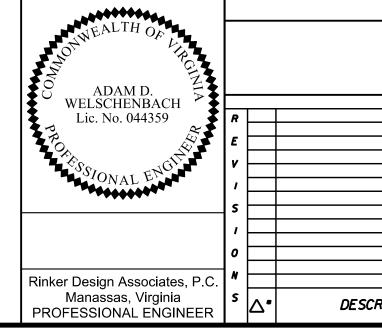
		C GRADE LINE ANALYSIS PROBABILITY <u>10-Year</u> JUNCTION LOSS																SIGNED ECKED		JZ NVD			12/13/2018 ENGLISH			
			DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	CTION L	055					Ad j.Ht	Inlet			Inlet	Top of MH	
INLET OR	STA.	EL.	of flow	WATER	PIPE	DISCH.	PIPE	SLOPE,Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION		OUTFLOW	OUTFLOW	SURF ACE	Do	Oo	LO	(FT/FT)	Hf	Vo	Но	Vî	Vi∗2/2g	0.35*MAX.	Angle	ĸ	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
301101 1011		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
Ex7																								402.02		
Ex5	10•60 , 11	400.22	2.25	402.02	27	33.971	209.00	0.01258	2.63	13.09	0.67	9.47	1.39	0.49	58.0	0.54	0.76	1.91	3.89	1.91	YES	0.95	3.58	406.64	410.44	<i>O.K.</i>
Ex4	// · 00 . 78	405.58	2.00	407 . 18	24	19,146	60.00	0.00749	0.45	9 . 47	0.35	11 . 80	2,16	0.76	41.0	0.43	0.93	2.03	0.73	2.03	YES	1.02	1.46	408.64	410.98	О.К.
Ex3	12•14,61	406.86	2.00	408.64	24	18.412	120.00	0.00692	0.83	II . 80	0.54	15.75	<i>3.</i> 85	1.35	75 . 0	0.62	2.40	4.29	0.00	4.29	YES	2,14	2.97	411,62	415.43	О.К.
Ex2	12•18.20	410.33	2.00	411.93	24	<i>16,148</i>	8.00	0.00533	0.04	15 . 75	0.96	9 . 87	1.51	0.53	59 . 0	0.55	0.83	2.32	I . 18	2.32	YES	I . 16	1.20	413,13	414,82	<i>O.K.</i>
3-1	14•02,67	411.05	1 . 50	413,13	18	14.971	190.00	0.02123	4.03	9. 87	0.38	9 . 89	1.52	0.53	40.0	0.42	0.54	1.45	<i>0,</i>]4	I .4 5	YES	0.72	4.76	417.89	423,18	<i>O.K.</i>
3-6	15+11.20	419.78	1.25	420.78	15	9.543	105.00	0.02281	2.40	9.89	0.38	9.36	1.36	0.48	4.0	0.06	0.08	0.93	1.95	1.21	YES	0 . 61	3.00	423.78	429,18	<i>O.K</i> .
3-7	<i>16</i> •75	425.78	1.25	426.78	/5	7.593	161.00	0.01444	2.33	9.36	0.34	7.78	0.94	0.33	0.0	0.00	0.00	0.67	3.37	0 . 87	YES	0.43	2.76	430.87	436.58	О.К.
4-1	18•85.00	433,18	1.25	434,18	15	4.226	207.00	0.00447	0.93	7.78	0.23	5.90	0.54	0.19	0.0	0.00	0.00	0.42	I . 87	0.55	YES	0.28	1.20	438.66	441.59	О.К.
4-2	21.08.13	438,19	1.25	439,19	15	2.361	215.00	0.00140	0.30	5 . 90	0.14	7.34	0.84	0.29	36.0	0.39	0.32	0.75	0.22	0.75	YES	0.38	0.68	442.32	445 . 87	О.К.
4-4	102•33.54	441.97	1.25	<i>442.</i> 97	15	2,140	26.00	0.00115	0.03	7.34	0.21	0.00	0.00	0.00	0.0	0.00	0.00	0.21	2.14	0 . 27	NO	0 . 27	0.30	443.27	446.35	О.К.
3-2	53•45,11	419.78	1.25	420.78	/5	5.913	48.00	0.00876	0.42	9,10	0.32	6.37	0.63	0,22	43.0	0.44	0.28	0.82	0.52	0.82	YES	0.41	0.83	421,81	426,16	О.К.
3-5	53•47.54	422.76	1.25	423.76	15	5,187	30.00	0.00674	0.20	6.37	0,16	0.00	0.00	0.00	0.0	0.00	0.00	0,16	5./9	0.20	NO	0 . 20	0.41	424,17	426.63	О.К.
3-3	54•51	422.76	1.25	423.76	15	0.326	102.00	0.00003	0.00	4.22	0.07	6.91	0.74	0.26	0.0	0.00	0.00	0.33	0.00	0.33	YES	0,16	0,17	426.34	430.20	О.К.
3-4	54•76	426.30	1.25	427.30	15	0.326	23.00	0.00003	0.00	6.91	0.19	0.00	0.00	0.00	0.0	0.00	0.00	0,19	0.00	0.19	NO	0,19	0.19	429.60	430.75	О.К.

PROPOSED OUTFALL 1A DITCH COMPUTATIONS FORM LD-268

													Earth					
					c	A							n=.025 (USGS-CL)				Depth	
STA	. TO STA.	FLOW	Area (Acres)	C-value	INCR.	ACC.	T _c	I ₂	Q ₂	C or F	Slope Ft/Ft	ALLOW. VEL.	VEL.	I ₁₀	Q ₁₀	Depth	Available I	
Old Co	urthouse Rd																	
Le	eft Side																	
31+12	31+12		0.40	0.70	0.28	0.28	5	5.23	1.46	C	0.0100	3.5	2.2	6.77	1.90	0.5	1.0	Design Ve

Storm Computations for Outfalls in Fairfax County

DESIGNED BY: AH CHECKED BY: SCT



105990_002K05**.**dgn Plotted By:rpapera

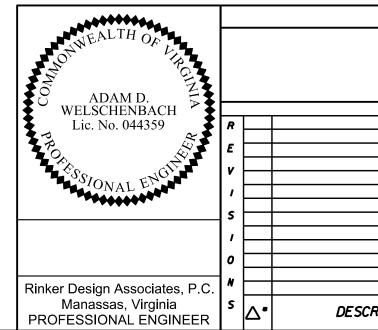


	DATE:	5/06/201	6				
-	UNITS	ENGLISH					
REMA	ARKS						
Velocity <	Allowable	e Velocity					
-	RGENCY		ICE -	FIRE - RES	SCUE	911	AP 29-3
						911	
	TO 127 (DEPARTMEN	NT OF F	NA, VIRGIN Public works Vienna, va. 2			
				DEPAR		F PUBLIC WC 55-6380	ORKS
				PEDESTR Storm	IAN ACCI Compute	HOUSE ROA ESS IMPROV tions for Out ax County FAIRFAX COUNTY.	EMENTS falls
							<u></u>
RIPTION	BY	APPROVED	DATE	SCALE HORIZ• N/A VERT• N/A	DESIGNED E DRAFTED B CHECKED B	Y: LKG.JRB	SHEET 2K(5)
							FUND*

Storm Computations for Outfalls in the Town of Vienna

										Т	Т							Т			T	Т		Т		T	Г	Т		T		. / . / . * .	Och			□ PRE-DEVELOPMENT	
	Inlet												0				છે														209	g Inlets	Uniy		4		
Number	Type	Length (Ft)	Station	Draiange Area (Ac)	ပ	СА	Sum CA	(JH/U])	0 Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (Ft/Ft)	Sx Cross Slope (F1/F1)	T (Spread)(F1)	W (Gutter Width (Ft)	W/T	Sw (Gutter Slope (F1/F1)	Sw/Sx	Eo (App.9C-8)	o	S'W	Se	Computed Length (Ft)	L. Specified Length (F1)	רירד	E (App.9C-18)	0 Intercepted (CFS)	Ob Carryover (CFS)	Depth at Curb (In)	ble Pon oth (Ft.	Height of Curb Opening h (Ft)	4/Р	Depth at Inlet (In)	T Spread @ SAG (Ft)	Remarks	INLET COMPUTATIONS FORM LD-204 INLET COMPUTATIONS ROUTE: Old Courthouse Road Pedestrian Access Improv	'ements
Ex20	DI-3B	8	99•90	0.48	0.44 (0.21																														DESIGNED BY: JZ DATE: 5/06/	2016
							0.21	4.00	0.84	0.00	0.84	0.020	00.020	0 4.61	1.50	0.3252	0.083	3 4.1650	0.811	2.64	0.1466	5 0,139	7.017	8	1,14	1.00	0.84	0.00	2.2								
																																				CHECKED BY: NVD UNITS: ENGL/S	
			T	 T	I	T								1	1	1	I	1	1			1	1				1			T			0-1		I	□ POST-DEVELOPMENT	
	Inlet																6														Sag	, Inlets	Only				
Number	Type	Length (Ft)	Station	Draiange Area (Ac)	ပ	СА	Sum CA	1 (IN/HL)	a Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (Ft/Ft)	Sx Cross Slope (Ft/Ft)	T (Spread)(Ft)	W (Gutter Width) (F1)	W/T	Sw (Gutter Slope) (F1/F1)	Sw/Sx	Eo (App.9C-8)	o	S'W	Se	Computed Length (Ft)	L. Specified Length (Ft)	רירד	E (App.9C-18)	0 Intercepted (CFS)	Ob Carryover (CFS)	Depth at Curb (In)	ble Por oth (Ft	Height of Curb Opening h (Ft)	4/Р	Depth at Inlet (In)	T Spread @ SAG	Remarks	INLET COMPUTATIONS FORM LD-204 INLET COMPUTATIONS ROUTE: Old Courthouse Road Pedestrian Access Improv	'ements
4-7	DI-3C	8	22.27	0.14	0.43 0	0.06																													Back/Lt.	DESIGNED BY: JZ DATE: 5/06/	2016
			-19 . 850'L				0.06	4.00	0.24	0.00																									Back/Lt.	CHECKED BY: NVD UNITS: ENGL/S	SH
				0.45	0.43 (0.19																													Ahead/Rt.		
							0.19	4.00	0.78	0.00		0.025	2 0.0217	7 3.90	1.50	0.3842	0.0833	3.84		2.61															Ahead/Rt.		
											1.02	0.025	2											8			1.02		1.4	0.21	0.02	12.00	2.9	5.50	Weir Flow		
4-8	DI-2B	10	23.04	0.57	0.44 0	.25																															
			-19.500°L				0.25	4.00	1.00	0.00	1.00	0.0100	0.020	8 5,16	2.00	0.3875	0.0833	3 4.00	0.88	3.50	0.1458	3 0.1484	5.89	2	1.70	1.00	1.00	0.00	2.8								
5-1	DI-2B	10	24.30.32	0.38	0.52 0).20																															
			-19 . 850′L				0.20	4.00	0.79	0.00	0.79	0.0100	0.020	8 4.37	2.00	0.4575	0.083	3 4.00	0.93	3.50	0,1458	B 0 . /559	5,18	2	1.93	1.00	0.79	0.00	2.6								
5-2	DI-2B	12	25.99.96	0.46	0.56 0	.26																															
			-19 . 850′L				0.26	4.00	1.03	0.00	1.03	0.0100	0.020	8 5.25	2.00	0.3807	0.083	3 4.00	0.87	3.50	0,1458	B 0J475	5.98	2	2.01	1.00	1.03	0.00	2.8								
5-3	DI-3B	4	31+12	0.01	0.45 0	0.00																															
			20.3301	0.14	0.90 (0,13																															
					0.30 (
							0,14	4.00	0.56	0.00	0.56	0.006	00.044	2 3.03	2.00	0.6607	0.0833	<i>1.88</i>	0.97	2.94	0.1224	4 0.1629	3.74	4	1.07	1.00	0.56	0.00	2.5								
6-1	DI-3C	6	31.54.67	0.02	0.90 0	0.02																													Back/Lt.		
	<u> </u>		20.3301																								1								Back/Lt.		
			1 I	 			002		0.00	1	1		10010		1	1	1	1	1	1	1	1	1		i	1	1	-	1	+ +				i	Peak /1+		

														-		- A															g iniers	5 Uniy	-		
Number	Type	Length (Ft) Station	Draiange Area (Ac)	J	CA	Sum CA	1 (In/Hr)	0 Incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (Ft/Ft)	Sx Cross Slope (F1/F1)	T (Spread)(Ft)	W (Gutter Width) (F1)	W/T	Sw (Gutter Slope) (F1/F1)	Sw/Sx	Eo (App.9C-8)	Ø	N,S	Se	Computed Length (Ft)	L. Specified Length (Ft)	דערד	E (App.9C-18)	0 Intercepted (CFS)	db Carryover (CFS)	Depth at Curb (In)	Allowable Ponding Depth (Ft)	Height of Curb Opening h (Ft)	ч/р	Depth at Inlet (In)	T Spread @ SAG (Ft)	Remarks	INLET COMPUTATIONS FORM LD-204 INLET COMPUTATIONS ROUTE: Old Courthouse Road Pedestrian Access Improvements
Ex20	DI-3B	8 99•9	0.48	0.44	0.21																														DESIGNED BY: JZ DATE: 5/06/2016
						0.21	4.00	0.84	0.00	0.84	0.0200	0.0200	4.61	1.50	0.3252	0.0833	4. 1650	0.811	2.64	0.1466	0,139	7.017	8	1,14	1.00	0.84	0.00	2.2							CHECKED BY: NVD UNITS: ENGLISH
Number	Type Type	Length (F1) Station	Draiange Area (AC)	J	CA	Sum CA	l (In/Hr)	a incr.(CFS)	Oc Carryover (CFS)	OT Gutter Flow (CFS)	S Gutter Slope (F1/F1)	Sx Cross Slope (Ft/Ft)	T (Spread) (Ft)	W (Gutter Width) (Ft)	W/T	Sw (Gutter Slope) (Ft/Ft)	Sw/Sx	E0 (App.9C-8)	Ø	S'W	Se	Computed Length (F1)	L. Specified Length (Ft)	רירד	E (App.9C-18)	0 Intercepted (CFS)	Ob Carryover (CFS)	Depth at Curb (In)	lowable Ponding Depth (F1)	Height of Curb Opening h (Ft)	ng Inlets	Depth at Inlet klu	Spread & SAG	Remarks	POST-DEVELOPMENT INLET COMPUTATIONS FORM LD-204 INLET COMPUTATIONS ROUTE: Old Courthouse Road Pedestrian Access Improvements
																													4						
4-7	DI-3C		97 <i>0,</i> ,,4	0.43	0.06																													Back/Lt.	DESIGNED BY: JZ DATE: 5/06/2016
	_	- <i>19.85</i> 0		0.47		0.06	4.00	0.24	0.00									<u> </u>																Back/Lt.	CHECKED BY: NVD UNITS: ENGLISH
			0.45	0.45	0.19	0.19	4.00	079	0.00		0.0250	2 0.0217	3.00	150	0 2010	0.0977	2 201		2.61										_					Ahead/Rt. Ahead/Rt.	
	-					0.19	4.00	0.70	0.00		0.0252		5.90	1.50	0.3042	0.0855	5.04		2,0/		-		8			1.02	-	1.4	0.21	0.02	1200	2.9	5.50	Weir Flow	_
4-8	DI-2B	10 23.0	4 0.57	044	0.25					1.02	0.02.52	•											0			1.02		7,4	0.21	0.02	12.00	2.5	5.50		
40		-19.500		0.77	0.25	0.25	4.00	1.00	0.00	100	0.0100	0.0208	5/6	200	0 3875	0.0833	4 00	0.88	3 50	0.1458	01484	5.89	2	1.70	1.00	1.00	0.00	2.8	-						—
		13.500					7.00	1.00		1.00	0.0700	0.0200	2.0	2.00	0.5075	0.0000	/ 7.00		5.50	0.150		5.05		1.10	1.00	7.00		2.0	-		1	+			
5-1	DI-2B	10 24.30.	32 0.38	0.52	0.20																								_						—
		-19.850				0.20	4.00	0.79	0.00	0.79	0.0100	0.0208	4.37	2.00	0.4575	0.0833	4.00	0.93	3.50	0,1458	0,1559	5.18	2	1.93	1.00	0.79	0.00	2.6							
5-2	DI-2B		96 0.46	0.56	0.26																														
		-19,850				0.26	4.00	1.03	0.00	1.03	0.0100	0.0208	5.25	2.00	0.3807	0.0833	5 4.00	0.87	3.50	0,1458	0,1475	5.98	2	2.01	1.00	1.03	0.00	2.8							
5-3	DI-3B	4 31+12	<u> 0.0</u> /	0.45	0.00																														
		-20.33	0′L 0,14	0.90	013																														
			0.03	0.30	0.01																														
						0.14	4.00	0.56	0.00	0.56	0.0060	0.0442	3.03	2.00	0.6607	0.0833	<i>1.88</i>	0.97	2.94	0,1224	0,1629	3.74	4	1.07	1.00	0.56	0.00	2.5							
6-1	DI-3C		67 0.02																															Back/Lt.	
		-20.33	01 0.01	0.30	0.00																													Back/Lt.	
						0.02	4.00	0.08	0.00		0.0044	0.0486	1.33	2.00	1.5004	0.0833	1,71		2.83															Back/Lt.	
			0.02	0.90	0.02																													Ahead/Rt.	
						0.02	4.00	0.07	0.00																									Ahead/Rt.	
										0,16	0.0044	4										<u> </u>	6			0,16		0.4	0.46	0.04	12.00	2.4	0.76	Weir Flow	
6-2	DI-3B		68 0J5	0.63	0.09													<u> </u>									<u> </u>				<u> </u>				
		-20.33				0.0	4.00	0.38	0.00	0.38	0.0112	0.0398	I . 97	2.00	1.0157	0.0833	2.09	1.00	3.04	0,1268	0.1666	3.78	4	1.06	1.00	0.38	0.00	2.0	_						
Ex20	DI-3B	8 99•89.	57 0.46	0.43	0.20																							-			<u> </u>	 			
						0.20	4.00	0.79	0.00	0.79	0.0200	0.0200	4.44	1.50	0.3378	0.0833	5 4.165 0	0.827	2.64	0.1466	0,1413	6.764	8	1,183	1.00	0.79	0.00	2.2							



					TAX	IAP 29-3
EMER	GENC	Y POL	ICE -	FIRE - RES	CUE 911	
		DEPARTME	NT OF P	NA, VIRGIN Public works Vienna, va. 22		
				DEPAR	TMENT OF PUBLIC W 703-255-6380	ORKS
				PEDESTRI Storm	O COURTHOUSE ROA AN ACCESS IMPROV Computations for Out n the Town of Vienna ILL DISTRICT.FAIRFAX COUNTY.	EMENTS Ifalls
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW,P.E. DRAFTED BY: LKG,JRB CHECKED BY: ADW,P.E.	SHEET 2К(6)

PRE-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

5	STORM SEWER DES	SIGN COMF	PUTATIO	NS																		DESIG	NED E	3 Y : <i>JZ</i>			DATE	: 5	5/06/20
S	STORM FREQUENC	Y [10-Year]																		CHECH	KED B	Y: NV	D		UNITS	S: <i>E</i>	ENGLISH
ſ		From H	Point	To P	oint	Drain		C	A	Total	Rain	Rur	nof f	Invert E	levations	Length	Slope	Size	Shape	Number	Capacity	Friction	,	Norn	nal Flow			Flow	
	Pipe No.				1	Area	Runoff Coeff.	Incre- ment	Accum- ulated	Inlet	Fall	Lateral		Upper End	Lower End	of Pipe		Dia.or Span/Rise		of Pipes		Slope	Depth of	Area of		Vn	En	Time	Rei
		Reference	Sta.	Reference	Sta.	(Acre)	"C"			(Minutes)	(In/Hr)	(CFS)	(CFS)			(Ft)	(Ft/Ft)			i ipes	(CFS)	(Ft/Ft)	(Ft)	n Flow, An (SqFt)	(Ft)	(Ft/Sec)	(F†)	(Sec)	
ľ	Ex22toEx21	Ex22	22•22.97	Ex21	22.10.51	<i>II.J</i> 7	0.36	4.02	4.02	15.30	4.58	0.00	18.41	445.23	443.40	47	0.0389	34 X 22	Elliptical	1	62.26	0.0034	0.69	1.40	0.45	13.3	3.44	4	
[Ex2ltoEx20	Ex21	22+10.51	Ex20	99•89.57	0.00		0.00	4 J8	15.36	4.57	0.00	<i>19.48</i>	<i>442.8</i> 5	436 , 14	158	0.0425	21	Circular	1	<i>32.</i> 65	0.0157	0.97	1.38	0.47	14.2	4.09	//	
	Ex20toEx19	Ex20	99•89.57	Ex19	99•90.18	0.48	0.44	0.21	4.39	15.55	4.54	0.00	19 . 96	436.00	435.31	40	0.0173	24	Circular	1	29.71	0.008/	1.20	1.97	0.56	I.O.I	2.80	4	
	Exl9toExl8	Ex19	99•90.18	Exl8	99•90.37	010	0.90	0.09	4.48	15.61	4.54	0.00	20.32	435.29	<i>435.20</i>	8	0.0113	24	Circular	1	24.00	0.0084	1.41	2.37	0.59	8.6	2.55	1	

POST-DEVELOPMENT STORM COMPUTATIONS, 10-YEAR STORM FORM LD-229

STORM	SEWER DES	IGN COMF	UTATIO	NS																		DESIG	NED B	Y : <i>JZ</i>	•		DATE		9/29/20
STORM	I FREQUENCY		10-Year]																		CHECK	ED B	r: NV	'D		UNIT	S:	ENGLISH
		From I	Point	To P	oint	Drain		С	A	Total	Rain		nof f	Invert E	levations	Lenath	Slope	Size	Shape	Number	Capacit	Friction		Norn	nal Flow			Flow	
	Pipe No.	Reference	Sta.	Reference	Sta.		Runoff Coeff. "C"	Incre- ment	Accum- ulated)- Inlet 1 Time (Minutes)(1	Fall	Fall Lateral	Total Q (CFS)	Upper End	Lower End	Lower of Pipe End (Ft)	Slope Size Dia.or Span/Rise (Ft/Ft) (In)		of Pipes		Slope	Deoth of	Area of Flow, An (SqFt)	Hm (Ft)	Vn (Ft/Sec.	En (Ft)	Time (Sec)	Rem	
	6-2106-1	6-2	31.81.68	6-1	31.54.67	0.15	0.63	0.0	0.10	5.00	6.76	0.00	0.64	485.95	485.70	24	0.0104	15	Circular	1	6.59	0.0001	0.26	0.19	0.16	3.4	0.44	7	
	6-Ito5-3	6-1	31+54.67	5-3	31+12	0.05	0.78	0.04	0.13	5.12	6.72	0.00	0.90	485.60	485.40	38	0.0053	15	Circular	1	4.69	0.0002	0.37	0.31	0.21	3.0	0.5/	13	
	5-3to6-3	5-3	31+12	6-3	31+12	0.18	0.78	0.14	0.27	5.33	5.09	0.00	1.62	485.30	485.20	4	0.0071	15	Circular	1	5.46	0.0007	0.47	0.42	0.25	3.9	0.70	4	
	5-2to5-I	5-2	25.99.96	5-1	24.30.32	0.46	0.56	0.26	0.26	5.00	6.77	0.00	1.75	466.69	453.97	158	0.0805	15	Circular	/	18.33	0.0008	0.26	0.19	0.16	9.4	1.64	17	
	5-1to4-8	5-1	24.30.32	4-8	23•04	0.38	0.52	0.20	0.46	5 . 28	6.68	0.00	3.07	<i>453.</i> 87	447,68	9	0.0520	15	Circular	/	14.73	0.0024	0.39	0.32	0.22	9.5	1.78	13	
	4-8to4-7	4-8	23.04	4-7	22•22.97	0.57	0.44	0.25	0.71	5.49	6.61	0.00	4.73	447.58	445.44	79	0.0271	15	Circular	/	10.63	0.0056	0.58	0.56	0.30	8.4	1.68	9	
	4-6to4-7	4-6	22•00	4-7	22•22.97	8.81	0.33	2.89	2.89	16.90	4.37	0.00	12.62	445.60	445.40	28	0.0071	24	Circular	/	19,12	0.0032	1.19	1.94	0.55	6.5	1.84	4	
	4-7toEx21	4-7	22+22.97	Ex21	22+10.51	0.59	0.43	0.25	<i>3.</i> 85	16.97	4.36	0.00	21.48	445.30	443.40	50	0.0380	34 X 22	Elliptical	/	61.51	0.0047	0.76	1.59	0.49	13.8	3.71	4	**
E	Ex2ltoEx20	Ex21	22+10.51	Ex20	99•89.57	0.00		0.00	4.01	17.03	4.36	0.00	22 , 17	442,85	436,14	158	0.0425	21	Circular	/	32.65	0.0204	1.06	1.52	0.49	14.6	4.36		
E	Ex20toEx19	Ex20	99•89.57	Ex19	99•90.18	0.46	0.43	0.20	4.20	17.23	4.33	0.00	18.37	436.00	435.31	40	0.0173	24	Circular	/	29.71	0.0069	1,14	I . 85	0.54	10.0	2.68	4	
E	Exl9toExl8	Ex19	99•90.18	Exl8	99•90.37	010	0.90	0.09	4.29	17.30	4.33	0.00	18.75	435.29	435.20	8	0.0113	24	Circular	1	24.00	0.0072	1.33	2.22	0.58	8.4	2.44	1	

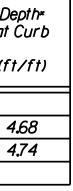
CURB & GUTTER OUTFALL SPREAD COMPUTATIONS

Outfall	Outfall Location (Station)	Drainage Area (ac)	Cw	Peak Flow,Q (cfs)	Channel Slope (ft/ft)	Gutter Cross Slope (ft/ft)	Road Cross Slope (ft/ft)	Spread* (See Note I) (ft/ft)	Deµ at C (ft/
Outfall #2	Old Courtho	use Road							
Pre-Developed	5•75 RT	2.94	0.637	12.70	0.0270	0.0833	0.0200	14.76	4.6
Post-Developed	5•75 RT	3.03	0.645	13.22	0.0270	0.0833	0.0200	14.99	4.

* Values derived using Bentley FlowMaster V8i.

Note I: Allowable Spread at Outfall *2 = 1/2 Driving Lane • On-Street Parking Width • Gutter Width = 8' • 6' • 1.5' = <u>15.5'</u>

Storm Computations for Outfalls in the Town of Vienna



DESIGNED BY: JZ CHECKED BY: NVD

DATE: 5/06/2016 UNITS: ENGLISH

ADAM D. WELSCHENBACH Lic. No. 044359 Rinker Design Associates, P.C. Manassas, Virginia PROFESSIONAL ENGINEER · 🛆 • DESCRI

5/2016 .ISH

Remarks

9/29/2016

Remarks

** Note:

4-7 to Ex.21 is an existing pipe being extended.Velocity of existing pipe Ex. 22 to Ex.21 greater than 10 fps in pre-development conditions.

					TAX	MAP 29-3				
EME	RGENC	Y POL	- JUCE -	FIRE - RE	SCUE 911					
		DEPARTME	NT OF F	NA, VIRGIN Public works , vienna, va. 2						
				DEPA	RTMENT OF PUBLIC W 703-255-6380	ORKS				
	OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEME Storm Computations for Outfalls in the Town of Vienna HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINI									
				SCALE	DESIGNED BY: ADW.P.E.	SHEET				
RIPTION	BY	APPROVED	DATE	HORIZ• N/A VERT• N/A	DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	2K(7)				
						FUND				

Storm Computations for Outfalls in the Town of Vienna

PRE-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

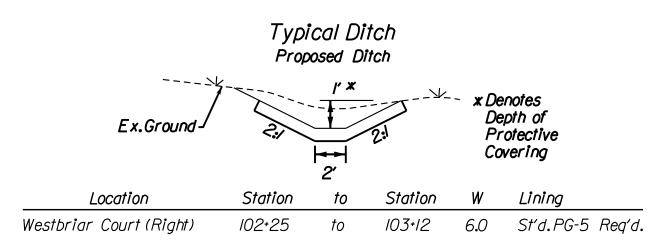
	IC GRADE L	_		_																	SIGNED		JZ			/06/2016
INCIDENC	E PROBABIL	ITY L	<u> 10-Year</u>	,																CH	ECKED	BY:	NVD	U	INITS: E	NGLISH
INLET		INVERT	DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	ICTION L	OSS					Ad j.Ht	Inlet			Inlet	Top of MH	
OR	STA.	EL.	OF FLOW	WATER	PIPE	DISCH.	PIPE	SLOPE.Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION		OUTFLOW	OUTFLOW	SURF ACE	Do	Qo	LO	(FT/FT)	Hf	Vo	Но	Vi	Vi*2/2g	0.35*MAX.	Angle	κ	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
JUNCTION		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(V12/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
Exl8																								436.80		
Ex19	99+90,18	435.20	2.00	436.80	24	20.32	8	0.0084	0.067	8.6	0 . 285	IO J	1.6	0.559	3.0	0.05	0.07	0.92	0.41	0.92	YES	0.46	0.53	437.33	439.41	<i>O.K.</i>
Ex20	99 • 89 . 57	435.31	2.00	437.33	24	19 . 96	40	0.008/	0.325	IO , I	0.399	14.2	31	1.091	69.0	0.60	I . 87	3.36	0.96	3.36	YES	1.68	2.00	439.33	439.72	<i>O.K.</i>
Ex21	22+10.51	436,14	1.75	439.33	21	19 . 48	158	0.0/58	2.497	14.2	0.780	13.3	2.7	0.961	12.0	0,15	0.42	2 . /6	0.00	2 . 16	YES	1.08	3.57	443.82	447.42	<i>O.K.</i>
Ex22	22•22.97	443.40	1.83	444.87 3	4 X 2.	2 18.41	47	0.0034	0.160	13.3	0.686	0.0	0.0	0.000	0.0	0.00	0.00	0.69	0.00	0.69	NO	0.69	0.85	445.92	448. I6	0.K.

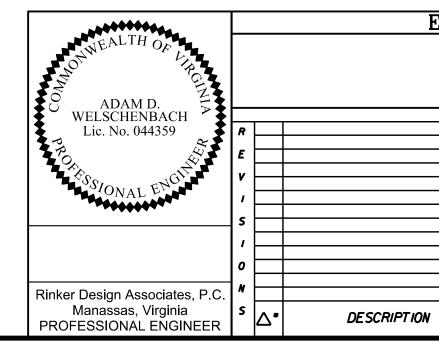
POST-DEVELOPMENT HGL COMPUTATIONS FORM LD-347

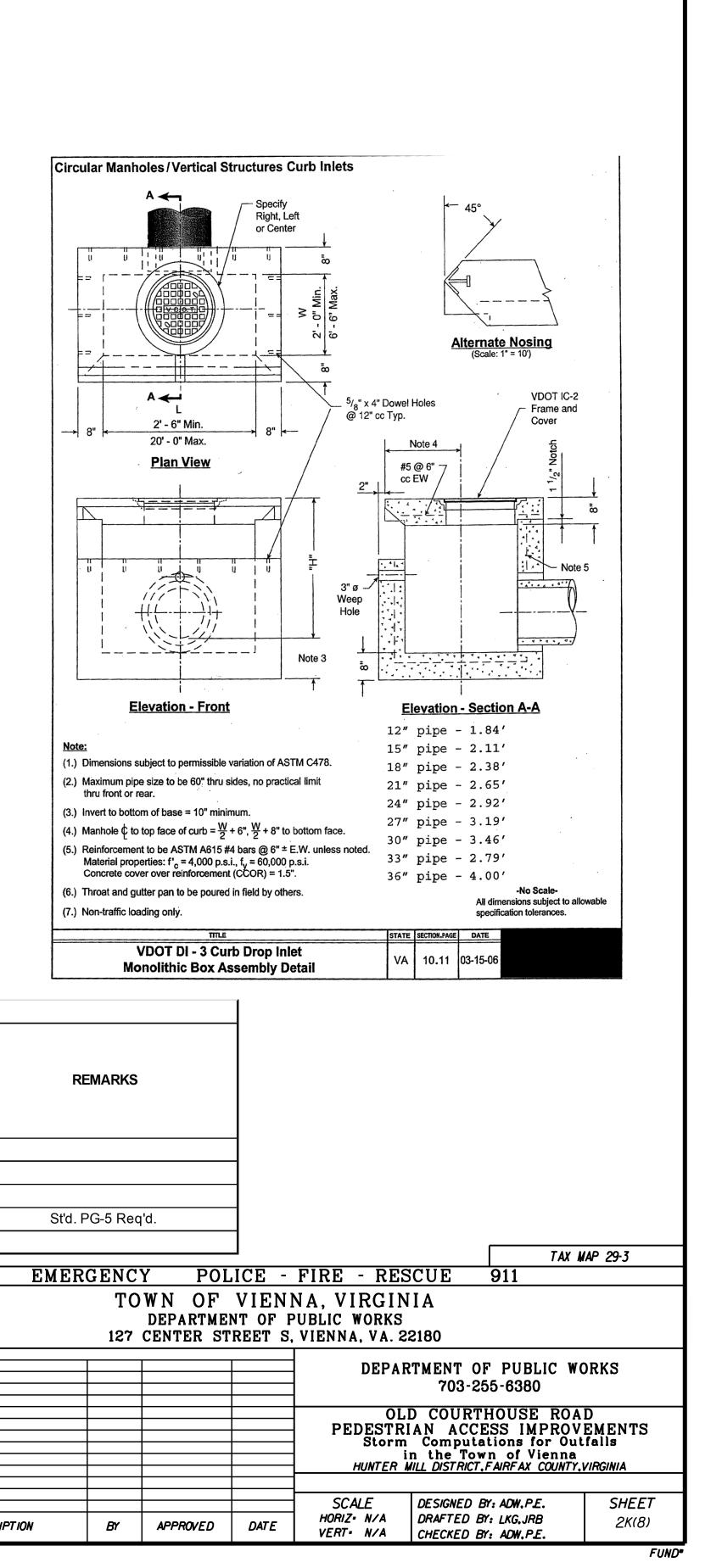
HYDRAUL	IC GRADE I	LINE AN	ALYSIS																	DE	SIGNED	BY:	JZ	D	ATE: 9	/29/2016
INCIDENC	E PROBABII	JITY [10-Year	·]																СН	ECKED	BY:	NVD	U	NITS: E	NGLISH
INLET		INVERT	DEPTH	OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION				JUN	ICTION L	OSS			-		Ad j. Ht	Inlet			Inlet	Top of MH	
OR	STA.	EL.	of flow	WATER	PIPE	DISCH.	PIPE	SLOPE,Sfo	LOSS		Contr.			Hî (Expn)	SKEW		Bend	Sum	SURF ACE	1.3	Shaping?	0.5	FINAL	Water	Top of Inlet	Ad justment?
JUNCTION		OUTFLOW	OUTFLOW	SURF ACE	Do	Qo	Lo	(FT/FT)	Hf	Vo	Но	VI	Vi*2/2g	0.35*MAX.	Angle	к	н	HL	FLOW	Ht		Ht	н	Surface	Elev.	
JUNCTION		PIPE	PIPE	ELEV.	(In/mm)	(CFS/CMS)	(Ft/M)	(M/M)	(Ft/M)		(Ft/M)			(Vi2/2g)			(Ft/M)	(Ft/M)		(Ft/M)	Y/N	(Ft/M)	(Ft/M)	Elevation	APPROX.	
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)		(12)	(13)		(14)	(15)		(16)		(16)	(17)	(18)	(19)	
6-3																								486.20		
5-3	31+12	485.20	1.25	486.20	15	1.62	4	0.0007	0.009	3.88	0.06	2.95	0J4	0.05	90.0	0.70	0.09	0.20	0.71	0.26	YES	0.13	0.14	486.34	488.30	0.K.
6-1	31•54,67	485.40	1.25	486.40	15	0.90	38	0.0002	0.008	2.95	0.03	3.41	0,18	0.06	0.0	0.00	0.00	OLO	0.26	013	YES	0.06	0.07	486.47	487.69	0 . K.
6-2	31•81,68	485.70	1.25	486.70	/5	0.64	24	0.0001	0.002	3.41	0.05	0.00	0.00	0.00	0.0	0.00	0.00	0.05	0.64	0.06	YES	0.03	0.03	486.73	487 . 85	О.К.
Exl8																								436.80		
Ex19	99•90.18	435.20	2.00	436.80	24	18.75	8	0.0072	0.057	8. 45	0.28	9.9 5	1.54	0.54	3.0	0.05	0.07	0.89	0.39	0.89	YES	0.44	0.50	437.30	439.41	О.К.
Ex20	99•89.5 7	435.31	2.00	437.30	24	18.37	40	0.0069	0.276	9 . 95	0.38	13 . 81	2.96	1.04	69.0	0.60	1,77	3.19	0.86	3,19	YES	1.60	1.87	439, 17	439.72	О.К.
Ex21	22+10.51	436.14	1.75	439 . 17	21	17,51	/58	0.0128	2.016	13,81	0.74	12.84	2.56	0.90	12.0	0,15	0.39	2.02	0.00	2.02	YES	1.01	3.03	443.76	447.42	0.K.
4-7	22•22.97	443.40	1.83	444.87 3	4 X 2	2 16.81	50	0.0028	0.142	12.84	0.64	6.50	0.66	0.23	75.0	0.62	0.68	1.55	,	1.55	YES	0.78	0.92	445.96	448.92	О.К.
4-8	23•04	445.44	1.25	446.44	15	4.73	79	0.0056	0.442	8 . 41	0.27	9 . 48	1.40	0.49	0.0	0.00	0.00	0.76	1.66	0.99	YES	0.50	0.94	448 . /6	451.58	О.К.
5-1	24•30.32	447,68	1.25	448.68	15	3.07	119	0.0024	0.281	9 . 48	0.35	9.42	1.38	0.48	0.0	0.00	0.00	0.83	1.32	1.08	YES	0.54	0.82	454.26	457 . 87	О.К.
5-2	25•99.96	453.97	1.25	454.97	15	1.75	/58	0.0008	0,121	9.42	0.34	0.00	0.00	0.00	0.0	0.00	0.00	0.34	1.75	0.45	NO	0.45	0.57	466.95	470 . 69	О.К.
Ex23	22•40.73	442.99	1.50	444,19	18	1.08	31	0.000/	0.003	6.33	0.16	0.00	0.00	0.00	0.0	0.00	0.00	0.16	1.08	0.20	NO	0.20	0.21	444.5/	447.72	О.К.
4-6	22.00	445.40	2.00	447.00	24	12.62	28	0.0033	0.091	6.50	0.16	0.00	0.00	0.00	0.0	0.00	0.00	0,16	0.00	0,16	NO	0,16	0.26	447.26	447,60	О.К.

	OSED	DIT	CH (COMF	PUTA	TION	IS										DESIGNI	ED BY:	JZ	DA	TE: 5/	/06/2016
FORM LD	-268																CHECKE	D BY:	SCT	UN	ITS: EA	IGLISH
													Earth		Pr	otective Lin	ing					
					, c	A							n=.025 (USGS-CL)		n=.05		n=.013 n=.035	• •				Depth
STA.	TO STA.	FLOW	Area (Acres)	C-value	INCR.	ACC.	T _c	l ₂	Q ₂	C or F	Slope Ft/Ft	ALLOW. VEL.	VEL.	Q _n	VEL	Depth	Q _n	Depth	I ₁₀	Q ₁₀	Depth	Available I
Westb	oriar Court																					
Rig	ht Side																					
102+25	103+12	▼	8.81	0.33	2.91	2.91	17	3.06	8.90	С	0.0640	2.3	7.0				8.90	0.3	4.16	12.09	0.4	1.0

DITCH TYPICAL







Introduction

This project is a pedestrian access improvement project along Old Courthouse Road NE between Pine Valley Drive and Gosnell Road. The project is mostly in the Town of Vienna, Virginia, but ties into existing sidewalks in Fairfax County at each end. The project proposes to add curb and gutter, sidewalk, and curb ramps with pedestrian crossings at Westbriar Court. Additionally, all drainage will be collected and conveyed via new proposed closed storm sewer systems which will tie into existing storm sewer systems. The project is located in the Wolftrap Creek watershed management area which is within the greater Difficult Run watershed (PL22).

The topography for this project is relatively flat with open ground cover and existing roadway. Areas adjacent to the project limits are residential. The construction area is adjacent to the existing roadway.

The project is within the limits of a watershed identified by Fairfax County as: Difficult Run (HUC Code PL22). The outfall analysis is provided to demonstrate adequacy on this sheet.

The stormwater management (SWM) requirements are assessed in accordance with Virginia Department of Transportation (VDOT) and Department of Environment and Quality (DEQ) criteria for existing versus post-project conditions at outfalls within the receiving drainage basin. This project is not grandfathered, and technical criteria IIB will apply in accordance with Fairfax County Stormwater Management Ordinance. There are no stormwater management facilities proposed as part of this project. A waiver of detention requirements has been applied for this project to satisfy the water quantity requirements. The Virginia Runoff Reduction Method was used to determine the phosphorus removal requirement. Water quality requirements within the Town of Vienna project limits will be met through the use of Filterra tree box filters; water quality requirements within Fairfax County project limits will be met through the purchase of nutrient credits.

Storm sewer pipe computations, including LD-229, LD-204, and LD-347, are presented on sheets 2K(3), 2K(4), and 2K(5) for structures outfalling in Fairfax County and on sheets 2K(6), 2K(7), and 2K(8) for structures outfalling in the Town of Vienna. The storm sewer and inlet layouts are intended to drain the roadway in conformance with the VDOT Drainage Manual (VDM) Chapter 9 and convey the project runoff to an adequate outfall. A soils map and tabulation is provided on sheet 2E.

Compliance with the Virginia Erosion and Sediment Control Regulations Minimum Standard 19 (VESCR MS-19) is verified by the outfall analysis through the design of receiving channels and the analysis of existing downstream systems as required.

Drainage Design Criteria and Methodology

This narrative summarizes our understanding of the design criteria and methods of analysis employed in the design of Old Courthouse Road drainage systems. The criteria as defined in the latest edition of the VDOT Drainage Manual (VDM), including all of its Technical Supplements, and I&IM are generally applied. A list of computer software utilized for this project is also provided.

Hydrology

The Rational method was utilized to calculate flow rates to all structures, inlets, and culverts in cubic feet per second (cfs) for drainage areas less than 200 acres. Runoff coefficients were taken from Appendix 6E-1 of the VDM.

Rainfall Intensity

Rainfall intensities used for rational method design of facilities are based upon the NOAA "Atlas 14" Rainfall Precipitation Frequency Data and assigned B, D, & E factors. The following rainfall intensities are developed from chart #76 B, D, & E factors for Fairfax County, Virginia.

	RAIN	IFALL INTENSITY	•	OUR)
Recurrence		Duration (To	c – Minutes)	
Interval (yr)	5	10	15	30
2	5.23	4.19	3.51	2.41
10	6.77	5.45	4.62	3.26
25	7.69	6.15	5.22	3.73
100	9.10	7.28	6.22	4.57

The correction factors of 1.1 and 1.25 shall be applied to 25-yr and 100-yr storm intensities respectively.

Storm Sewer Design

All storm sewer pipes along Old Courthouse Road are designed to convey the 10-year design storm event based upon Tables 9-1 & 9-2 of VDM Chapter 9.3.1. A minimum of 0.1-foot drop between the lowest incoming storm sewer pipe through a manhole or inlet and the outgoing storm sewer pipe invert is provided where possible. The Hydraulic Grade Line is analyzed for the 10-year storm event for all storm sewer systems with more than two links utilizing the PipeSoftVA 2.0 computer program. Specified storm sewer pipe materials shall comply with VDOT Drainage Manual and Road and Bridge Standards for "Allowable Pipe Material for Storm Sewer Systems."

Inlet Design

Detailed inlet reports have been provided as documentation for inlet design computations. They were generated using the InletSoftVA 2.0 modeling computer program, which utilizes the HEC-22 methodology to calculate the spread and depth for roadway inlets on grade and in sump.

<u>Roadway Inlets on Grade</u>: Drop inlets on grade are designed for intensities of four (4) inches per hour. The maximum allowable spread from the face of curb for drop inlets on grade is half the width of the travel lane + the width of the gutter pan. The maximum allowable spread is 9.5 feet $(1/2 \times 16 \text{ feet} + 1.5)$ feet) for Old Courthouse Road, 8.1 feet (1/2 x 13.5 feet + 1.5 feet) for Westbriar Court (West), and 6 feet (1/2 x 12 feet + 0 feet) for Westbriar Court (East).

A minimum of ninety percent capture efficiency has been attempted to maximize inlet efficiency. At super-elevation reversals, curb returns and intersections, we have made every attempt to provide 100% interception.

<u>Roadway Inlets at Sumps</u>: In order to correctly evaluate the performance of sump inlets, the overflow from upstream inlets has been accounted for. The maximum allowable spreads for sump inlets are the same as for inlets on grade. To compensate for partial clogging, the computed slot length value will be adjusted by multiplying by a factor of 2. Locations of 0.10% longitudinal slope approaching sumps will be checked to assure that the allowable maximum spread is not exceeded. Flanking inlets shall be located where the edge of pavement elevation is no higher than 0.3 feet above the edge of pavement elevation at the sag point.

Hydrology/Hydraulics Software Utilized In Drainage Computations

- InletSoftVA, Version 2.00.11 Virginia Edition
- PipeSoftVA, Version 2.00.11 Virginia Edition 2.

Stormwater Management Narrative

The project is within the limits of a watershed identified by Fairfax County as: Difficult Run (HUC Code PL22). Stormwater Management requirements are assessed for individual watersheds in accordance with VDOT and Fairfax County criteria for existing versus post-project conditions at outfalls within the receiving watersheds. "Site Area" as defined in SWPA 12-01 was calculated to develop the overall SWM approach (to meet the requirements and determine the treatment required). The overall site area is 1.30 acres, of which 1.08 acres is within the limits of the Town of Vienna and 0.22 acres is within Fairfax County. The total existing impervious area is 0.51 acres and the total proposed impervious area is 0.75 acres. This project results in 0.24 acres of new impervious area. An overview of the approach to the SWM requirements in the watershed is as follows:

As part of this project, there are no structural methods proposed. A waiver of the detention requirements has been requested for this project. A waiver of WQN requirements is justified as an adequate receiving channel is available. This project proposes 0.24 acre of new impervious area. At Outfall #1A in Fairfax County, there is a minor increase in peak flows due to an increase in impervious area. The outfall at this location is an existing channel. At Outfall #3 in Fairfax County, there is a negligible increase in peak flows due to an increase in impervious area. The outfall at this location is an existing storm sewer system. At Outfall #2 in the Town of Vienna, there is a minor increase in peak flows due to an increase in impervious area. The outfall at this location is existing curb and gutter. At Outfall #1 in the Town of Vienna, there is a minor increase in peak flows due to an increase in impervious area. The outfall at this location is an existing storm sewer system. The adequacy of project outfalls to convey storm water is discussed on this sheet. WQL within this sub-watershed is addressed by the proposed purchase of nutrient credits and Filterra tree boxes.

BMP Narrative

This project is not grandfathered and, therefore, technical criteria IIB applies to this project in accordance with the Fairfax County Stormwater Management Ordinance. This project is considered as a re-development project. The WQL requirements for this project were assessed in accordance with SWPA 12-01, and the Virginia Runoff Reduction Method spreadsheet was used to determine the phosphorus removal for the entire project.

In the Difficult Run watershed, the total site area in accordance with SWPA12-01 is 1.30 acres., of which 1.08 acres is within the limits of the Town of Vienna and 0.22 acres is within Fairfax County. The total existing impervious area is 0.51 acres and the total proposed impervious area is 0.75 acres. This project results in 0.24 acres of new impervious area. Per the calculations on sheet 2K(11) - 2K(11d), this project requires a total of 0.74 lb/year of total phosphorus load removal.

Within the Town of Vienna, 0.60 lb/yr of total phosphorus load removal is required. The phosphorus removal requirement will be met through four Filterra manufactured BMPs. Please refer to the drainage descriptions on sheet 2K and BMP notes and details on sheets 2L-2L(1) for more information.

For the Fairfax County portion of this project, 0.14 lb/yr of total phosphorus load removal is required. There are no structural facilities proposed in this phase of the project. The phosphorus removal requirement for Fairfax County will be met through the purchase of nutrient credits.

Outfall Analysis Narrative

Project runoff and outfalls are located within the Difficult Run watershed (PL22). Analysis is provided for each outfall associated with the project. With this project, three outfalls have been identified as key locations of study for adequate outfall analysis. The pre-versus post-development drainage conditions of the outfalls are tabulated in the Outfall Analysis Summary Table on sheet 2K(10). The Outfall Maps on sheet 2K(10) identify the location and limits of analysis which are based on peak flow rate. The site's peak flow rate from the 10-year 24 hour storm event is less than or equal to 1.0% of the existing peak flow rate from the 10-year 24 hour storm event prior to the implementation of any stormwater quantity control measures. Offsite drainage areas flowing to or through the project are tabulated assuming actual or current proposed land use.

Soils data provided on sheet 2E is used to determine maximum allowable velocity for the 2-year storm. Soils data is taken from the mapping and data provided by Fairfax County. Individual outfalls are described in detail as follows:

Drainage, SWM, and Outfall Narratives

CulvertSoftVA, Version 2.00.03 – Virginia Edition

Outfall #1 (In the Town of Vienna)

Description: Outfall #1 is an outfall to an existing storm sewer system running from Old Courthouse Road to Westbriar Court. With the development on this project, there is an increase to the impervious area in the post-developed scenario. Therefore, there is a slight increase in the peak flow rate at this outfall. The proposed system collects runoff from Old Courthouse Road and Westbriar Court and surrounding off-site area.

Drainage Area: The proposed drainage area is 12.16 acres which is a decrease of 0.01 acre over the existing condition. There is an addition of 0.13 acre of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 19.77 cfs, which is an increase of 0.07 cfs over the predevelopment condition.

<u>Receiving System Cross Section and Capacity</u>: Outfall #1 is an existing storm sewer system downstream of an existing inlet on Old Courthouse Road. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.07 cfs to this system for the 10-year storm condition. The existing system is adequate to handle this negligible increase in peak flow. Please refer to the LD-229 Storm Computations on sheet 2K(7).

Limits of Study: The location of the outfall is shown on the Outfall Map on sheet 2K(10). The limit of analysis is 250 L.F. downstream and contained within an existing storm sewer system which extends from proposed structure 4-7 (Old Courthouse Road Station 22+25 LT) to existing structure Ex. 18.

Permissible Velocity: Permissible velocity is not applicable as the runoff is confined within a closed sewer system.

Easement Requirements: The existing system is located within the existing right-of-way or existing storm drain easement as necessary; therefore, no additional easement is required.

Final Opinion: The drainage to the existing system has an insignificant increase in peak flows and flows are contained within the existing closed storm sewer system. The peak flow rate for the project site increases by less than or equal to 1.0% of the existing peak flow rate prior to the implementation of any stormwater quantity control measures. Therefore, it is our professional opinion that Outfall #1 is an adequate outfall and the requirements of MS-19 are satisfied.

Outfall #2 (In the Town of Vienna)

Description: Outfall #2 is an outfall with sheet flow to existing curb and gutter running along Old Courthouse Road. With the development on this project, there is an increase in impervious area in the post-developed scenario. Therefore, there is a minor increase in the peak flow rate at this outfall.

Drainage Area: The proposed drainage area is 3.03 acres which is an increase of 0.09 acre over the existing condition. There is an addition of 0.08 acre of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 13.22 cfs, which is an increase of 0.52 cfs over the predevelopment condition. The existing curb and gutter is adequate to handle this minor increase in peak flow. Please refer to the Curb & Gutter Outfall Spread Computations on sheet 2K(7).

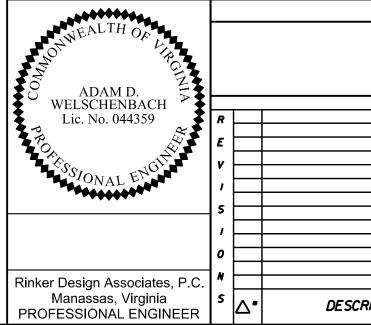
<u>Receiving System Cross Section and Capacity</u>: Outfall #2 is existing curb and gutter downstream of the project along Old Courthouse Road. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.52 cfs to this system for the 10-year storm condition. The existing curb and gutter is adequate to handle this increase in peak flow. Please refer to the Curb & Gutter Outfall Spread Computations on sheet 2K(7).

Limits of Study: The location of the outfall is shown on the Outfall Map on sheet 2K(10). The limit of analysis is 450 L.F. downstream of the intersection of Old Courthouse Road and Pine Valley Drive and contained within existing curb and gutter along Old Courthouse Road.

Permissible Velocity: Permissible velocity is not applicable as the runoff is confined within existing curb and gutter.

Easement Requirements: The existing system is located within the existing right-of-way or existing storm drain easement as necessary; therefore, no additional easement is required.

Final Opinion: The drainage to the existing curb and gutter has a minor increase in peak flows and flows are contained within the existing curb and gutter. Therefore, it is our professional opinion that Outfall #2 is an adequate outfall and the requirements of MS-19 are satisfied.



Outfall #1A (In Town of Vienna)

Description: Outfall #1A is an outfall to an existing channel running along Westbriar Court. With the development on this project, there is an increase to the impervious area in the post-developed scenario. Therefore, there is a slight increase in the peak flow rate at this outfall. The channel collects runoff from Old Courthouse Road within Fairfax County limits and surrounding off-site area and re-enters a proposed storm sewer system downstream in the Town of Vienna.

Drainage Area: The proposed drainage area is 5.98 acres which is the same as in existing conditions. There is an addition of 0.04 acres of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 11.94 cfs, which is an increase of 0.18 cfs over the predevelopment condition.

<u>Receiving System Cross Section and Capacity</u> Outfall #1A an existing channel downstream of Old Courthouse Road in Fairfax County and running along Westbriar Court. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.18 cfs to this system for the 10-year storm condition. Cross section data and computations are shown on sheet 2K(10a). The capacity of the channel is shown to be adequate for conveyance of the 10 year storm event as required, with a water surface depth of 0.5 ft at cross section A-A, 0.4 ft at cross section B-B, and 0.5 ft at cross section C-C. The 10 year storm is contained within the channel.

Limits of Study: The points of study for the outfall are indicated by the cross sections shown on the Outfall Map on sheet 2K(10). The limit of analysis is 350 L.F. downstream of the proposed structure 6-3 (Old Courthouse Road Station 31+00 LT) in Fairfax County.

<u>Permissible Velocity</u>: Existing channel permissible velocities are based on soil classification and comply with VDM Appendix 7D-6 for existing and proposed vegetated channels and appendix 7D-2 for the existing and proposed channels without established linings or proposed channel protection. Soils data is taken from the tabulation on sheet 2E. Soil along the existing channel alignment is type 31C (Danripple Gravelly Loam), and has a maximum permissible velocity of 2.3 fps.

Channel Velocity: The 2 year velocity in the channel is 2.2 fps at cross section A-A, 1.9 fps at cross section B-B, and 2.3 fps and cross section C-C. These velocities are less than or equal to the permissible velocity of 2.3 fps.

Easement Requirements: Outfall #1A is an existing channel. The flows are contained within the channel, therefore no easement for the existing channel is required. Necessary easement will be procured for the proposed ditch to the existing channel.

<u>Final Opinion</u>: The drainage to the existing system has slight increase in peak flows and flows are contained within the existing channel. Therefore, it is our professional opinion that Outfall #1A is an adequate outfall and the requirements of MS-19 are satisfied.

Outfall #3 (In Fairfax County)

Description: Outfall #3 is an outfall to an existing storm sewer system running from east to west along Old Courthouse Road. With the development on this project, there is an increase to the impervious area in the post-developed scenario. Therefore, there is a slight increase in the peak flow rate at this outfall. The proposed system collects runoff from Old Courthouse Road and Pine Valley Drive and surrounding off-site area.

Drainage Area: The proposed drainage area is 17.30 acres which is a decrease of 0.07 acre over the existing condition. There is an addition of 0.04 acre of new impervious area within this outfall.

Outfall Discharge: Pre- and post-development discharges for this outfall are shown in the Outfall Analysis Summary Table and contributing areas are shown in the Outfall Map on sheet 2K(10). The proposed 10-year flow to the receiving system is 28.75 cfs, which is an increase of 0.01 cfs over the predevelopment condition.

<u>Receiving System Cross Section and Capacity</u>: Outfall #3 is an existing storm sewer system downstream of an existing inlet on Old Courthouse Road. The designated location of the outfall section is shown on the Outfall Map on sheet 2K(10). There is an increase of 0.01 cfs to this system for the 10-year storm condition. The existing system is adequate to handle this negligible increase in peak flow. Please refer to the LD-229 Storm Computations on sheet 2K(4).

Limits of Study: The location of the outfall is shown on the Outfall Map on sheet 2K(10). The limit of analysis is 400 L.F. downstream and contained within an existing storm sewer system which extends from existing structure Ex. 2 (Old Courthouse Road Station 12+20 LT) to existing structure Ex. 7.

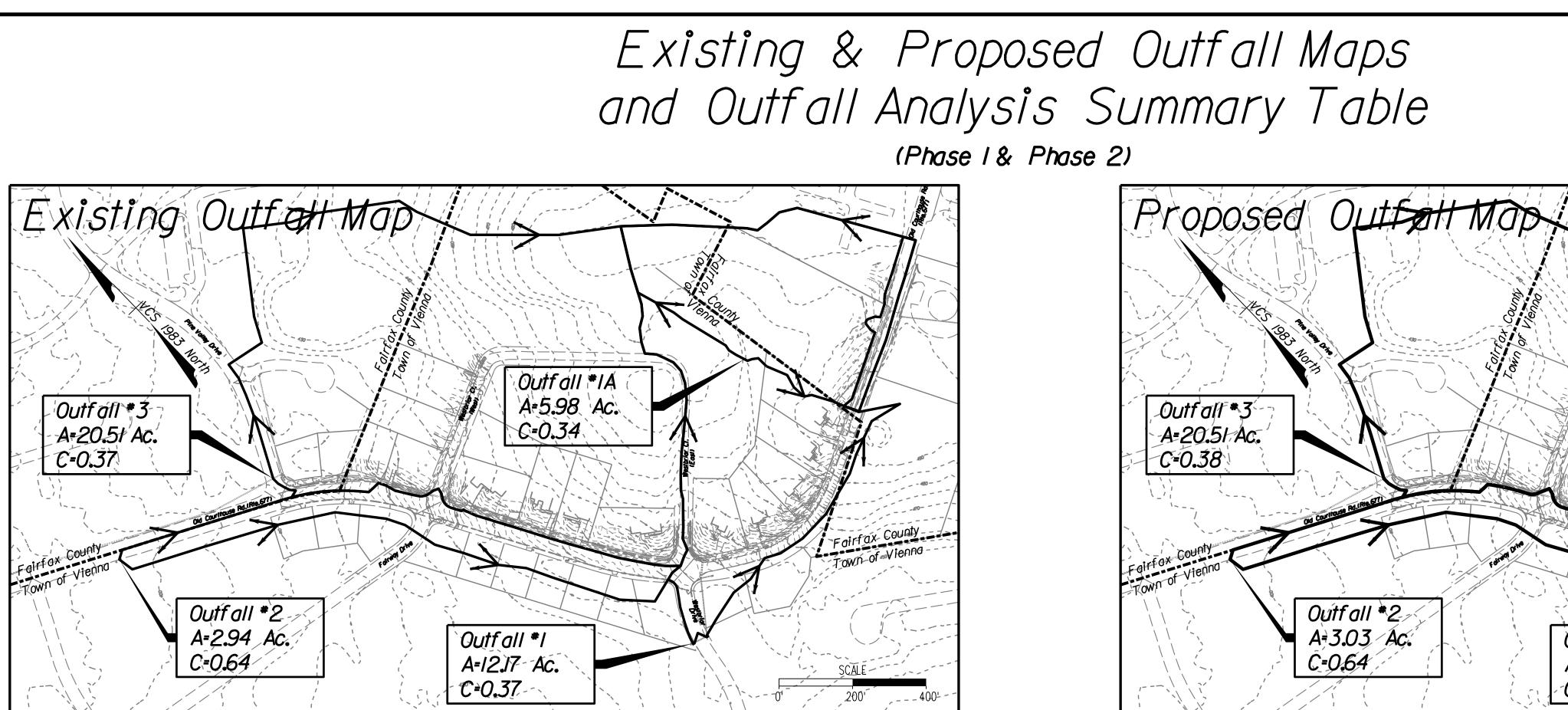
<u>Permissible Velocity</u>: Permissible velocity is not applicable as the runoff is confined within a closed sewer system.

Easement Requirements: The existing system is located within the existing right-of-way or existing storm drain easement as necessary; therefore, no additional easement is required.

Final Opinion: The drainage to the existing system has an insignificant increase in peak flows and flows are contained within the existing closed storm sewer system. The peak flow rate for the project site increases by less than or equal to 1.0% of the existing peak flow rate prior to the implementation of any stormwater quantity control measures. Therefore, it is our professional opinion that Outfall #3 is an adequate outfall and the requirements of MS-19 are satisfied.

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				PEDESTRI Drainage	AN ACCE e, SWM, and (Phase 1)	HOUSE ROA ESS IMPROV Outfall Narr Phase 2) FAIRFAX COUNTY.	EMENTS atives
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RIPTION	BY	APPROVED	DATE	HORIZ= N/A VERT= N/A	DRAFTED B CHECKED B		2K(9)



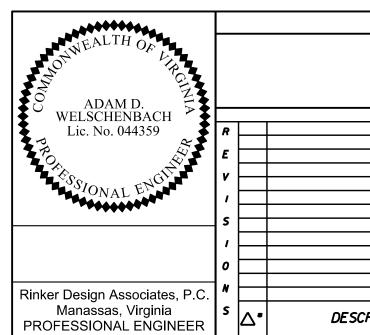


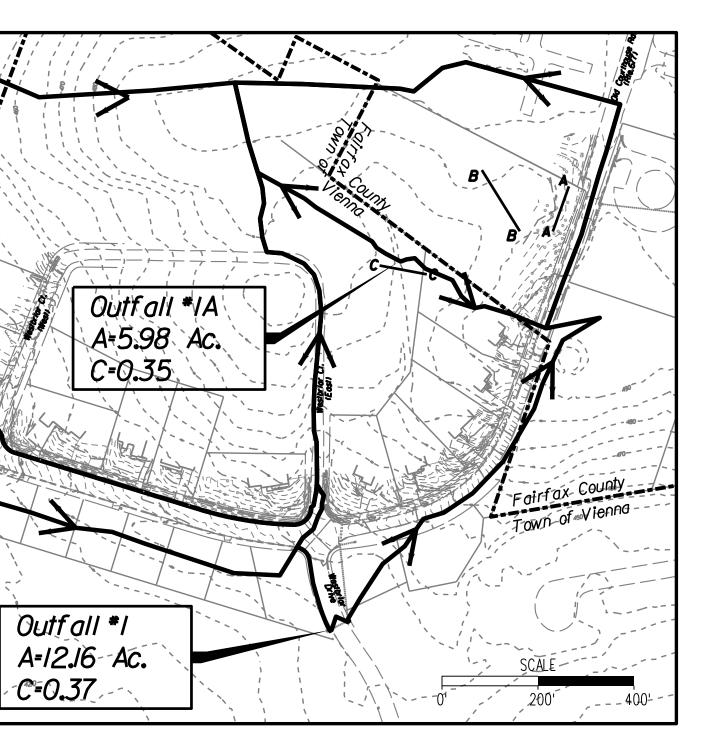
Outfall #I:Within the Town of Vienna Outfall #2:Within the Town of Vienna Outfall #3:Within Fairfax County

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	Outfall	Outfall Location (Station)	Outfall Structure (if any)	Total Area	Imperv. (C=0.90)	Grass (C=0.30)	Forest (C=0,25)	Resident. < 2k sqft (C=0.45)	Resident. I7k sqft (C=0.40)	Resident. >I/2 ac (C=0.35)	Cw	Tc (min)	(in/ 2-yr	/hr) IO-yr	(c 2-yr	lO-yr	in Peak F 2-yr	Flow (cfs) IO-yr	Peak 2-yr		Channel Velocity 2-yr Check (ft/s)	Natural Channel 2-yr Avail. Depth (in)	Manmade Channel IO-yr Avail. Depth (in)	Size	ormdrain <u>System</u> IO-yr Pipe Capacity (cfs)	Outfall Adequacy Yes/No	Rem	orks			
	Outfall *I	Westbriar (Court																				· · · · · · · · · · · · · · · · · · ·						=		
	Pre-Developed	99•90 LT	Ex.18	12,17	0.77	0.78	3.31	1,19	0.72	5.40	0.37	16.6	3.34	4.41	14.93	19.69	-				N/A	N/A	N/A	24	24.00	Yes	Existing Storm	-			
TOWN	Post-Developed	99•90 LT	Ex.18	12,16	0.90	0.72	3.23	I , 19	0.72	5.40	0.37	I7,J	3.29	4.35	14,97	19.77	0.04	0.07	0.3%	0.4%	N/A	N/A	N/A	24	24.00	Yes	Existing Storm	System			
OF	Outfall *IA	Old Courtha	use Road													·'	<u> </u>												_		
VIENNA	Pre-Developed	31.00 LT	N/A	5.98	0.28	0.07	3.22	,	0.00	1.31	0.34	8.5	4.45	5.78	9.05	//.76					2.3	N/A	2.00	-	N/A	Yes	Sheet Flow to E	xisting Chann	nel		
(Phase 1)	Post-Developed	31•00 LT	6-3	5 . 98	0.32	0.20	3.04	,	0.00	1.31	0.35	8.5	4.45	5.78	9,18	11.94	0,14	0.18	1.5%	1.5%	2.3	N/A	2.00	-	N/A	Yes	Ditch to Existing				
	0.46-14-80	Old Counth														'	 												_		
	Outfall *2 Pre-Developed	Old Courtha	Ex.40	2.94	1.24	0.00	0.00	1.61	OLO	0.00	0.64	5.0	5.23	6.77	9.80	12.70	<u> </u>				N/A	N/A	N/A	+ - +	N/A	Yes	Existing Gutter		_		
	Post-Developed	5•75 RT	Ex. 40	3.03	1.32	0.00	0.00	1 . 61	010	0.00	0.64	5.0	5.23	6.77	10.21	13.22	0.40	0.52	4.1%	4.1%	N/A	N/A	N/A	-	N/A	Yes	Existing Gutter				
FAIRFAX																'															
COUNTY	Outfall *3	Old Courth		00.51	0.46			074			0.77		0.17			<u> </u>	 							07	(0.00			<u> </u>	_		
(Phase 2)	Pre-Developed	8•57 LT 8•57 LT	<i>Ex.7</i> <i>Ex.7</i>	20.51 20.51	0.46 0.99	1.18 1.06	0.60	2.74 2.74	0.22 0.22	15 . 31 14 . 90	0.37 0.38	-	013 013	0.20	<i>1.00</i> <i>1.04</i>	<i>1.54</i> <i>1.60</i>	0.04	0.06	3.83%	3.83%	N/A N/A	N/A N/A	N/A N/A	27 27	48.00 48.00	Yes Yes	Existing Storm Existing Storm	-			
(Thuse S/				20.31	0.55	1.00	0.00	<i>۲</i> ۰/ ۲	0.22	14.30	0.50		015	0.20	1.04	1.00	0.04	0.00	5.05%	5.05%					-0.00	1 65		System]		
Note: 2yr 24hr an	d 10 yr 24hr rainfa	Il intensities	using for co	mputations	s in Fairif	ax County	/.					PROJE	CT SITE	TOTALS																	TAX MAP 29-3
													re-Develope		25.74											EMERG	ENCY PO	DLICE -	FIRE - RES	CUE 911	
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Outfall #1:Within the Town of Vienna Outfall #2:Within the Town of Vienna Outfall #3:Within Fairfax County

PROJECT SITE TOTALS	5					
Pre-Developed	25.74	33.93				
Post-Developed	26.22	<i>34.</i> 59	0.48	0.66	1.88%	1.94%
·	•					





FUND"

Worksheet for Outfall IA AA IOYr

Roughness Coefficient,0.025

Left Side Slope, 3.00, ft/ft (H:V)

Right Side Slope, 3.00, ft/ft (H:V)

Channel Slope,0.01000,ft/ft

Discharge, I.90 fps

Normal Depth,0.52,ft

Wetted Perimeter, 3.29, ft

Hydraulic Radius,0.25,ft

Critical Slope,0.01574,ft/ft

Flow Area,0.81 sf

Top Width.3.12.ft Critical Depth,0.48,ft

Velocity,2.34,ft/s

Velocity Head,0.09,ft

Froude Number,0.81

Specific Energy, 0.61, ft

Flow Type,Subcritical

Input Data

Results

Worksheet for Outfall IA AA 2Yr

Input Data

Roughness Coefficient,0.025 Channel Slope,0.01000,ft/ft Left Side Slope,3.00,ft/ft (H:V) Right Side Slope, 3.00, ft/ft (H:V) Discharge, I.46, fps

Results

Normal Depth.0.47.ft Flow Area,0.67,sf Wetted Perimeter, 2.98, ft Hydraulic Radius,0.22,ft Top Width 2.83.ft Critical Depth,0.43,ft Critical Slope,0.01631,ft/ft Velocity,2.19.ft/s Velocity Head, 0.07, ft Specific Energy,0.55,ft Froude Number,0.80 Flow Type,Subcritical

Outfall IA A-A (IO Yr) Cross Section

0.52 ft

Worksheet for Outfall IA BB 2Yr

Input Data Channel Slope,0.03200,ft/ft Discharge,9.18 fps

Section Definitions Station (ft),Elevation (ft) 0.00,485.00 0+17,484.00 0•50,483.00 0.80,482.00 0.88,482.00 1.10,483.00 1.29,484.00 1.50,485.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0.00, 485.00), (0.50, 483.00), 0.060 (0.50,483.00),(1.10,483.00),0.050 (1.10,483.00),(1.50,485.00),0.060

Results Normal Depth,0.31,ft Elevation Range,482.00 to 485.00 ft Flow Area,4.95 sf Wetted Perimeter,24.08,ft Hydraulic Radius,0.21,ft Top Width 24.07, ft Normal Depth,0.31,ft Critical Depth,0.26,ft Critical Slope,0.06478,ft/ft Velocity,1.85,ft/s Velocity Head,0.05,ft Specific Energy,0.36,ft Froude Number,0.72 Flow Type,Subcritical

Worksheet for Outfall IA BB IOYr

Input Data Channel Slope,0.03200,ft/ft Discharge, II.94 fps

Section Definitions Station (ft), Elevation (ft) 0.00,485.00 0•17,484.00 0.50,483.00 0.80,482.00 0.88,482.00 1•10,483.00 1.29,484.00 1.50,485.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0.00, 485.00), (0.50, 483.00), 0.060 (0.50,483.00),(1.10,483.00),0.050 (1.10,483.00),(1.50,485.00),0.060

Results Normal Depth,0.35,ft Elevation Range,482.00 to 485.00 ft Flow Area,6.00 sf Wetted Perimeter, 26.25, ft Hydraulic Radius,0.23,ft Top Width,26,23,ft Normal Depth,0.35,ft Critical Depth,0.30,ft Critical Slope,0.06232,ft/ft VelocityJ.99.ft/s Velocity Head,0.06,ft Specific Energy,0.41,ft Froude Number,0.73 Flow Type,Subcritical

Outfall Computations for Outfalls in the Town of Vienna

Worksheet for Outfall IA CC 2Yr

Input Data Channel Slope,0.02000,ft/ft Discharge,9.18 fps

Section Definitions Station (ft), Elevation (ft) 0.00,478.00 0•11,478.00 0.35,476.00 0.50,475.00 0.72,476.00 0•90,478.00 1.00,479.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0.00, 478,00),(0.35, 476,00),0.060 (0.35,476.00),(0.72,476.00),0.035 (0+72,476.00),(1+00,479.00),0.035

Results Normal Depth,0.47,ft Elevation Range,475.00 to 479.00 ft Flow Area,4.04,sf Wetted Perimeter.J7.32.ft Hydraulic Radius,0.23,ft Top Width, 17, 29, ft Normal Depth.0.47.ft Critical Depth,0.43,ft Critical Slope,0.02978,ft/ft Velocity,2.27.ft/s Velocity Head, 0.08, ft Specific Energy,0.55,ft Froude Number,0.83 Flow Type,Subcritical

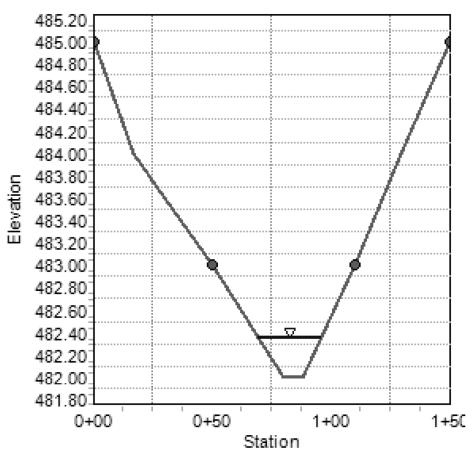
Worksheet for Outfall IA CC IOYr

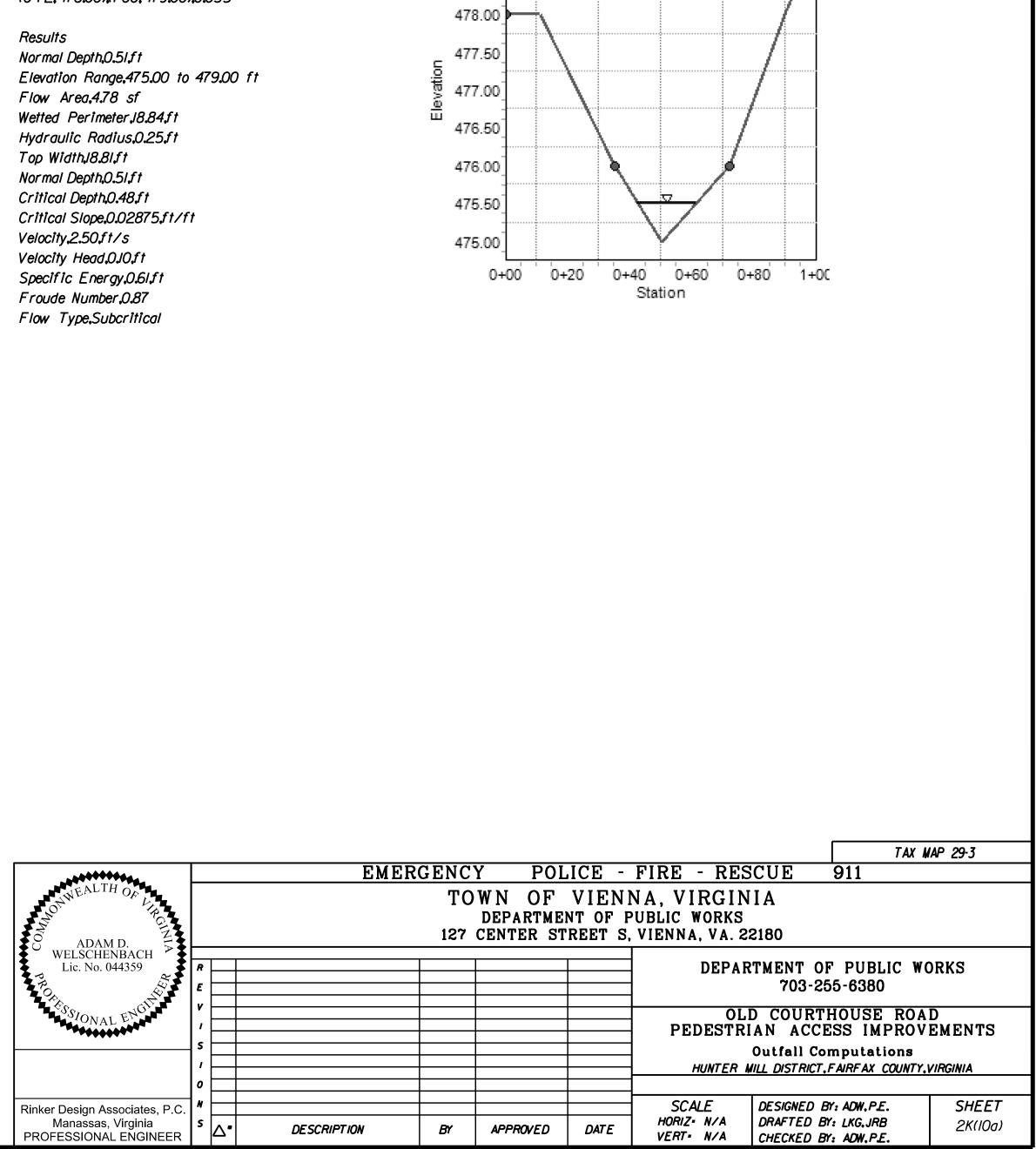
Input Data Channel Slope.0.02150.ft/ft Discharge, II.94 fps

Section Definitions Station (ft), Elevation (ft) 0.00,478,00 0•11,478.00 0.35,476.00 0.50.475.00 0.72,476.00 0•90,478.00 1.00,479.00

Roughness Segment Definitions Start Station, Ending Station, Roughness Coefficient (0.00, 478.00), (0.35, 476.00), 0.060 (0.35,476.00),(0.72,476.00),0.035 (0.72, 476.00),(1.00, 479.00),0.035

Outfall IA B-B (IO Yr) Cross Section





Outfall IA C-C (IO Yr) Cross Section

479.00

478.50

35x23 (în.) V:\DesignAid\Plot-Drivers\default_rda_25.tbl default_rda_25**.**tbl

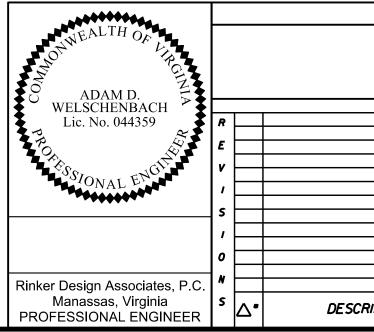
	Old Courtho			ements - Phase 1		CLEAR	ALL	•			
				Yes				calculation cells			
ite Information								final results			
ost-Development Project	(Treatme	nt Volume	and Loads)								
		Ente	r Total Disturbed	d Area <i>(acres)</i> →	1.08]	BMP Design Sne		2011 St	ds & Sners	
				=				Linear project?	No		
						La La		-			
	c)										
	-	B Soils	C Soils	D Soils	Totals	1					
otected forest/open space or reforested land anaged Turf (acres) disturbed, graded for				0.00							
rds or other turf to be mowed/managed											
				0.43							
ost-Development Land Cover (acres)											
rest/Open Space (acres) undisturbed,	A Soils	B Soils	C Soils								
otected forest/open space or reforested land anaged Turf (acres) disturbed, graded for											
rds or other turf to be mowed/managed											
	OK.	ОК.	ОК.			J					
onstants nual Rainfall (inches)	<u>/</u> 2		Runoff Coefficien		R Soils	Csoile	D Soile				
arget Rainfall Event (inches)	1.00			0.02	0.03	0.04	0.05				
otal Nitrogen (TN) EMC (mg/L)	1.86										
rget TP Load (Ib/acre/yr) (unitless correction factor)											
LAND COVER SUMMARY PI	RE-REDEVE	LOPMENT			L	AND COVER	SUMMARY P	OST DEVEL	OPME	NT	
Land Cover Summ	ary-Pre			Land Cover Summo	ıry-Post (Final)		Land Cover Sun	nmary-Post		Land Cover Sumn	nary-Post
Pre-ReDevelopment	Listed	Adjusted ¹			w Impervious			lopment		Post-Development Ne	w Impervious
Forest/Open Space Cover (acres)				Cover (acres)			Cover (acres)				
Managed Turf Cover (acres)	0.65	0.44			0.44		-	0.44			
Weighted Rv(turf)	0.25	0.25		Weighted Rv (turf)	0.25		Weighted Rv (turf)	0.25			
% Managed Turf	60%	51%		% Managed Turf	41%		% Managed Turf	51%			
Impervious Cover (acres)	0.43	0.43			0.63		· ·	0.43		· · ·	0.21
Rv(impervious)	0.95	0.95		Rv(impervious)			Rv(impervious)	0.95		Rv(impervious)	0.95
					0.00	Troot					
Pre-ReDevelopment Treatment Volume				Development Treatment Volume	0.0593		Post-ReDevelopment Treatment Volume		au	Treatment Volume	0.0163
Pre-ReDevelopment Treatment Volume (cubic feet)	2,061	1,874		Development Treatment Volume	2,584		Treatment Volume	1,874		Treatment Volume (cubic	710
Pre-ReDevelopment TP Load (lb/yr)	1.29	1.18		Development TP Load	1.62		Load (TP)	1.18			0.45
Pre-ReDevelopment TP Load per acre (lb/acre/yr)	1.20	1.35		Load per acre	1.51		Load per acre	1.35			
.41 lbs/acre/yr applied to pre-redevelopment area		0.36					(Below Pre-				
inaged turf) acreage proposed for new impervio justed total acreage is consistent with Post-ReD	ous cover.						Required for Redeveloped Area	0.24		Required for New	0.36
	w impervious cov	er (based on new									
			Post-Dev	elopment Requ	irement for	Site Area					
			TP Load	Reduction Required	(lb/yr)	0.60					
			Linear Pr	oject TP Load Reductio	n Required (lb/yr)	0.59					
			Nit	rogen Loads (Info	rmational Pur	poses Only)					
in the second seco						Final Post-De	velopment TN Load				

stions - Town of Vienna

Area Checks	6 D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	AREA CHECK
FOREST/OPEN SPACE (ac	0.00	0.00	0.00	0.00	0.00	ОК.
IMPERVIOUS COVER (ac		0.15	0.11	0.04	0.00	ОК.
IMPERVIOUS COVER TREATED (ac		0.15	0.11	0.04	0.00	ОК.
MANAGED TURF AREA (ac	0.24	0.15	0.36	0.07	0.00	AREA EXCEEDED
MANAGED TURF AREA TREATED (ac	0.24	0.15	0.36	0.07	0.00	ОК.
AREA CHECK	ОК.	ОК.	ОК.	OK.	ОК.	
Site Treatment Volume (ft ³)	2,584]				
unoff Reduction Volume and TP By Drainage Area						
	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	TOTAL
RUNOFF REDUCTION VOLUME ACHIEVED (ft ³) 0	0	0	0	0	0
TP LOAD AVAILABLE FOR REMOVAL (Ib/yr		0.41	0.44	0.13	0.00	1.53
TP LOAD REDUCTION ACHIEVED (lb/yr	0.27	0.21	0.09	0.06	0.00	0.63
TP LOAD REMAINING (lb/yr	0.27	0.21	0.35	0.06	0.00	0.90
NITROGEN LOAD REDUCTION ACHIEVED (lb/yr	0.00	0.00	0.00	0.00	0.00	0.00
Total Phosphorus			LINEAR PROJECT:			
FINAL POST-DEVELOPMENT TP LOAD (lb/yr		1	1.62			
TP LOAD REDUCTION REQUIRED (Ib/yr			x			
TP LOAD REDUCTION ACHIEVED (Ib/yr			X			
TP LOAD REMAINING (lb/yr)		1	x			
REMAINING TP LOAD REDUCTION REQUIRED (lb/yr)	1		CHECK AREAS!		1	1
Total Nitrogen (For Information Purposes)						
POST-DEVELOPMENT LOAD (Ib/yr		1				
NITROGEN LOAD REDUCTION ACHIEVED (Ib/yr						
REMAINING POST-DEVELOPMENT NITROGEN LOAD (lb/yr						

TOTAL PHOSPHORUS SUMMARY TABLE

Site Results (Water Quality Complian	ce)
Total Phosphorus	LINEAR PROJECT
FINAL POST-DEVELOPMENT TP LOAD (lb/yr)	1.62
TP LOAD REDUCTION REQUIRED (lb/yr)	0.60
TP LOAD REDUCTION ACHIEVED (lb/yr)	0.63
TP LOAD REMAINING (lb/yr):	0.99
REMAINING TP LOAD REDUCTION REQUIRED (Ib/yr):	0.00



NOTE: The "CHECK AREAS!" message in the VRRM is generated on account of contributing offsite areas included in the Drainage Areas tabs of the spreadsheet.

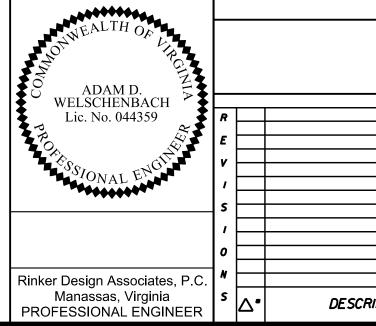
						TAX M	IAP 29-3
EME	RGENC	Y POL	- ICE -	FIRE - RE	SCUE 91	11	
		DEPARTME	NT OF F	NA, VIRGIN Public works Vienna, va. 2			
				DEPA	RTMENT OF I 703-255-		DRKS
					LD COURTHO		
					ater Quality C <i>MILL DISTRICT, FAI</i>		
				SCALE	DESIGNED BY: A	DW,P.E.	SHEET
RIPTION	BY	APPROVED	DATE	HORIZ• N/A VERT• N/A	DRAFTED BY: L		2K(II)
							FUND*

	V:\Design								
			Wa	ter	Quali	°ty C	`alcu	lations	· - 7
DEQ Virginia Runoff Reduction Method Re			readsheet - Versic	on 3.0					
BMP Design Specifications List: 2 Site Summary - Linear Develop	-								
Г	Total R	ainfall (in):	43	1					
1		rbed Acreage:	1.08	1					
Site Land Cover Summary									
Pre-ReDevelopment Land Cover (acres	s)								
	A soils	B Soils	C Soils	D Soils	Totals	% of Total			
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0	-		
Managed Turf (acres) Impervious Cover (acres)	0.00	0.00	0.00	0.65	0.65	60 40	-		
	0.00	0.00	0.00	0110	1.08	100			
Post BoDovolonmont Land Cover (acro									
Post-ReDevelopment Land Cover (acre	A soils	B Soils	C Soils	D Soils	Totals	% of Total	7		
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0			
Managed Turf (acres)	0.00	0.00	0.00	0.44	0.44	41	_		
Impervious Cover (acres)	0.00	0.00	0.00	0.63	0.63	59 100	-		
Site Rv	& New	Development Impervious) D.66	ReDevelopment 0.59	Development (New Impervious) 0.95	ReDevelopment 0.59		TP Load per acre (lb/acre/yr) 1.35	TP Load per acre (lb/acre/yr) 1.51	Load per acre (lb/acre/yr) 1.35
Treatment Volume (ft ³)		,584	1,874	710	1,874	-			
TP Load (lb/yr)		1.62	1.18	0.45	1.18				
Total TP Load Reduction Required (lb/yr)	().59	N/A***	N/A***]				
II	***This is a line	ar development p	roject		1				
			evelopment Load		Pre-]			
TN Load (lb/yr)	(ent & New Impervio	ous)	ReDevelopment 9.26	-			
			11.01		5.20	1			
Site Compliance Summary - *	***Linear D	Developmen	t Project						
Maximum % Reduction	Required Below	20%]						
FIE-REDE			1						
Total Runoff Volume Reduction (ft ³)	0		Error Summary:						
Total Runoff Volume Reduction (ft ³) Total TP Load Reduction Achieved (lb/yr)	0 <i>x</i>		Error Summary:						
			Error Summary:						
Total TP Load Reduction Achieved (lb/yr)	x		Areas on D.A. tab(areas				

wn of Vienna

SITE COMPLIANCE SUMMARY TABLE -LINEAR DEVELOPMENT

Site Compliance Summary						
Maximum % Reduction Required Below	20%					
Pre-ReDevelopment Load	20%					
Total Runoff Volume Reduction (ft ³)	0					
Total TP Load Reduction Achieved (lb/yr)	0.63					
Total TN Load Reduction Achieved (lb/yr)	0.00					
Remaining Post Development TP Load (lb/yr)	0.99					
Remaining TP Load Reduction (lb/yr) Required	0.00					





NOTE: The "CHECK AREAS!" message in the VRRM is generated on account of contributing offsite areas included in the Drainage Areas tabs of the spreadsheet.

						TAX M	IAP 29-3
EMER	GENC	Y POL	ICE -	FIRE - RES	SCUE	911	
		DEPARTME	NT OF P	NA, VIRGIN PUBLIC WORKS VIENNA, VA. 2			
				DEPA		F PUBLIC WC 5-6380	DRKS
				PEDESTR	IAN ACCE	HOUSE ROA SS IMPROV	EMENTS
					•	<pre>v Calculations FAIRFAX COUNTY.</pre>	
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY DRAFTED BY CHECKED BY	: LKG.JRB	SHEET 2K(IIa)
							FUND"

STR. 4-2A (FILTERRA)

Drainage Area A Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% 0
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	
Managed Turf (acres)	0.00	0.00	0.00	0.24	0.24	
Impervious Cover (acres)	0.00	0.00	0.00	0.19	0.19	
					0.43	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.24	0.19	873.02	0.00	0.55	0.27	0.27	

Total Impervious Cover Treated (acres)	0.19
Total Turf Area Treated (acres)	0.24
Total TP Load Reduction Achieved in D.A. (Ib/yr)	0.27
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00

STR. 4-8A (FILTERRA)

Drainage Area B Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0
Managed Turf (acres)	0.00	0.00	0.00	0.15	0.15	50
Impervious Cover (acres)	0.00	0.00	0.00	0.15	0.15	50
					0.30	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)
14.b. Manufactured Treatment Device- Filtering	0.15	0.15	653.40	0.00	0.41	0.21

Total Impervious Cover Treated (acres)	0.15
Total Turf Area Treated (acres)	0.15
Total TP Load Reduction Achieved in D.A. (Ib/yr)	0.21
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00

STR. 5-1A (FILTERRA)

Drainage Area C Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	%
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	
Managed Turf (acres)	0.00	0.00	0.00	0.36	0.36	
Impervious Cover (acres)	0.00	0.00	0.00	0.11	0.11	
					0.47	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.36	0.11	706.04	0.00	0.44	0.09	0.35	
Total Impervious Cover Treated (acres)	0.11							
Total Turf Area Treated (acres)	0.36							
Total TP Load Reduction Achieved in D.A. (Ib/yr)	0.09							
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00							

Water Quality Calculations - Town of Vienna

6 of Total
0
56
44

STR. 5-2A (FILTERRA)

Drainage Area D Summary

Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0
Managed Turf (acres)	0.00	0.00	0.00	0.07	0.07	64
Impervious Cover (acres)	0.00	0.00	0.00	0.04	0.04	36
					0.11	

BMP Selections

Practice	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	BMP Treatment Volume (ft ³)	TP Load from Upstream Practices (lbs)	Untreated TP Load to Practice (lbs)	TP Removed (lb/yr)	TP Remaining (Ib/yr)	Downstream Treatment to be Employed
14.b. Manufactured Treatment Device- Filtering	0.07	0.04	201.47	0.00	0.13	0.06	0.06	
Total Impervious Cover Treated (acres)	0.04							
Total Turf Area Treated (acres)	0.07							
Total TP Load Reduction Achieved in D.A. (lb/yr)	0.06							
Total TN Load Reduction Achieved in D.A. (Ib/yr)	0.00							

Drainage Area Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
Forest/Open (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Managed Turf (acres)	0.24	0.15	0.36	0.07	0.00	0.82
Impervious Cover (acres)	0.19	0.15	0.11	0.04	0.00	0.49
Total Area (acres)	0.43	0.30	0.47	0.11	0.00	1.31

Drainage Area Compliance Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
TP Load Reduced (lb/yr)	0.27	0.21	0.09	0.06	0.00	0.63
TN Load Reduced (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00

% of Total					
0					
77					
23					

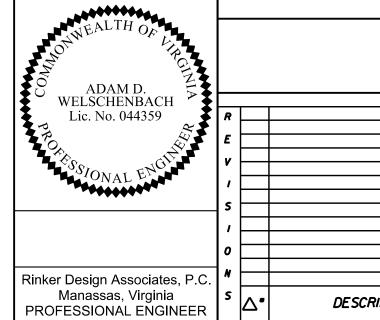
TP Remaining

(lb/yr)

0.21

Downstream Treatment

to be Employed

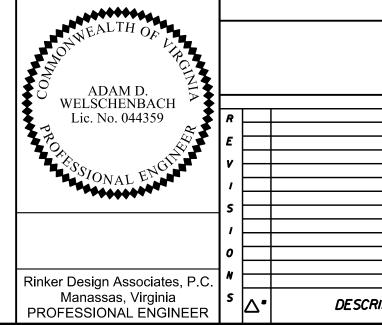


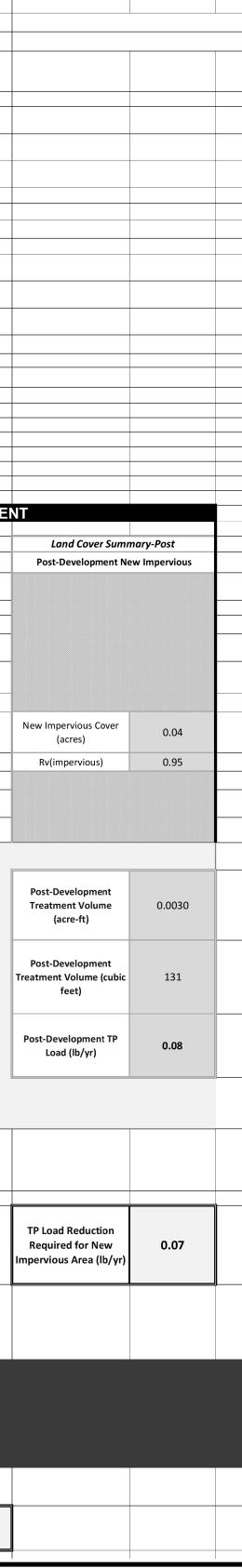
						TAX M	IAP 29-3	
EME	RGENC	Y POL	- ICE -	FIRE - RE	SCUE	911		
		DEPARTME	NT OF F	NA, VIRGIN Public works , vienna, va. 2				
				DEPA		F PUBLIC W(55-6380	DRKS	
				OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS Water Quality Calculations HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA				
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED B DRAFTED B CHECKED B	Y: LKG.JRB	SHEET 2K(IIb)	
							FUND*	

Project Name:	Old Courtho		-	vements - Phase 2		CLEAF	R ALL	data input cells		
Date:			5/4/2016 elopment Project?	Yes				constant values calculation cells		
Site Information								final results		
Post-Development Project	: (Treatme	nt Volume	and Loads)							
· ·			-	d Area <i>(acres)</i> →	0.22]		Check:		
				reduction required:				Linear project?	No	ds & Specs
	ı			ous cover (acres) is: tion for Site (lb/yr):			and cover areas ent. Total disturbed	tered correctly? d area entered?	\checkmark	
Pre-ReDevelopment Land Cover (acre	25)									
orest/Open Space (acres) undisturbed,	A Soils	B Soils	C Soils	D Soils	Totals					
protected forest/open space or reforested land Managed Turf (acres) disturbed, graded for				0.09	0.09]				
vards or other turf to be mowed/managed mpervious Cover (acres)				0.05	0.08					
					0.22					
Post-Development Land Cover (acres) A Soils	B Soils	C Soils	D Soils	Totals					
Forest/Open Space (acres) undisturbed, protected forest/open space or reforested land				0.00	0.00					
Managed Turf (acres) disturbed, graded for vards or other turf to be mowed/managed				0.10	0.10					
mpervious Cover (acres) Area Check	ОК.	ОК.	ОК.	0.12 ОК.	0.12					
Constants Annual Rainfall (inches)	43		Runoff Coefficier	nts (Rv) A Soils	B Soils	C Soils	D Soils			
Target Rainfall Event (inches) Total Phosphorus (TP) EMC (mg/L)	1.00 0.26		Forest/Open Space Managed Turf	0.02 0.15	0.03	0.04	0.05			
Total Nitrogen (TN) EMC (mg/L) Target TP Load (lb/acre/yr)	1.86 0.41		Impervious Cover	0.95	0.95	0.95	0.95			
Pj (unitless correction factor) LAND COVER SUMMARY P							R SUMMARY P		ODME	
LAND COVER SOMMART F				Land Cover Summa			Land Cover Sur			Land C
Pre-ReDevelopment	Listed	Adjusted ¹		Post ReDev. & Ne Forest/Open Space			Post-ReDeve	-		Post-Deve
Forest/Open Space Cover (acres) Weighted Rv(forest)	0.09	0.05		Cover (acres) Weighted Rv(forest)	0.00		Forest/Open Space Cover (acres) Weighted Rv(forest)	0.00		-
% Forest	41%	29%		% Forest Managed Turf Cover	0%		% Forest Managed Turf Cover	0%		-
Managed Turf Cover (acres)	0.05	0.05		(acres)	0.10		(acres)	0.10		-
Weighted Rv(turf) % Managed Turf	0.25 	0.25		Weighted Rv (turf) % Managed Turf	0.25 47%	-	Weighted Rv (turf) % Managed Turf	0.25		-
Impervious Cover (acres)	0.08	0.08		Impervious Cover	0.12		ReDev. Impervious	0.08		New Impervio
Rv(impervious)	0.95	0.95		(acres) Rv(impervious)	0.95		Cover (acres) Rv(impervious)	0.95		(acres Rv(imperv
% Impervious	35%	43%		% Impervious	53%		% Impervious Total ReDev. Site Area	43%		-
Total Site Area (acres) Site Rv	0.22	0.18		Final Site Area (acres) Final Post Dev Site Rv	0.22		(acres) ReDev Site Rv	0.18		
Treatment Volume and	d Nutrient Lo	ad				Trea	tment Volume an	d Nutrient Loa	ad	
Pre-ReDevelopment Treatment Volume (acre-ft)	0.0075	0.0074		Final Post- Development Treatment Volume (acre-ft)	0.0113		Post-ReDevelopment Treatment Volume (acre-ft)	0.0082		Post-Develo Treatment V (acre-1
Pre-ReDevelopment Treatment Volume (cubic feet)	328	321		Final Post- Development Treatment Volume (cubic feet)	490		Post-ReDevelopment Treatment Volume (cubic feet)	359		Post-Develo Treatment Volu feet)
Pre-ReDevelopment TP Load (lb/yr)	0.21	0.20		Final Post- Development TP Load (lb/yr)	0.31		Post-ReDevelopment Load (TP) (lb/yr)*	0.23		Post-Develop Load (lb,
Pre-ReDevelopment TP Load per acre (lb/acre/yr)	0.95	1.12		Final Post-Development TP Load per acre (Ib/acre/yr)	1.41		Post-ReDevelopment TP Load per acre (lb/acre/yr)	1.25		
Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious cov		0.07					Max. Reduction Required (Below Pre- ReDevelopment Load)	20%		
¹ Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious la managed turf) acreage proposed for new imperv.	ious cover.						TP Load Reduction Required for Redeveloped Area (lb/yr)	0.06		TP Load Red Required fo Impervious Ar
Adjusted total acreage is consistent with Post-Rel acreage of new impervious cover). Column I shows load reduction requriement for n										
development load limit, 0.41 lbs/acre/year).			- Deat De-	(alapment P	irone ent-f	Site Ano				
			Post-Dev	/elopment Requ	irement for	Site Area				
				Reduction Required		0.13				
			Linear Pr	oject TP Load Reductio	n Required (lb/yr)	: 0.14				
			Nit	rogen Loads (Info	rmational Pur	poses Only)				
	Pre-ReDevelopm	ent TN Load (lb/yr)	1.47			(Post-ReD	Development TN Load Development & New	2.20		
						l Impe	ervious) (lb/yr)			1

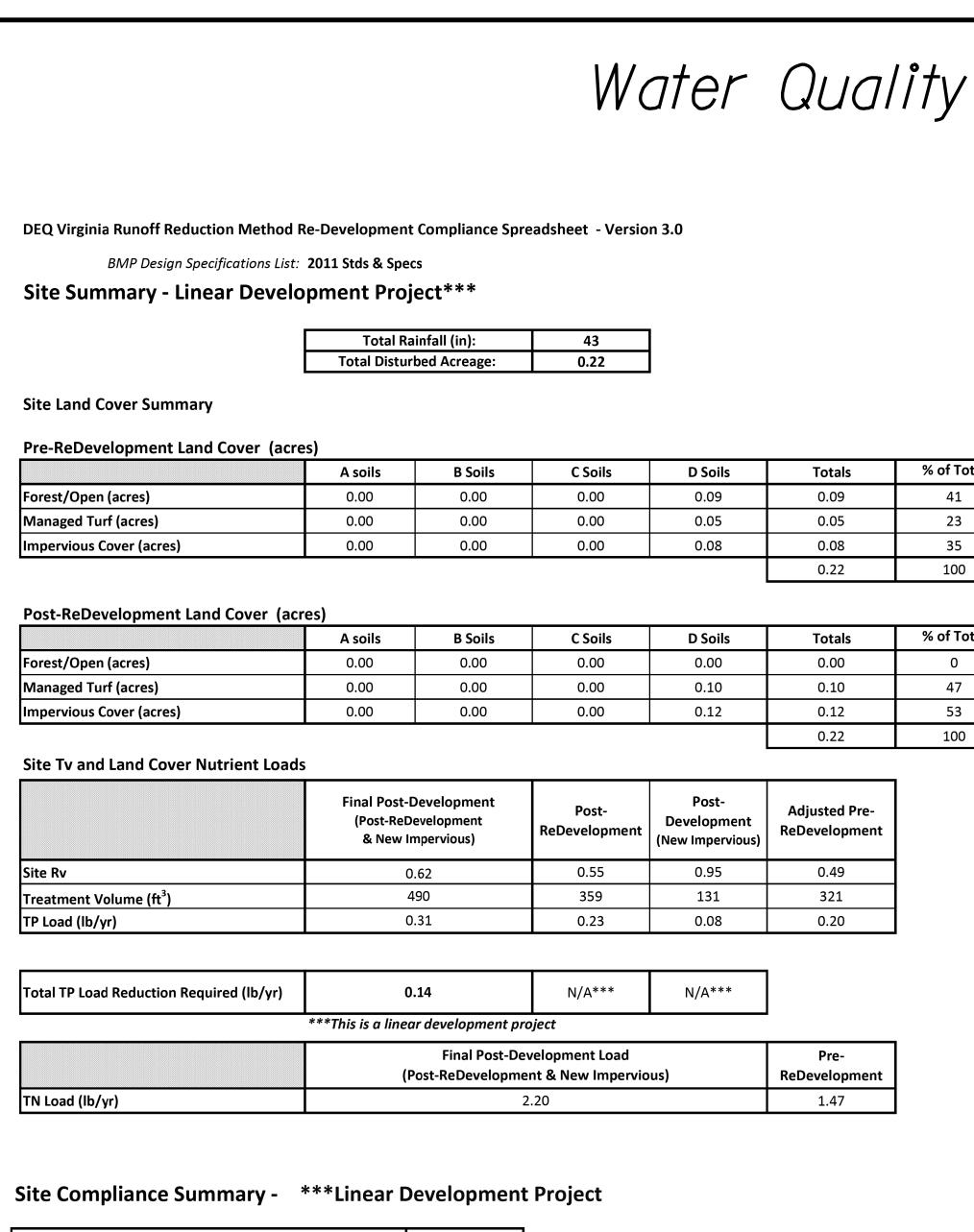
ty Calculations - Fairfax County

Area Checks	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	AREA CHECK
FOREST/OPEN SPACE (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
IMPERVIOUS COVER (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
IMPERVIOUS COVER TREATED (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
MANAGED TURF AREA (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
MANAGED TURF AREA TREATED (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
AREA CHECK	OK.	ОК.	ОК.	ОК.	ОК.	
Site Treatment Volume (ft ³)	490]				
Runoff Reduction Volume and TP By Drainage Area						
	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	TOTAL
RUNOFF REDUCTION VOLUME ACHIEVED (ft ³)	0	0	0	0	0	0
TP LOAD AVAILABLE FOR REMOVAL (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
TP LOAD REDUCTION ACHIEVED (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
TP LOAD REMAINING (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
NITROGEN LOAD REDUCTION ACHIEVED (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
	0.00	0.00	0.00	0.00	0.00	0.00
Total Phosphorus			LINEAR PROJECT:			
FINAL POST-DEVELOPMENT TP LOAD (lb/yr)			0.31			
TP LOAD REDUCTION REQUIRED (lb/yr)			0.14			
TP LOAD REDUCTION ACHIEVED (lb/yr)			0.00			
TP LOAD REMAINING (lb/yr):			0.31			
REMAINING TP LOAD REDUCTION REQUIRED (lb/yr):			0.14	-		
Total Nitrogen (For Information Purposes)						
	2.20	1				
POST-DEVELOPMENT LOAD (lb/yr) NITROGEN LOAD REDUCTION ACHIEVED (lb/yr)	2.20					
REMAINING POST-DEVELOPMENT NITROGEN LOAD (Ib/yr)	2.20					





						TAX	MAP 29-3	
EME	RGENC	Y POL	ICE -	FIRE - RE	SCUE	911		
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				DEPA		F PUBLIC W 5-6380	ORKS	
				OLD COURTHOUSE ROAD PEDESTRIAN ACCESS IMPROVEMENTS				
					•	<pre>v Calculation FAIRFAX COUNTY</pre>		
				SCALE	DESIGNED B	Y. ADW. P.F.	SHEET	
RIPTION	BY	APPROVED	DATE	HORIZ N/A VERT N/A	DRAFTED BY	: LKG.JRB	2K(IIc)	
							FUND"	



Maximum % Reduction Required Below Pre-ReDevelopment Load

20%

Total Runoff Volume Reduction (ft ³)	0
Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (Ib/yr)	031
Remaining TP Load Reduction (lb/yr) Required	0.14

Water Quality Calculations - Fairfax County

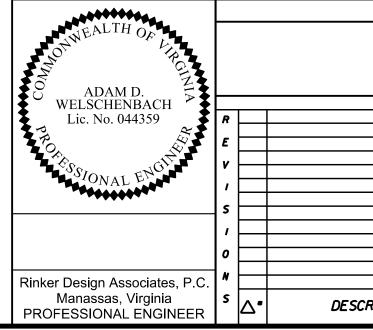
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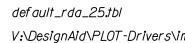
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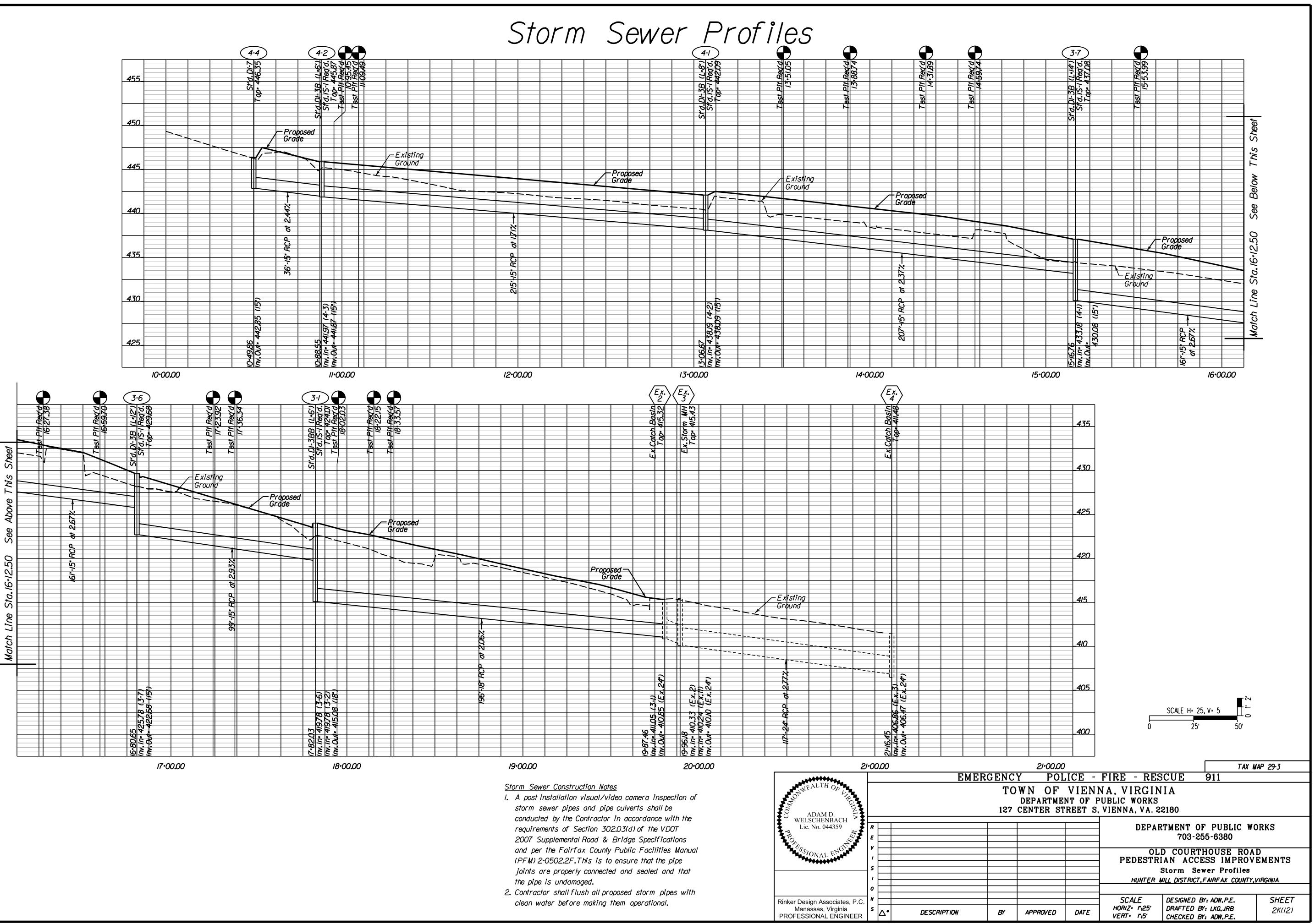
Pre- ReDevelopment TP Load per acre (lb/acre/yr)	Final Post-Development TP Load per acre (Ib/acre/yr)	Post-ReDevelopment TP Load per acre (Ib/acre/yr)
1.12	1.41	1.25

NOTE: BMP requirements within Fairfax County are proposed to be met through the purchase of nutrient credits.



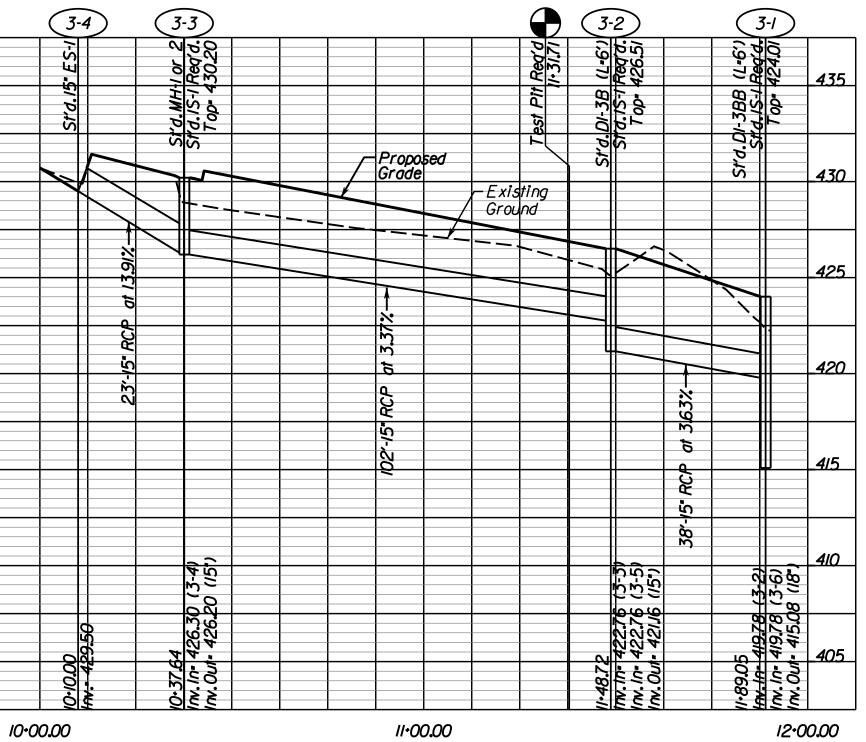
						MAP 29-3
EMER	ТО	WN OF DEPARTME	VIENI NT OF P	<u>FIRE - RES</u> NA, VIRGIN PUBLIC WORKS VIENNA, VA. 2	IIA	
				DEPAR	RTMENT OF PUBLIC V 703-255-6380	ORKS
				PEDESTRI Wa	D COURTHOUSE RO IAN ACCESS IMPRO ter Quality Calculatio WILL DISTRICT, FAIRFAX COUNT	VEMENTS ns
RIPTION	BY	APPROVED	DATE	SCALE HORIZ= N/A VERT= N/A	DESIGNED BY: ADW.P.E. DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	SHEET 2K(IId)



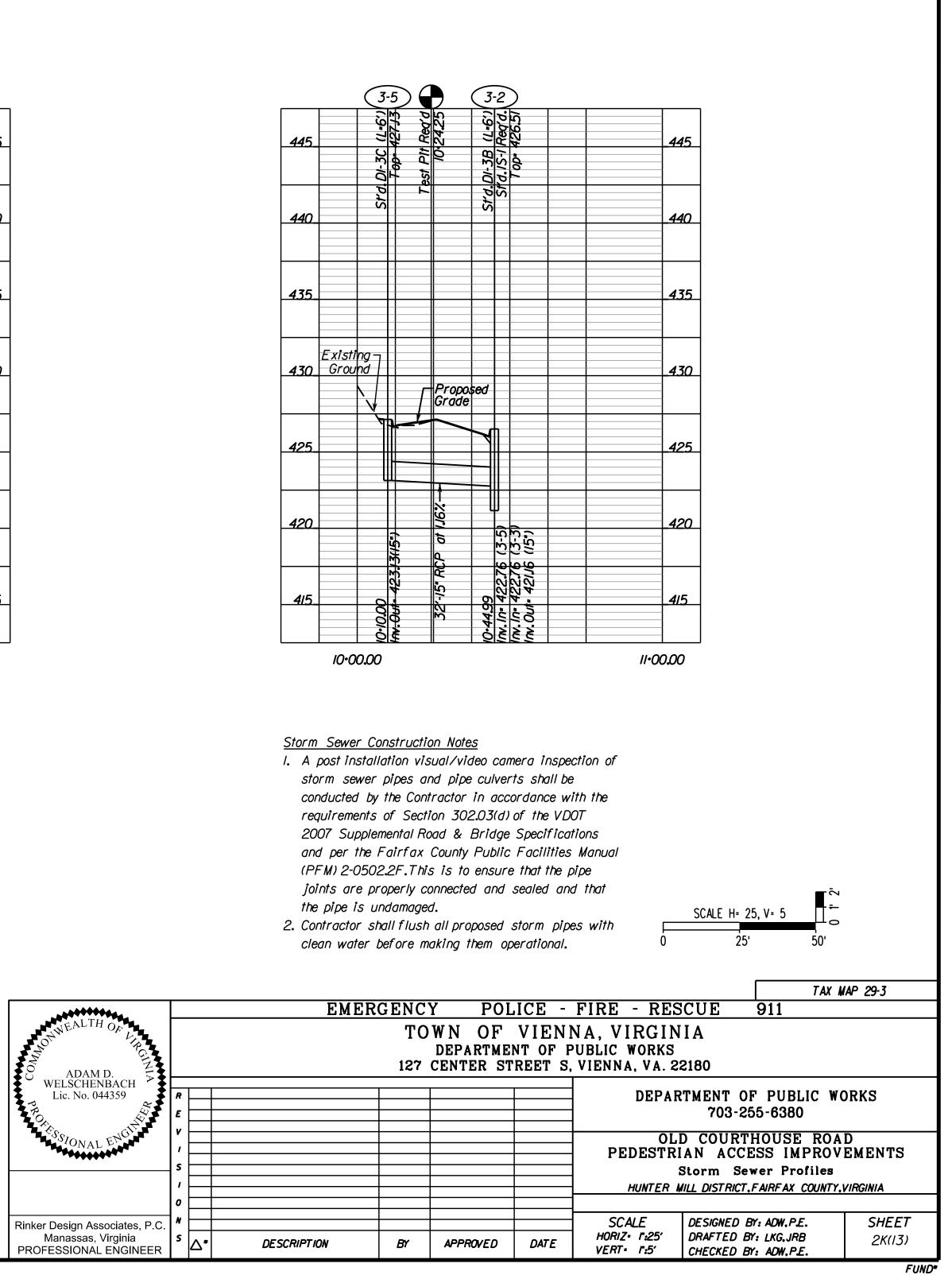


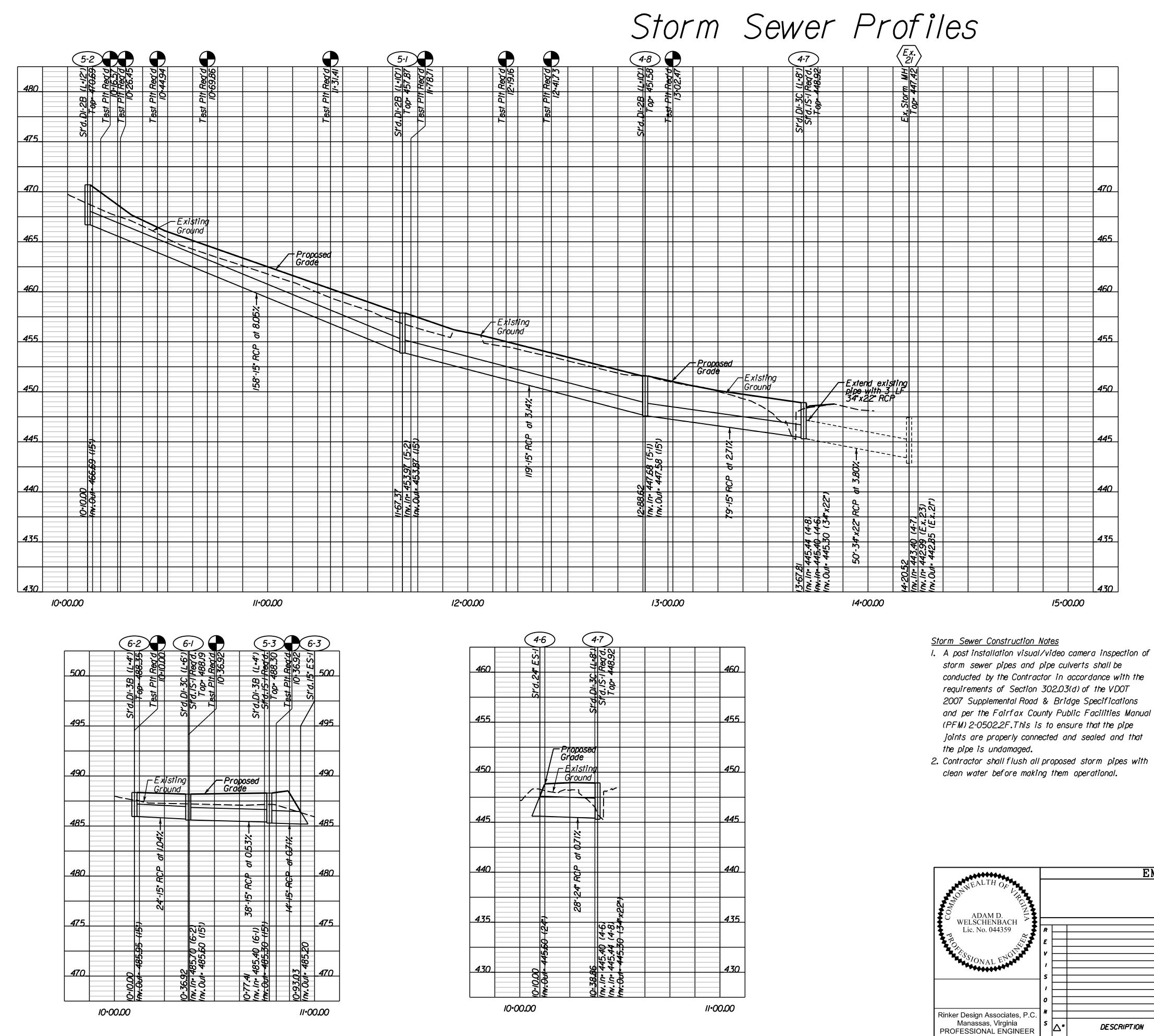
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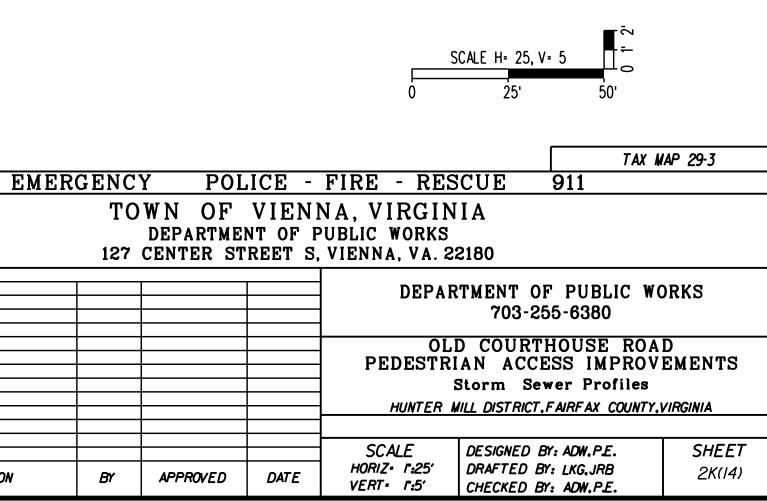
Storm Sewer Profiles







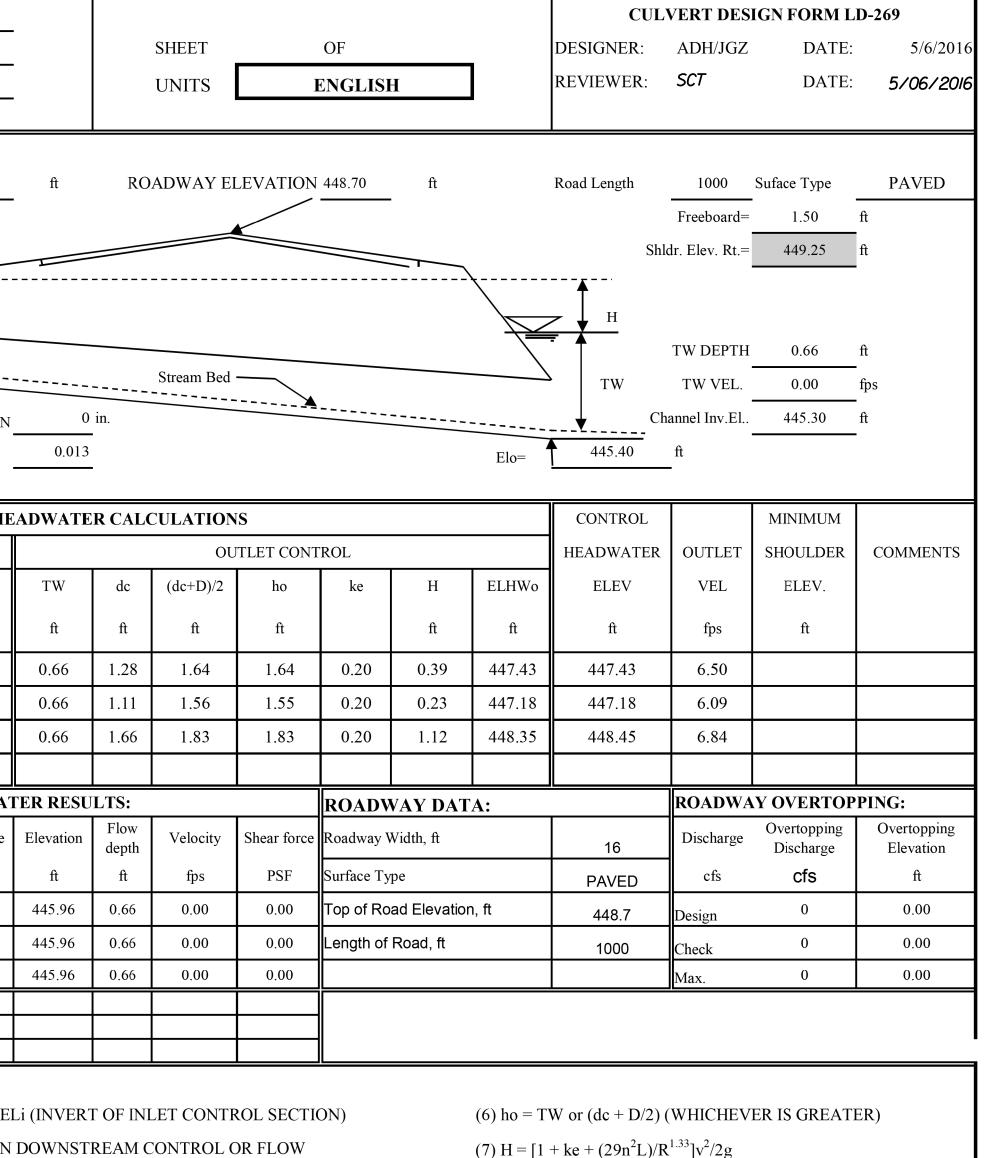




CULVERT COMPUTATION FOR STR. 4-6 HEADWATER DEPTH

PROJECT	Old Court	thouse Rd 1	[mprove	ments						
ROAD	Old Court	thouse Rd					COUNTY	Fairfax		
CULVERT	4-6							VA		
	21+99				40' LT					
	HYDROL	OGICAL D	ATA							
Method:	INPUT							Roadwa	y Width	16
Drainage Area:	8.81			_						
Time of Concer	ntration	16.9			Sł	nldr. Elev. Lt. =	448.70	ft		
DESIGN FLO	WS					ELHWd=	447.20		- <u>-</u>	·-¥/
R.I. (years)		FLOW (cfs))		Elst	£= 445.60	ft		Hwi	
10	Design	12.70			Eli	i= 445.60	ft		- ~	\
2	Check	9.62			L	= 28	ft			
100	Max.	21.48			S	= 0.71%	-) DEPRESSIO
CULVERT DI		N:			SKEW	<i>r</i> = 0	0		Ľ	"n" Bed
TYPE:		tiple Confor	ming				-			II Ded
Inlet Edge Des	-		8		TOTAL	FLOW PER				Н
Groove End Pro	-				FLOW	BARREL		INLET CO	NTROL	
	ngle / Multiple Conforming / Broken				Q	Q/N	HWi/D	HWi	FALL	ELHWi
				Mannings						
MATERIAL	SHAPE	Size (in)	N	n	cfs	cfs		ft	ft	ft
Concrete	Circular	24	1	0.01	12.70	12.7	0.91	1.60	0.00	447.20
					9.62	9.6	0.79	1.29	0.00	446.89
					21.48	21.5	1.42	2.85	0.00	448.45
Broken Back	Culvert			TAILWA	TER DATA	•				TAILWA
LENGTH	Elev.	SKEW ⁰		Channel Sha	pe					Discharge
				Bottom Wid	th. ft	0.00	"n" =	0.035]	cfs
				Side Slope		0.00			4	Design
				Side Slope		0.00				Check
				Channel Slo		0.0100				Max.
				Distance						
				Elevation						<u> </u>
TECHNICAL	ΕΛΟΤΝΟΤΙ	75.		"n" =						
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(2) $HWi/D = H$										BASED O
(3) FALL = HV	×		ll 15 ZE	KU FUR CUL	LVERIS ON C					TH IN CHA
	CRIPT DEFINITIONS:				N H N = =		COMMENT	IS / DISCU	5510N:	
HWd		EADWATE		i	INLET					
HWi		LET CONTR		0	OUTLET					
HWo	HW IN OU	TLET CON	ΓROL	sf	Streambed					
					(a) culvert fac	e				

Storm Computations (For Information Only, Phase I Element)

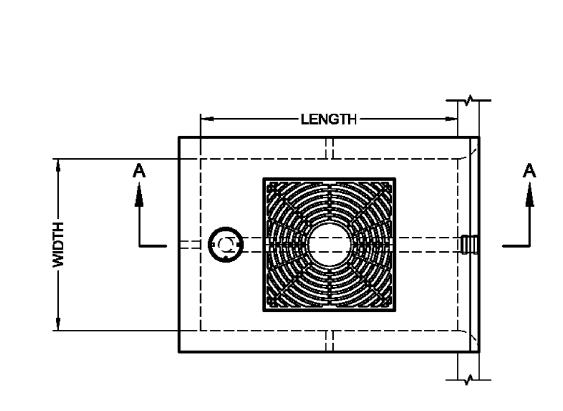


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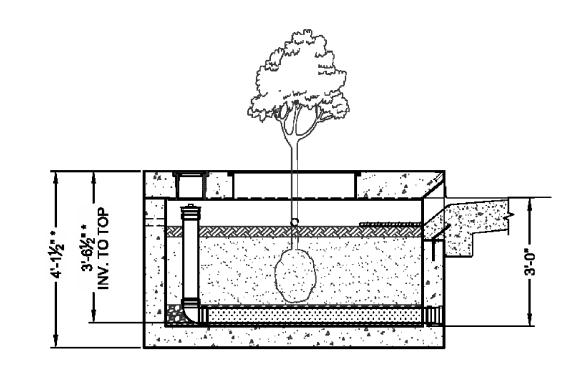
(7) H = $[1 + ke + (29n^2L)/R^{1.33}]v^2/2g$

	CULVER	T BARREL SELECTED	
SIZE	8:	n:	
SHA	PE:	MATERIAL:	
ENT	RANCE:	-	

									TAX	MAP 29-3
ſ				EMER	GENC	Y POL	ICE -	FIRE - RES	CUE 911	
	ADAM D.					DEPARTME	NT OF P	NA, VIRGIN Ublic works Vienna, va. 22		
	WELSCHENBACH Lic. No. 044359	R E						DEPAR	TMENT OF PUBLIC W 703-255-6380	ORKS
	TESC ENCITE	V							COURTHOUSE ROA	
	TONAL ET	1							AN ACCESS IMPROV	
ł		s							Culvert Computation ILL DISTRICT.FAIRFAX COUNTY	
		0							ILL DISTRICT, FAIRFAA COUNT	, VINGINIA
ŀ	Rinker Design Associates, P.C.	•						SCALE	DESIGNED BY: ADW.P.E.	SHEET
	Manassas, Virginia PROFESSIONAL ENGINEER	S	∆ •	DESCRIPTION	BY	APPROVED	DATE	HORIZ= N/A VERT= N/A	DRAFTED BY: LKG.JRB CHECKED BY: ADW.P.E.	2K(I5)





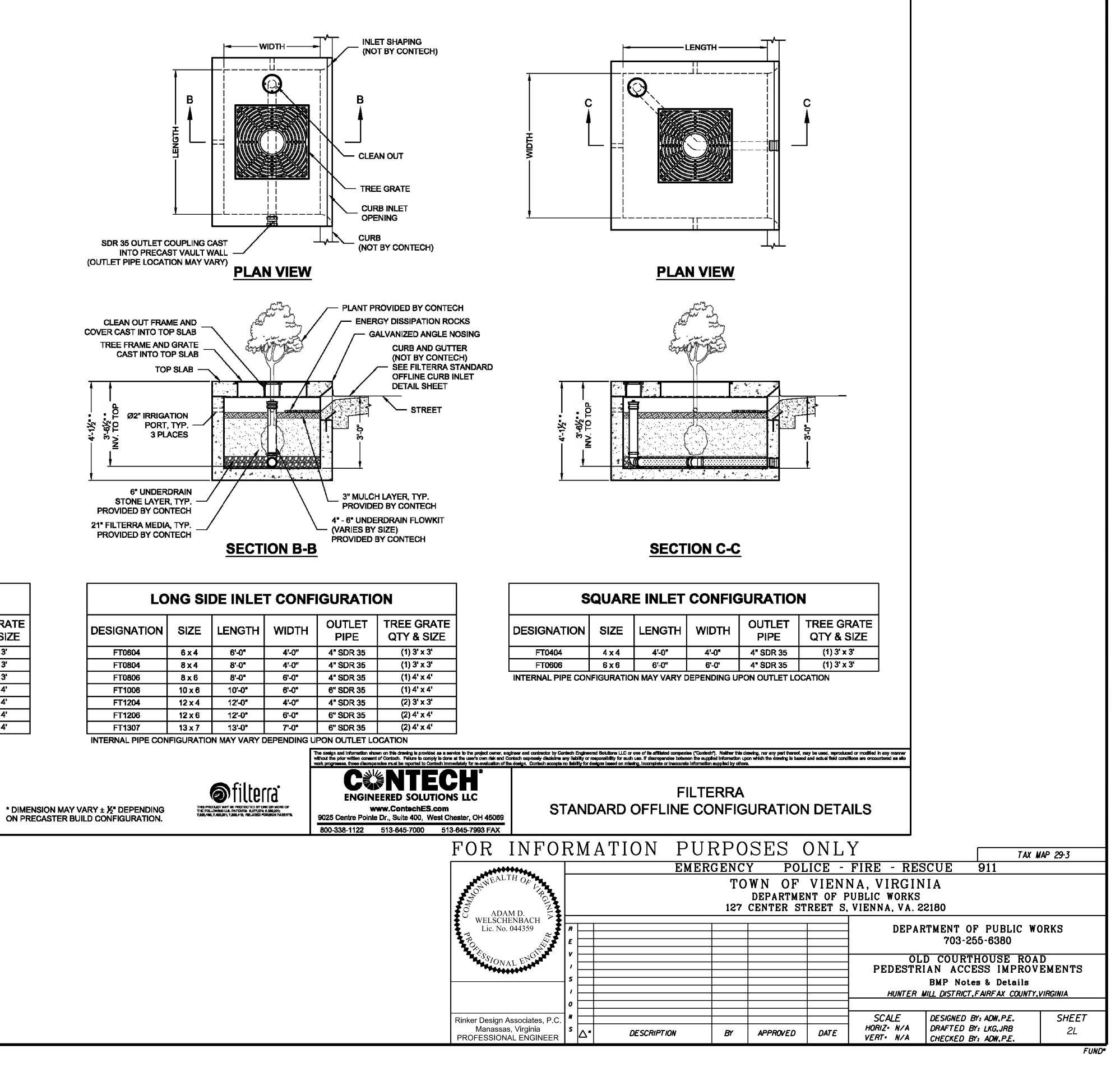


SECTION A-A

SHC	ORT SI	IDE INLE		IGURATI	ON						
FT0408 4 x 8 8'-0" 4'-0" 4" SDR 35 (1) 3 FT0412 4 x 12 12'-0" 4'-0" 4" SDR 35 (2) 3 FT0608 6 x 8 8'-0" 6'-0" 4" SDR 35 (1) 4 FT0610 6 x 10 10'-0" 6'-0" 6" SDR 35 (1) 4 FT0612 6 x 12 12'-0" 6'-0" 6" SDR 35 (2) 4											
FT0406	4 x 6	6'-0"	4'-0"	4" SDR 35	(1) 3' x 3'						
FT0408	4 x 8	8'-0"	4'-0"	4" SDR 35	(1) 3' x 3'						
FT0412	4 x 12	12'-0"	4'-0"	4" SDR 35	(2) 3' x 3'						
FT0608	6 x 8	8'-0"	6'-0"	4" SDR 35	(1) 4' x 4'						
FT0610	6 x 10	10'-0"	6'-0"	6" SDR 35	(1) 4' x 4'						
FT0612	6 x 12	12'-0"	6'-0"	6" SDR 35	(2) 4' x 4'						
FT0713	7 x 13	13'-0"	7'-0"	6" SDR 35	(2) 4' x 4'						

Note: See drainage descriptions on Sheet 2K for unit sizing.

Filterra Notes & Details



FILTERRA STANDARD PLAN NOTES

Construction & Installation

The Contractor is responsible for correct installation of Filterra units as shown in plans. Α.

Each unit shall be constructed at the locations and elevations according to the sizes shown on the approved Β. drawings. Any modifications to the elevation or location shall be at the direction of and approved by the Engineer. C. If the Filterra® is stored before installation, the top slab must be placed on the box using the 2x4 wood provided, to prevent any contamination from the site. All internal fittings supplied (if any), must be left in place as per the delivery. D. The unit shall be placed on a compacted sub-grade with a minimum 6-inch gravel base matching the final grade of the curb line in the area of the unit. The unit to be placed such that the unit and top slab match the grade of the curb in the area of the unit. Compact undisturbed sub-grade materials to 95% of maximum density at +1- 2% of optimum moisture. Unsuitable material below sub-grade shall be replaced to the site engineer's approval.

Outlet connections shall be aligned and sealed to meet the approved drawings with modifications necessary to E. meet site conditions and local regulations.

F. Once the unit is set, the internal wooden forms and protective mesh cover must be left intact. Remove only the temporary wooden shipping blocks between the box and top slab. The top lid should be sealed onto the box section before backfilling, using a nonshrink grout, butyl rubber or similar waterproof seal. The boards on top of the lid and boards sealed in the unit's throat must NOT be removed. The Supplier (Americast or its authorized dealer) will remove these sections at the time of activation. Backfilling should be performed in a careful manner, bringing the appropriate fill material up in 6"lifts on all sides. Precast sections shall be set in a manner that will result in a watertight joint. In all instances, installation of Filterra®unit shall conform to ASTM specification C891 "Standard Practice for Installation of Underground Precast Utility Structures", unless directed otherwise in contract documents.

G. Curb and gutter construction (where present) shall ensure that the flow-line of the Filterra®units is at a greater elevation than the flow-line of the bypass structure or relief (drop inlet, curb cut or similar). Failure to comply with this guideline may cause failure and/or damage to the Filterra®environmental device.

Each Filterra®unit must receive adequate irrigation to ensure survival of the living system during periods of drier H. weather. This may be achieved through gutter flow or through the tree grate.

Activation

A. Activation of the Filterra®unit is performed ONLY by the Supplier. Purchaser is responsible for Filterra®inlet protection and subsequent clean out cost. This process cannot commence until the project site is fully stabilized and cleaned (full landscaping, grass cover, final paving and street sweeping completed), negating the chance of construction materials contaminating the Filterra®system. Care shall be taken during construction not to damage the protective throat and top plates.

Activation includes installation of plant(s) and mulch layers as necessary. B.

Maintenance

A. Each correctly installed Filterra®unit is to be maintained by the Supplier, or a Supplier approved contractor for a minimum period of 1 year. The cost of this service is to be included in the price of each Filterra®unit. Extended maintenance contracts are available at extra cost upon request.

Annual maintenance consists of a maximum of (2) scheduled visits. The visits are scheduled seasonally; the spring B. visit aims to clean up after winter loads including salts and sands. The fall visit helps the system by removing excessive leaf litter.

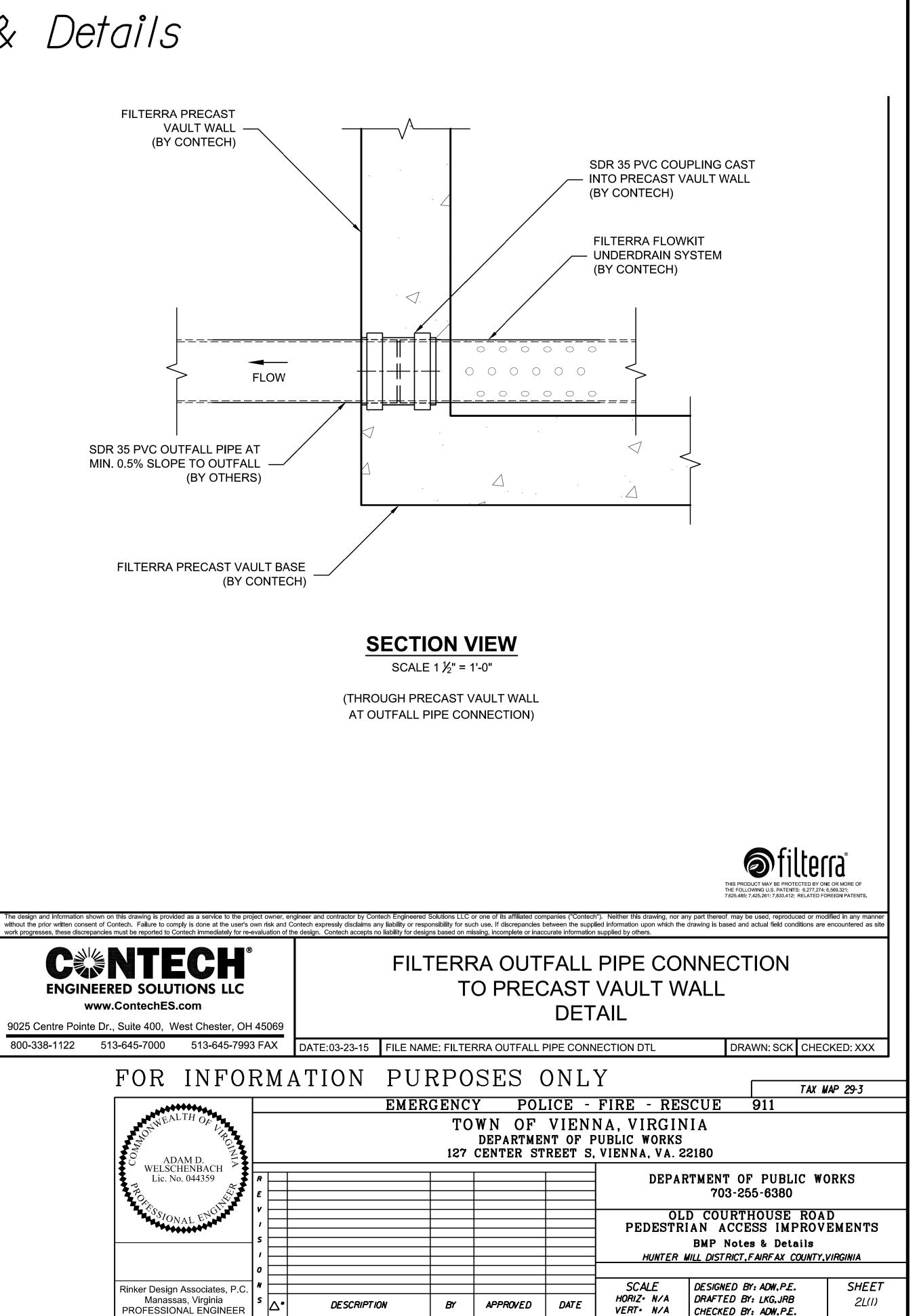
- Each maintenance visit consists of the following tasks. С.
- Filterra®unit inspection
- Foreign debris, silt, mulch & trash removal
- Filter media evaluation and recharge as necessary
- Plant health evaluation and pruning or replacement as necessary
- Replacement of mulch
- Disposal of all maintenance refuse items 6.
- Maintenance records updated and stored (reports available upon request)

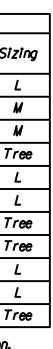
D. The beginning and ending date of Supplier's obligation to maintain the installed system shall be determined by the Supplier at the time the system is activated. Owners must promptly notify the Supplier of any damage to the plant(s), which constitute(s) an integral part of the bioretention technology.

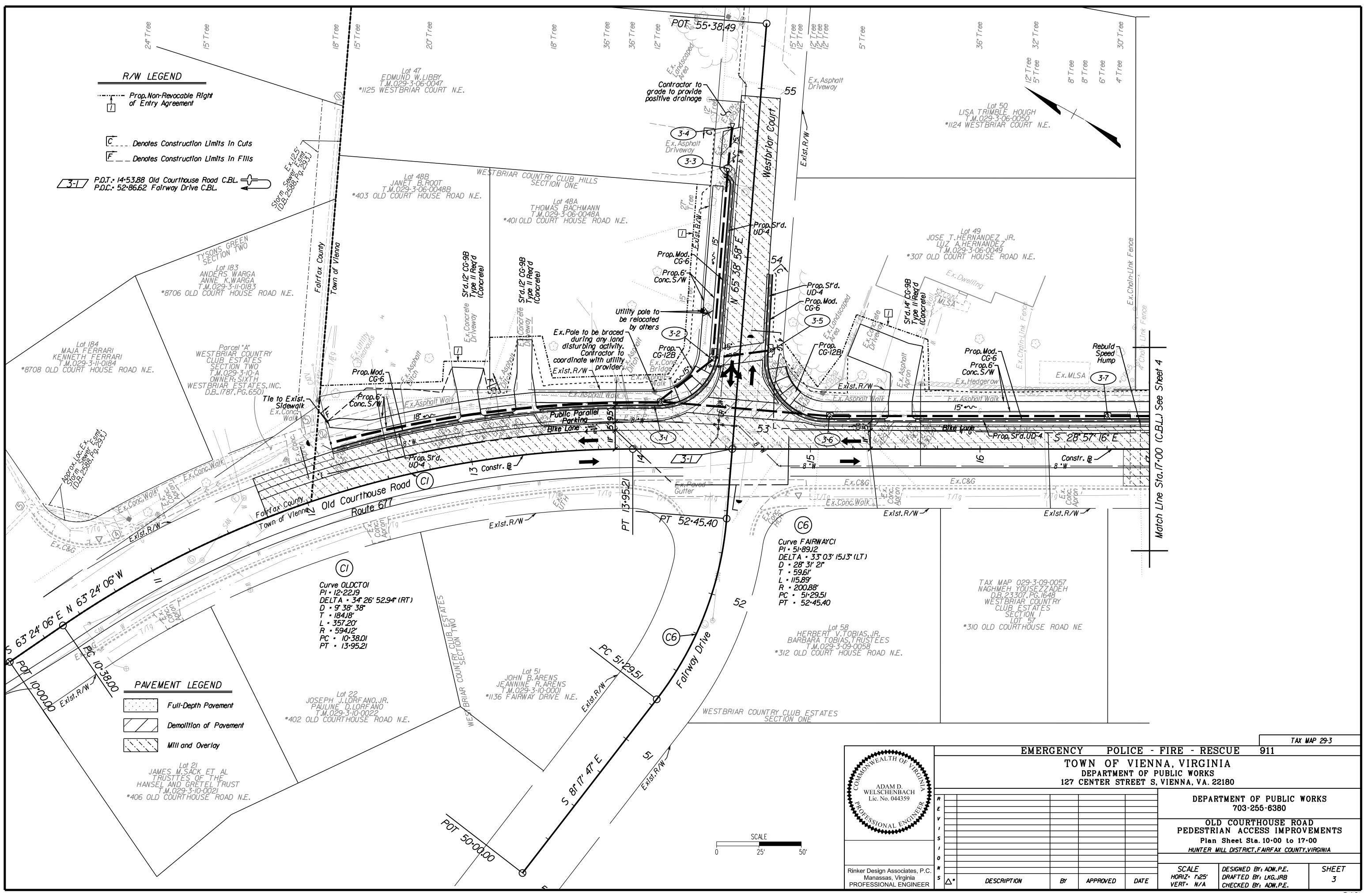
		_	FILTERRA PL	ANT LIST			-
Scientific Name	Common Name	Plant Type	Sun	Hardy Range	Height	Spread	Siz
Cephalanthus occidentalis	Buttonbush	Deciduous	Partial Shade to Full Sun	4A-10A	4'-6'	6'-10'	1
Aronia melanocarpa	Black Chokeberry	Deciduous	Full Shade to Full Sun	3B-6B	3′-6′	4'-6'	N
Aronia arbutifolia	Red Chokeberry	Deciduous	Partial Shade to Full Sun	4B-9A	6′-10′	4'-6'	N
Cornus florida	Flowering Dogwood	Deciduous	Partial Shade to Full Sun	5A-8B	15′-20′	15'-20'	Tr
Cornus amomum	Silky Dogwood	Deciduous	Full shade to Full Sun	4B-8A	8'-10'	8'-15'	L
Sambucus canadensis	American Elderberry	Deciduous	Partial Shade to Full Sun	4A-9B	10^-15′	6'-10'	L
Chionanthus virginicus	White Fringe Tree	Deciduous	Full Shade to Full Sun	4A-9A	12"-20"	10'-15'	Tr
llex decidua	Possum How Holly	Deciduous	Full Shade to Full Sun	5A-9A	15′-20′	15'-250'	Tr
llex verticillata	Winterberry Holly	Deciduous	Partial Shade to Full Sun	<i>3B</i> -9A	6′-10′	8'-15'	L
Myrica pensylvanica	Northern Bayberry	Deciduous	Partial Shade to Full Sun	3A-7A	10^-15′	6'-10'	L
Cercis canadensis	Eastern Redbud	Deciduous	Partial Shade to Full Sun	4B-9A	15'-25'	15'-25'	Tr

Note: This is a recommended planting list. For complete planting list, please refer to Filterra Vault Configuration Plant List - Mid Atlantic Region.

Filterra Notes & Details







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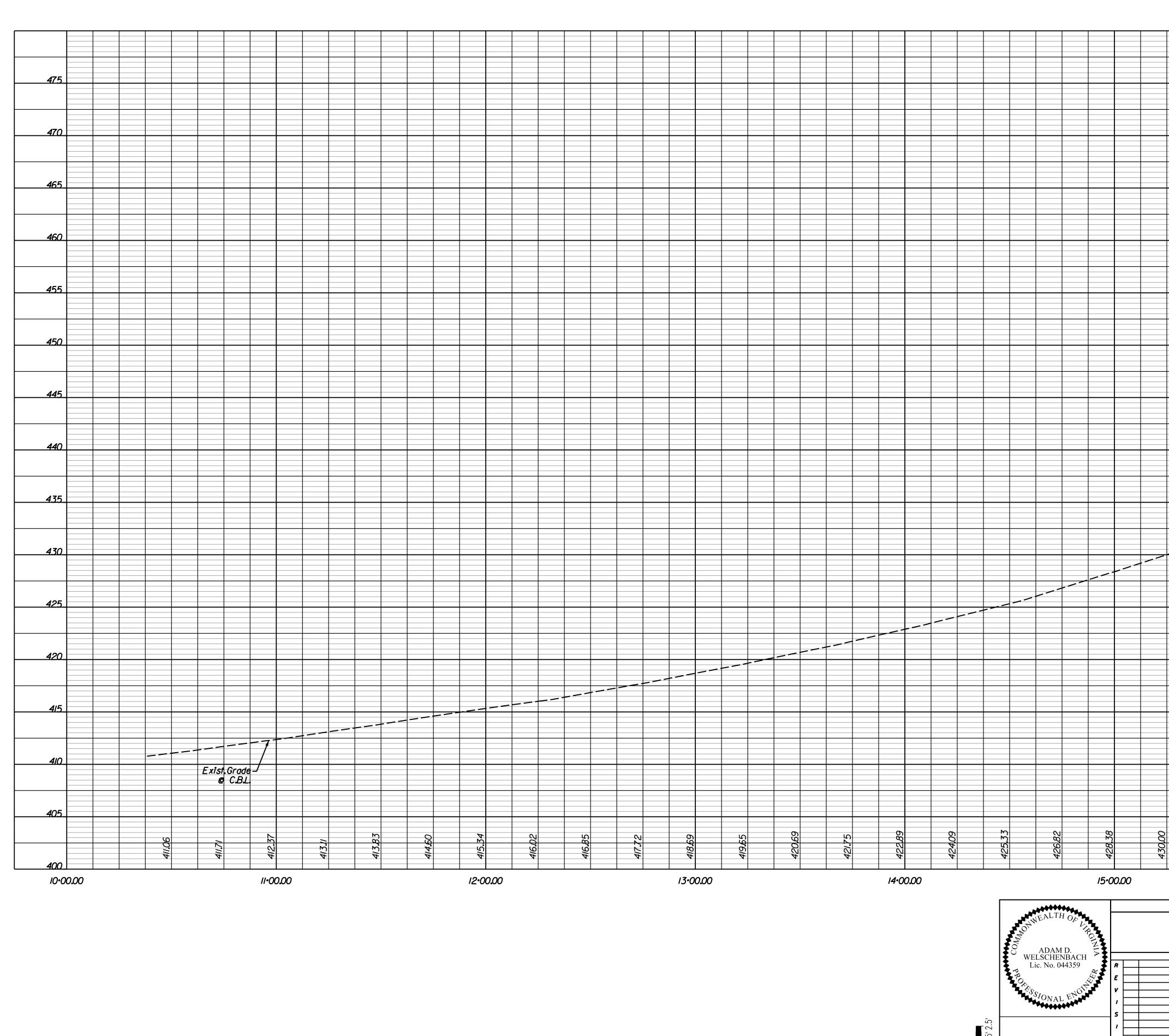
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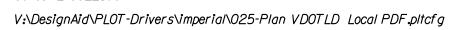
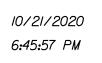




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DEPARTMENT OF PUBLIC WORKS 127 CENTER STREET S, VIENNA, VA. 22180

DEPARTMENT OF PUBLIC WORKS 703-255-6380 OLD COURTHOUSE ROAD PEDESTRIAN IMPROVEMENTS Profile Sheet Sta. 10.00 to 17.00 HUNTER MILL DISTRICT, FAIRFAX COUNTY, VIRGINIA DESIGNED BY: CMW.P.E. DRAFTED BY: T.J.W. CHECKED BY: ADW.P.E. SCALE SHEET See Scale Bars 3A APPROVED DATE DESCRIPTION BY



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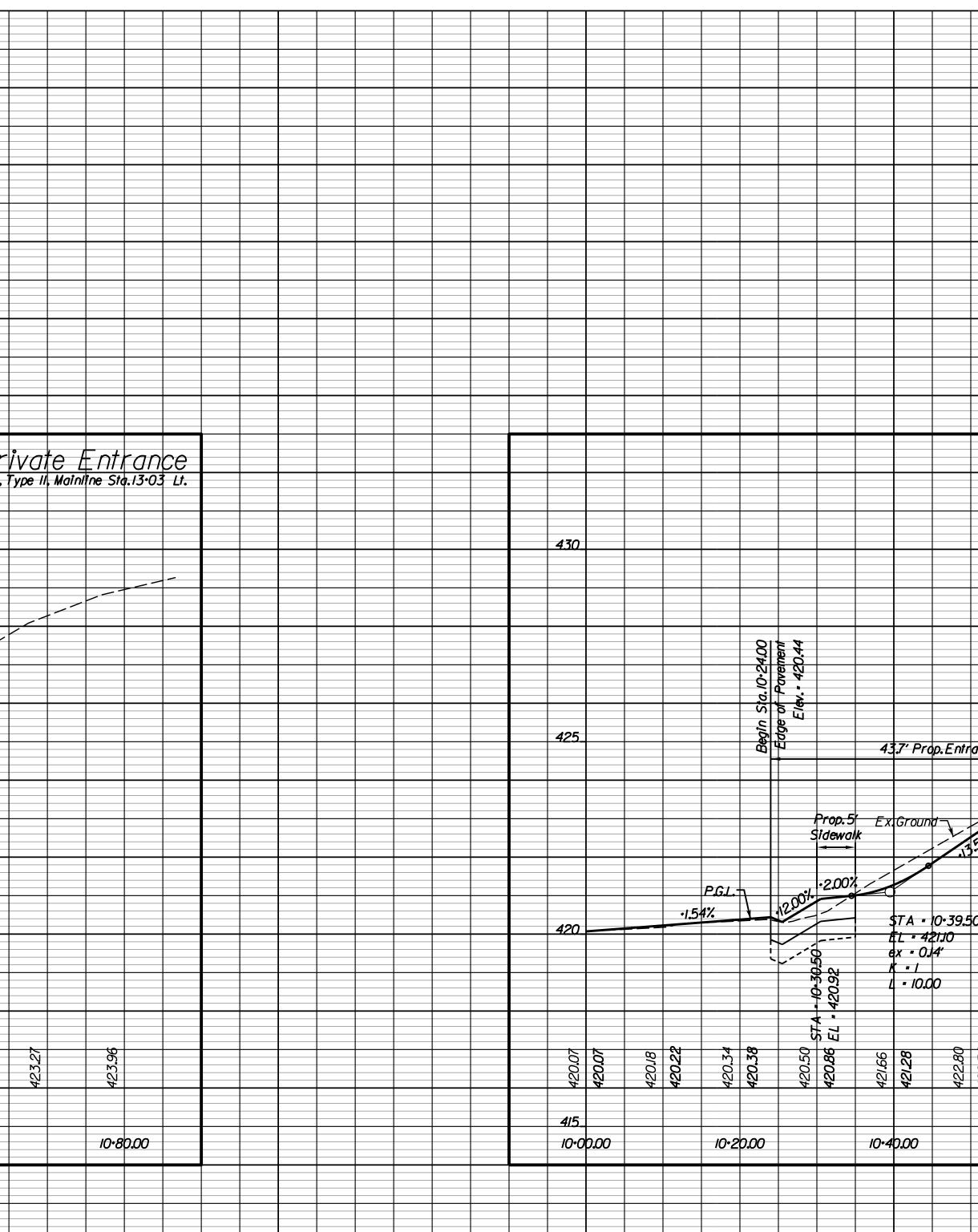
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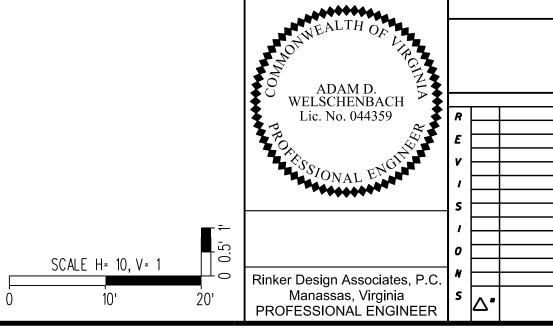
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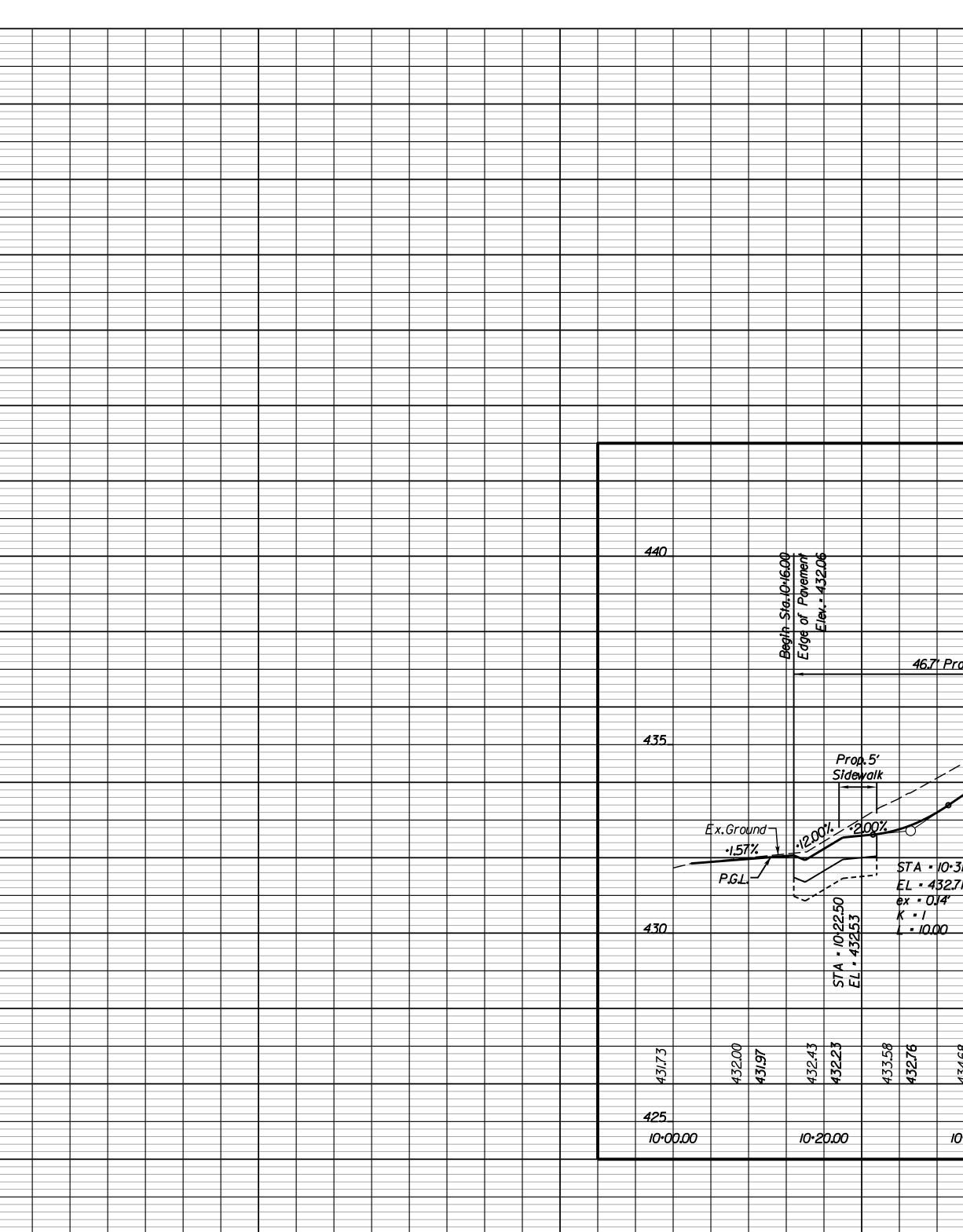
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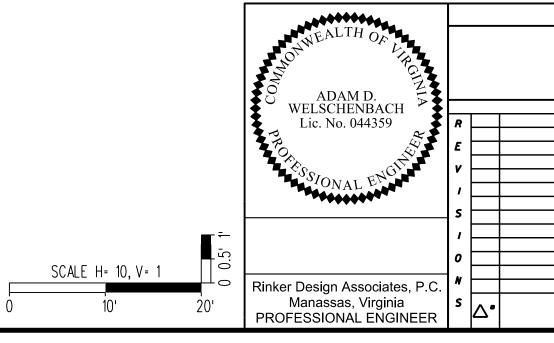
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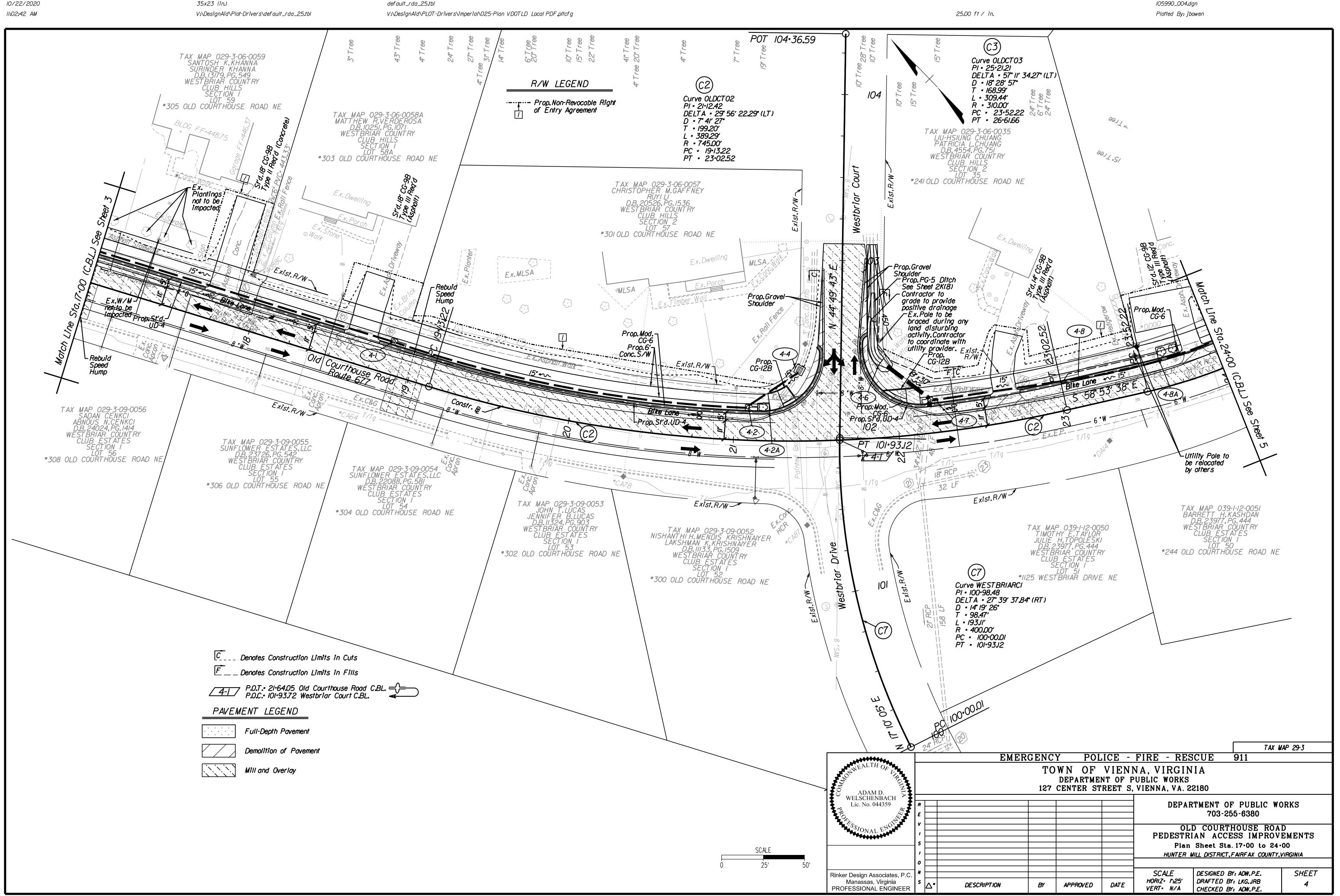
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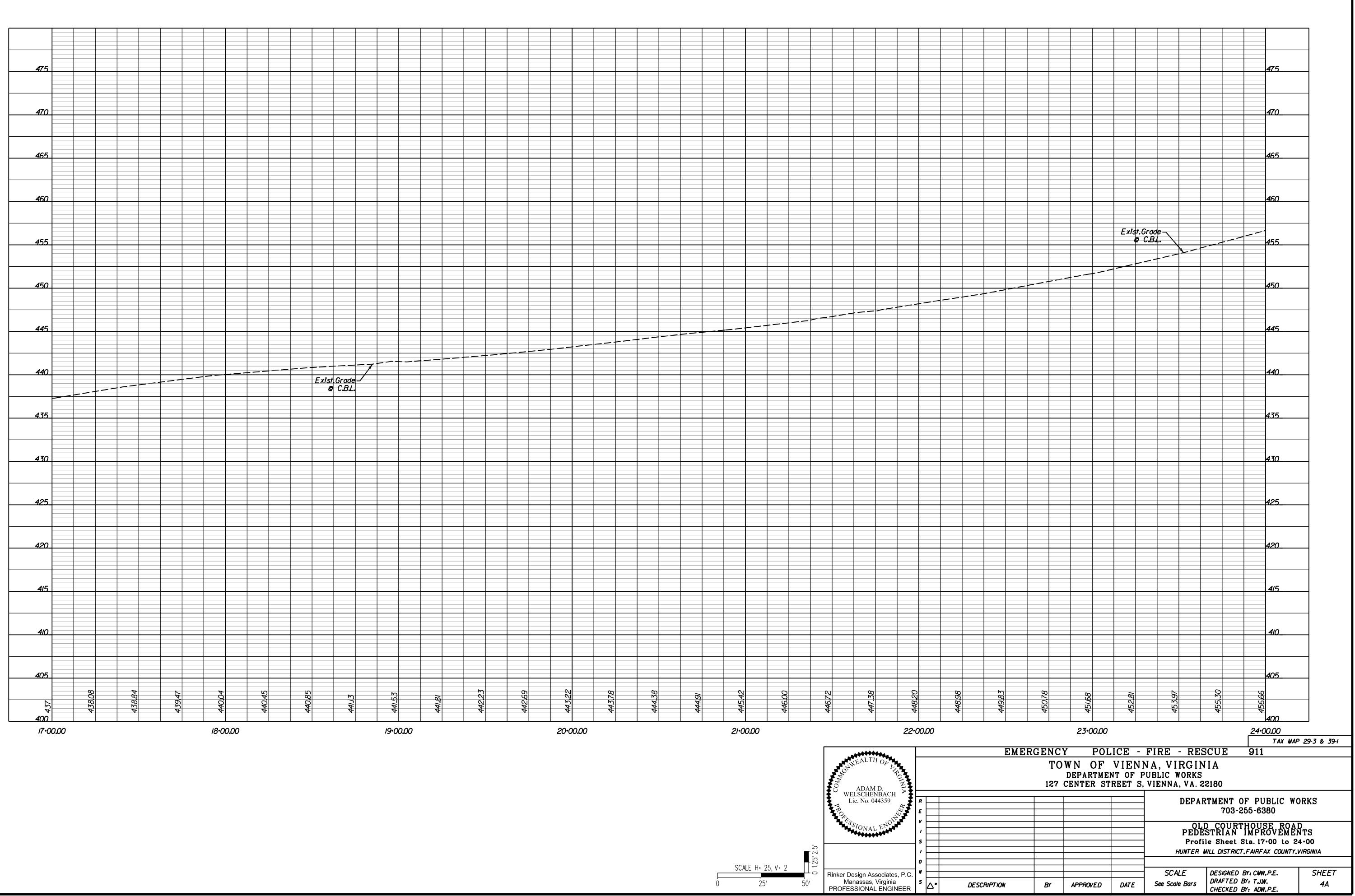


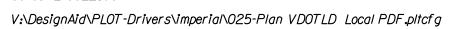


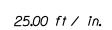
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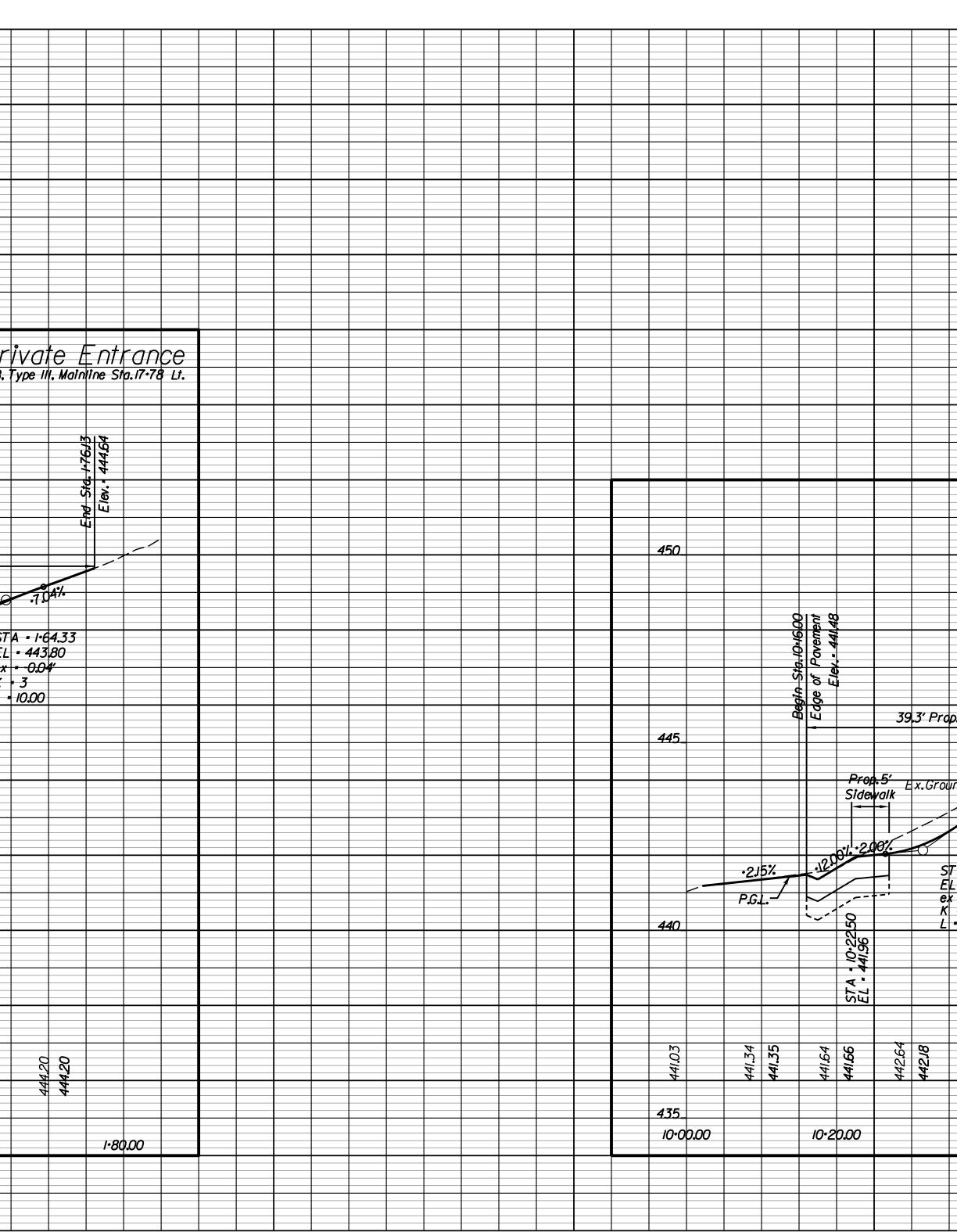
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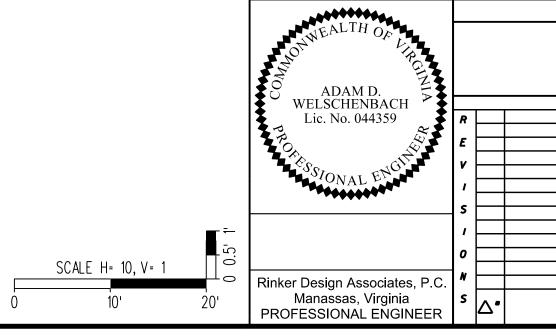
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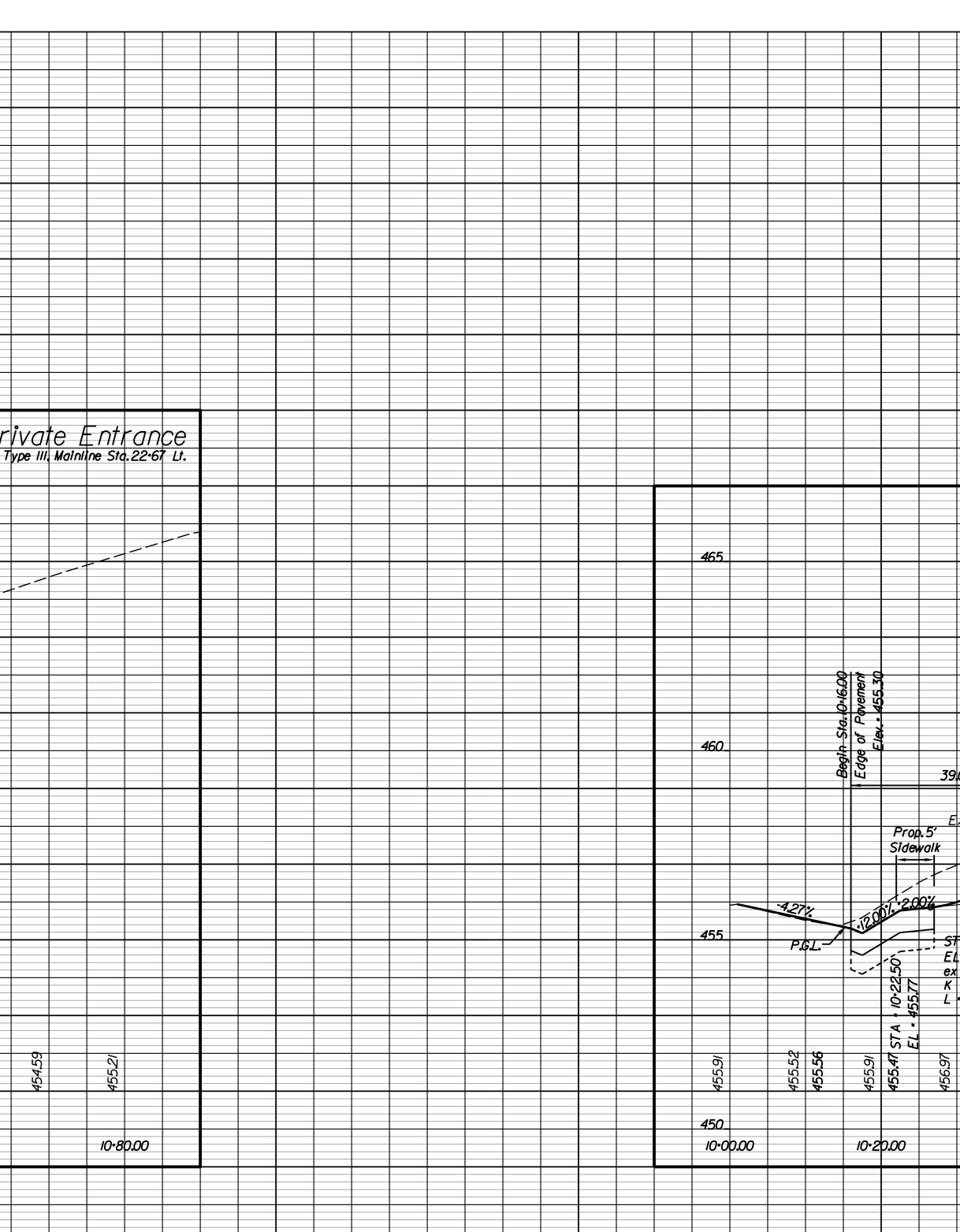
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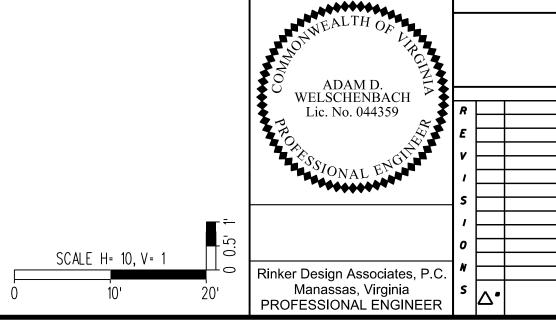
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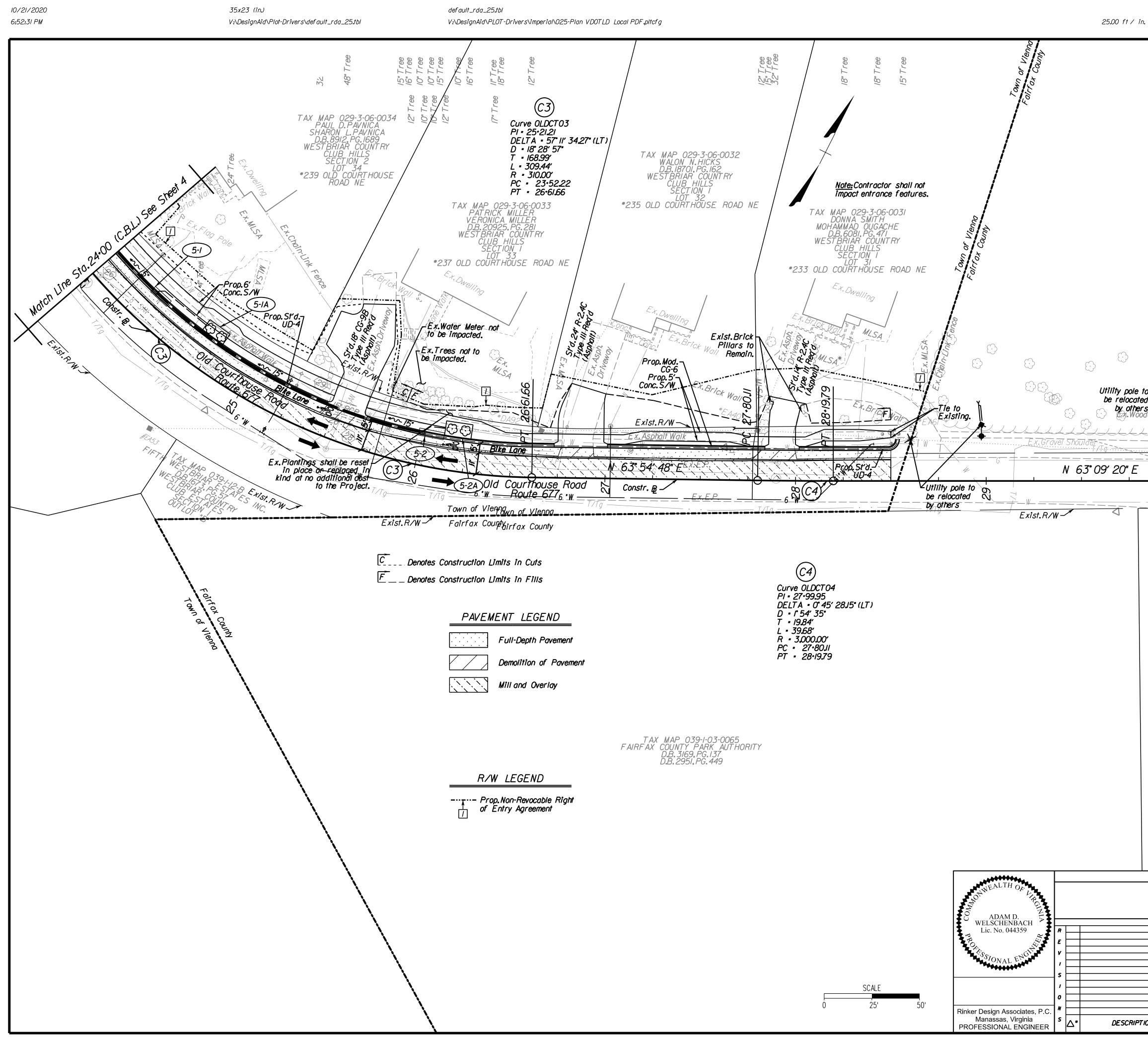
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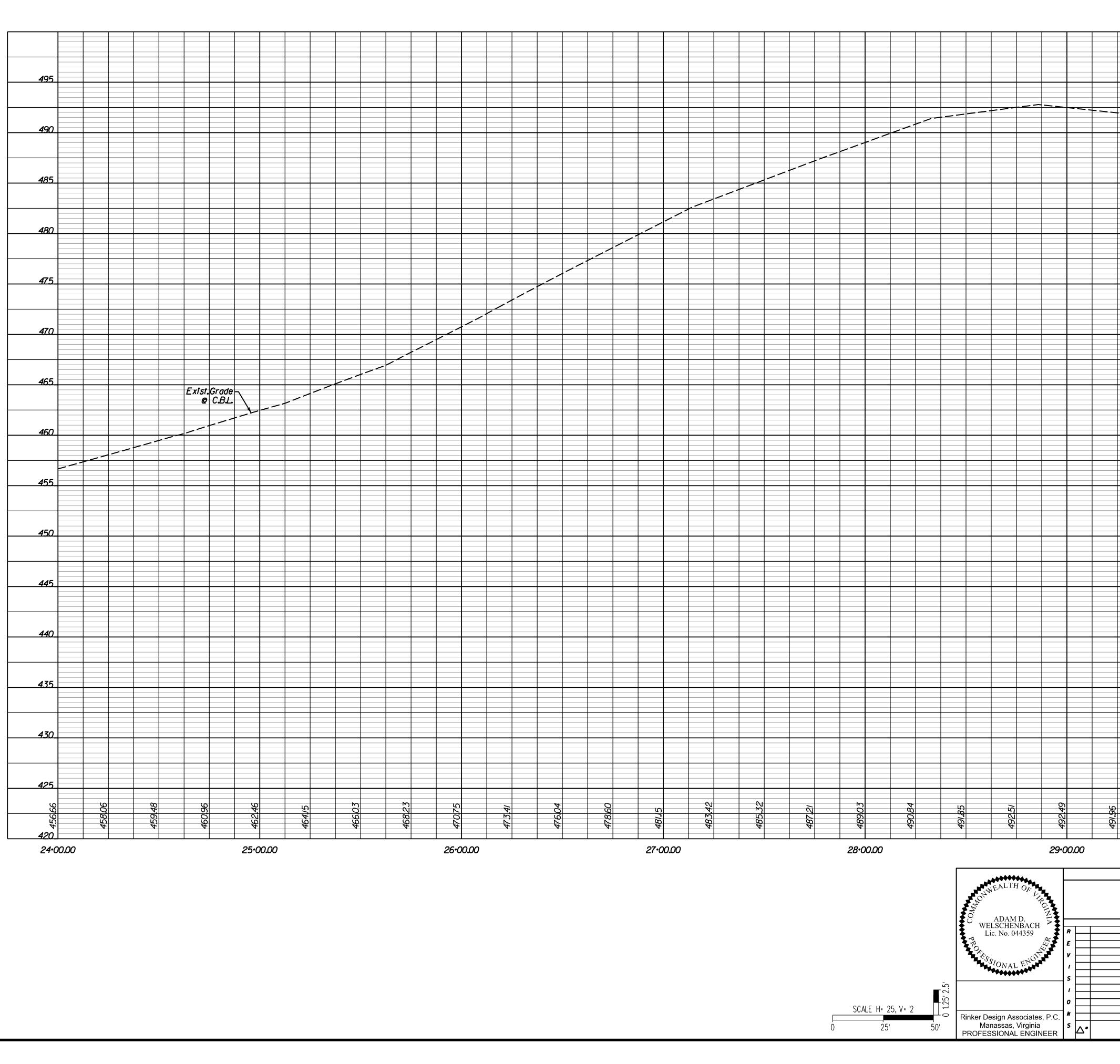
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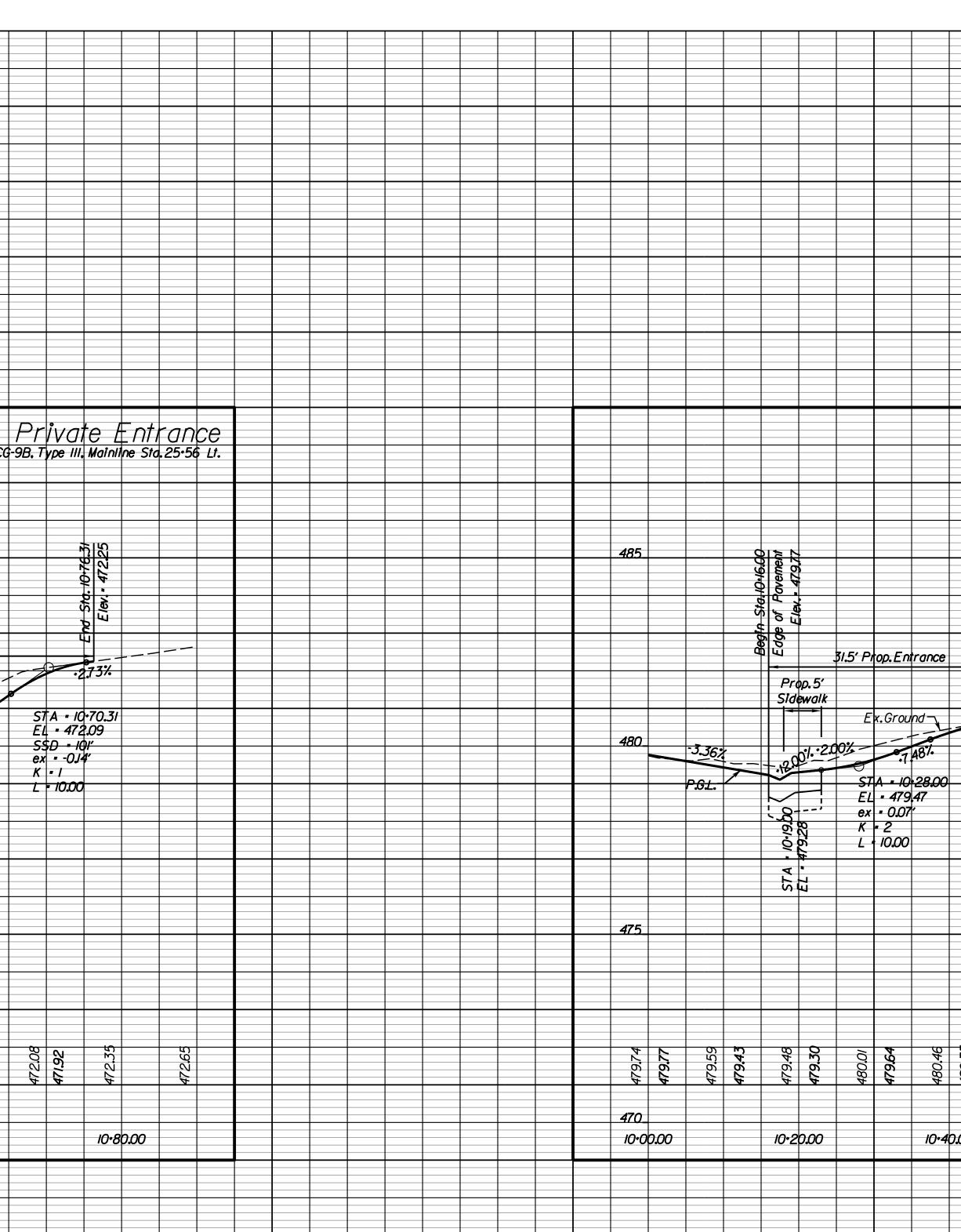
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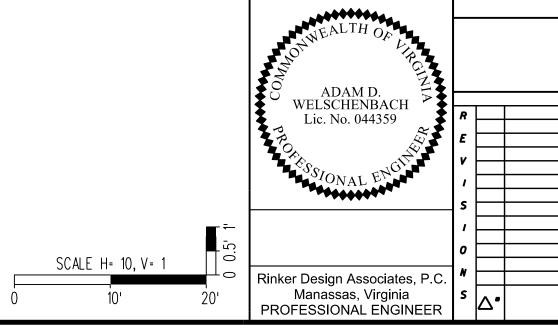
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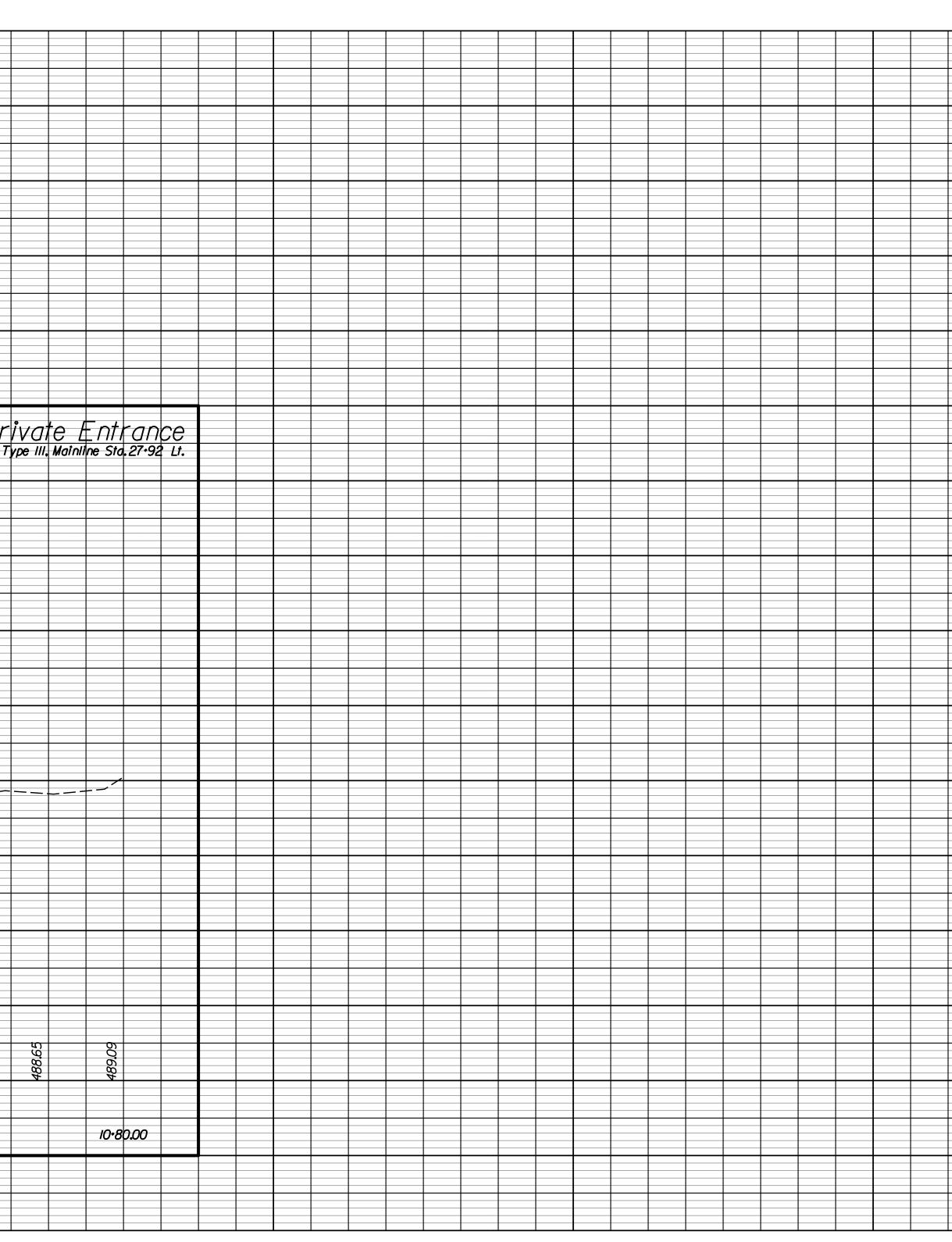
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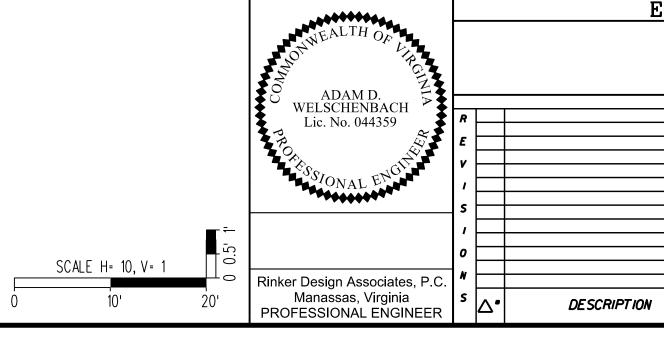
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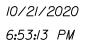
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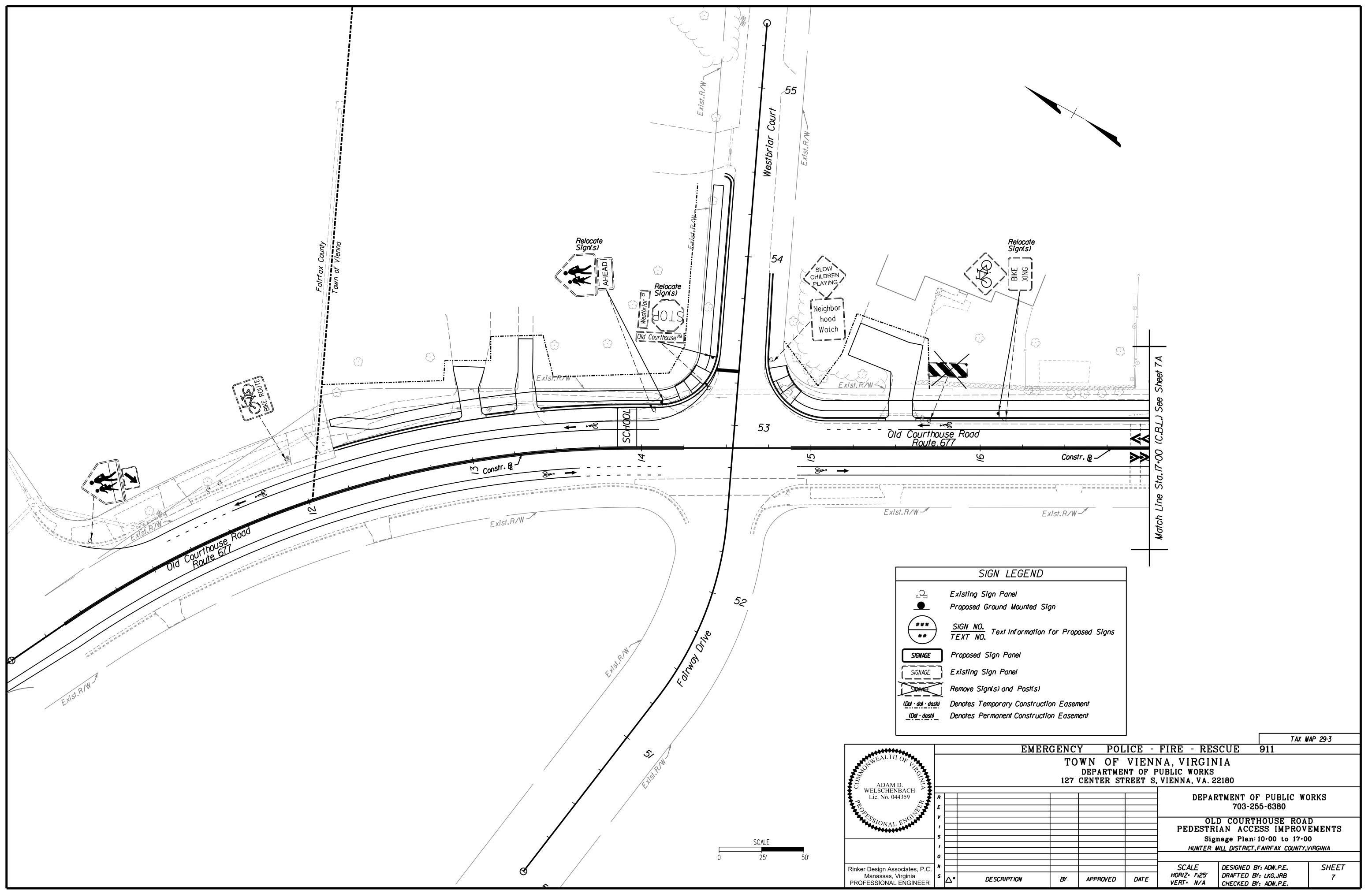
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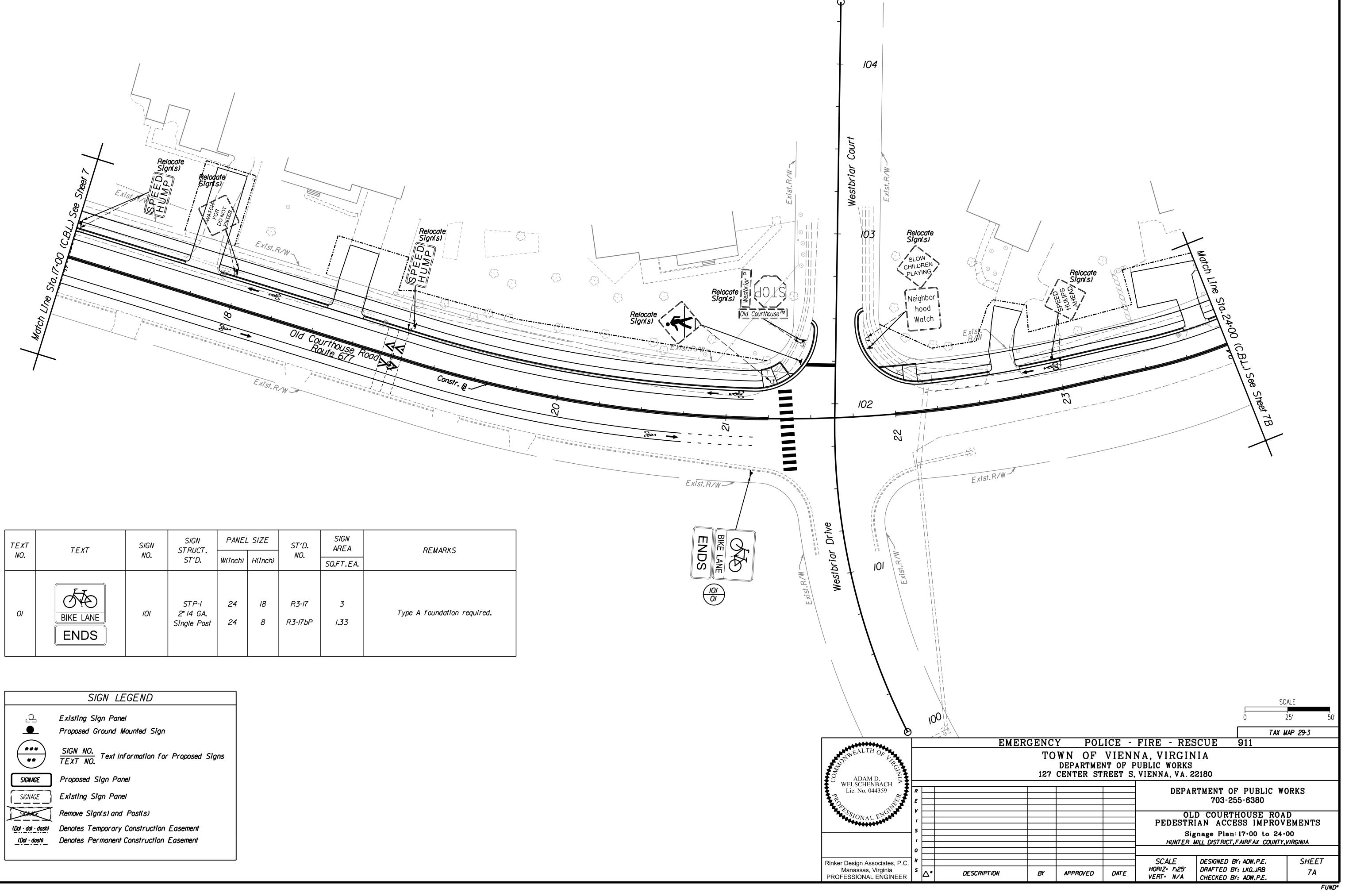




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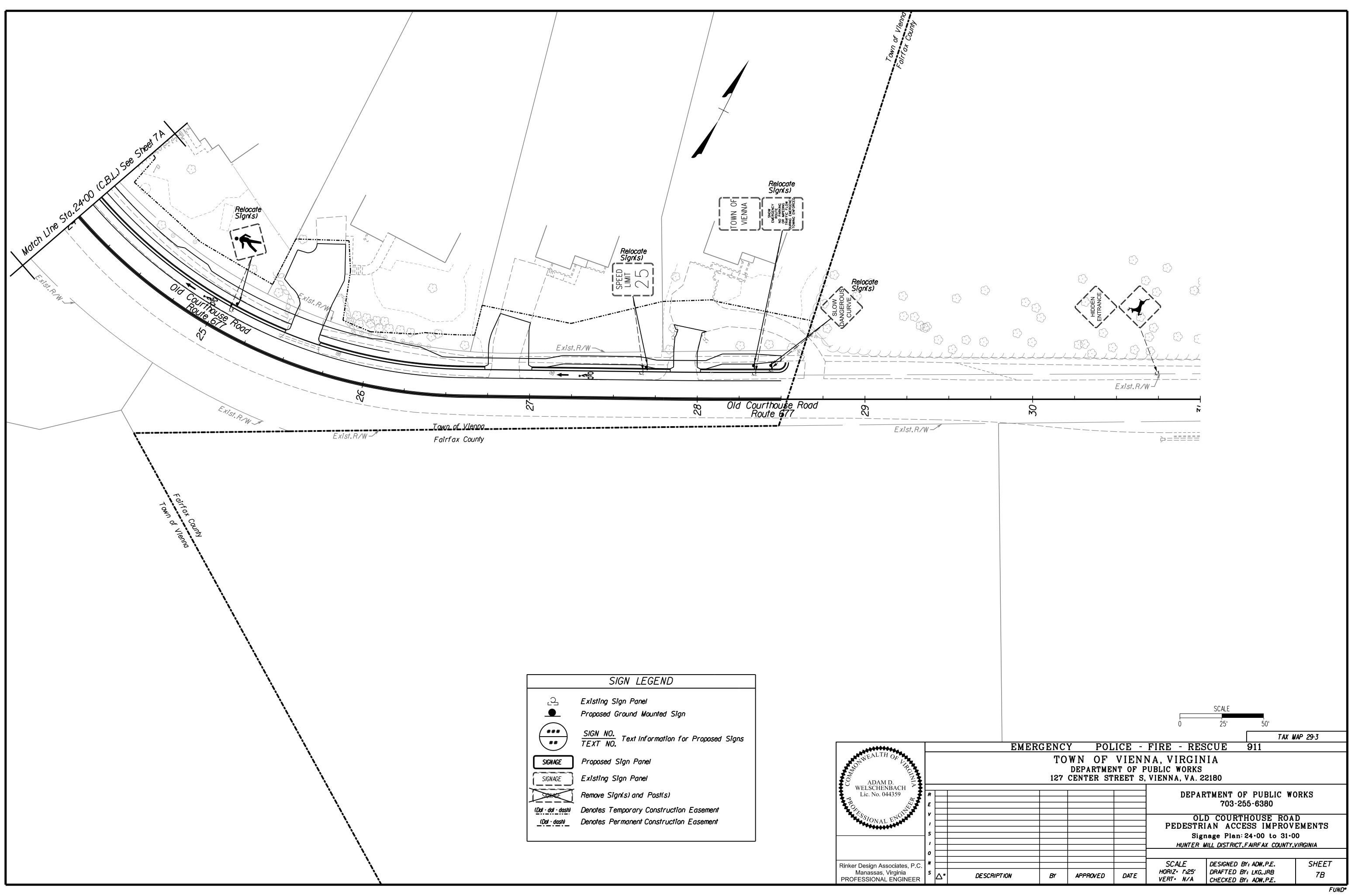


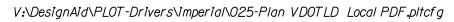




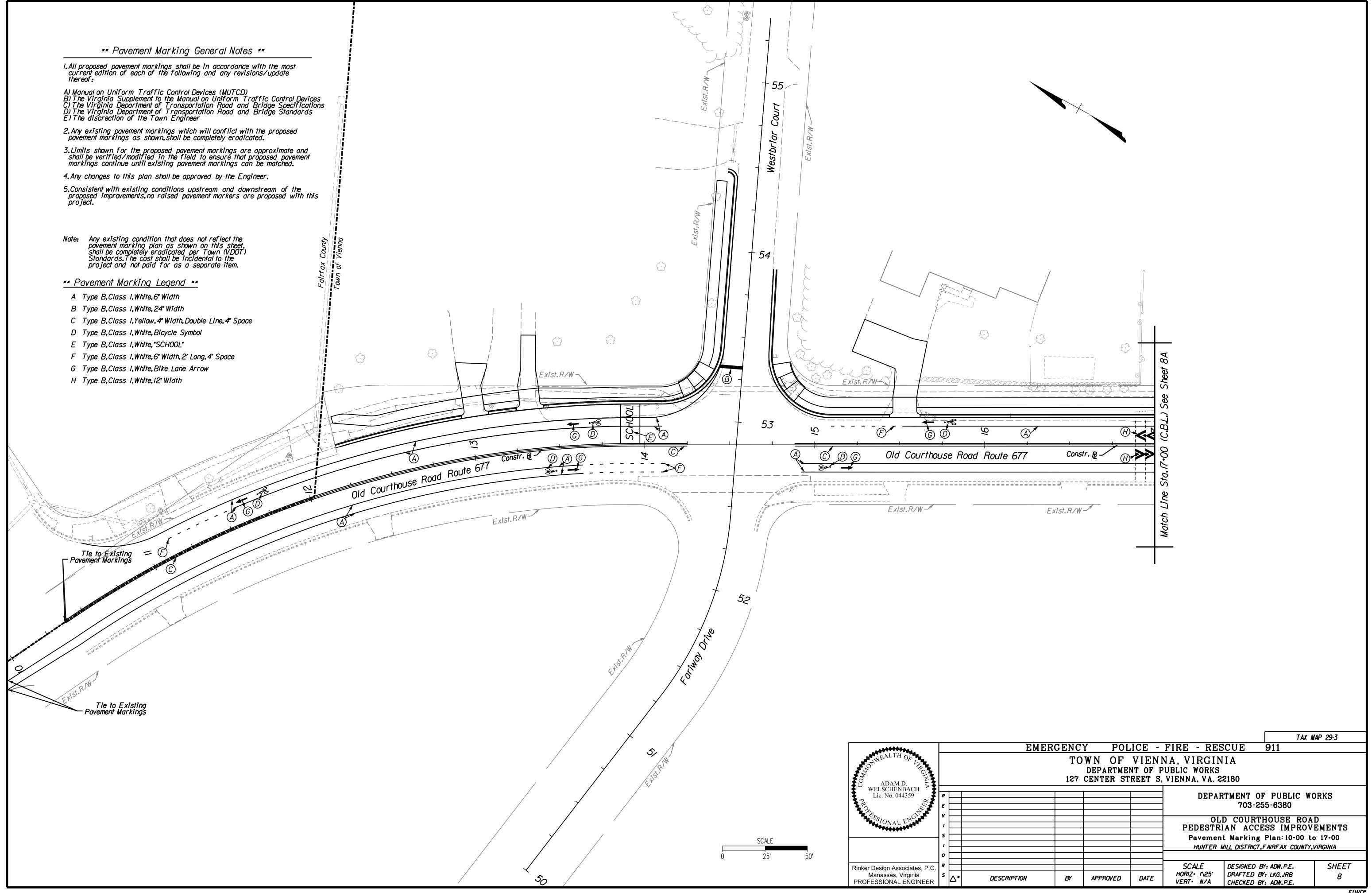
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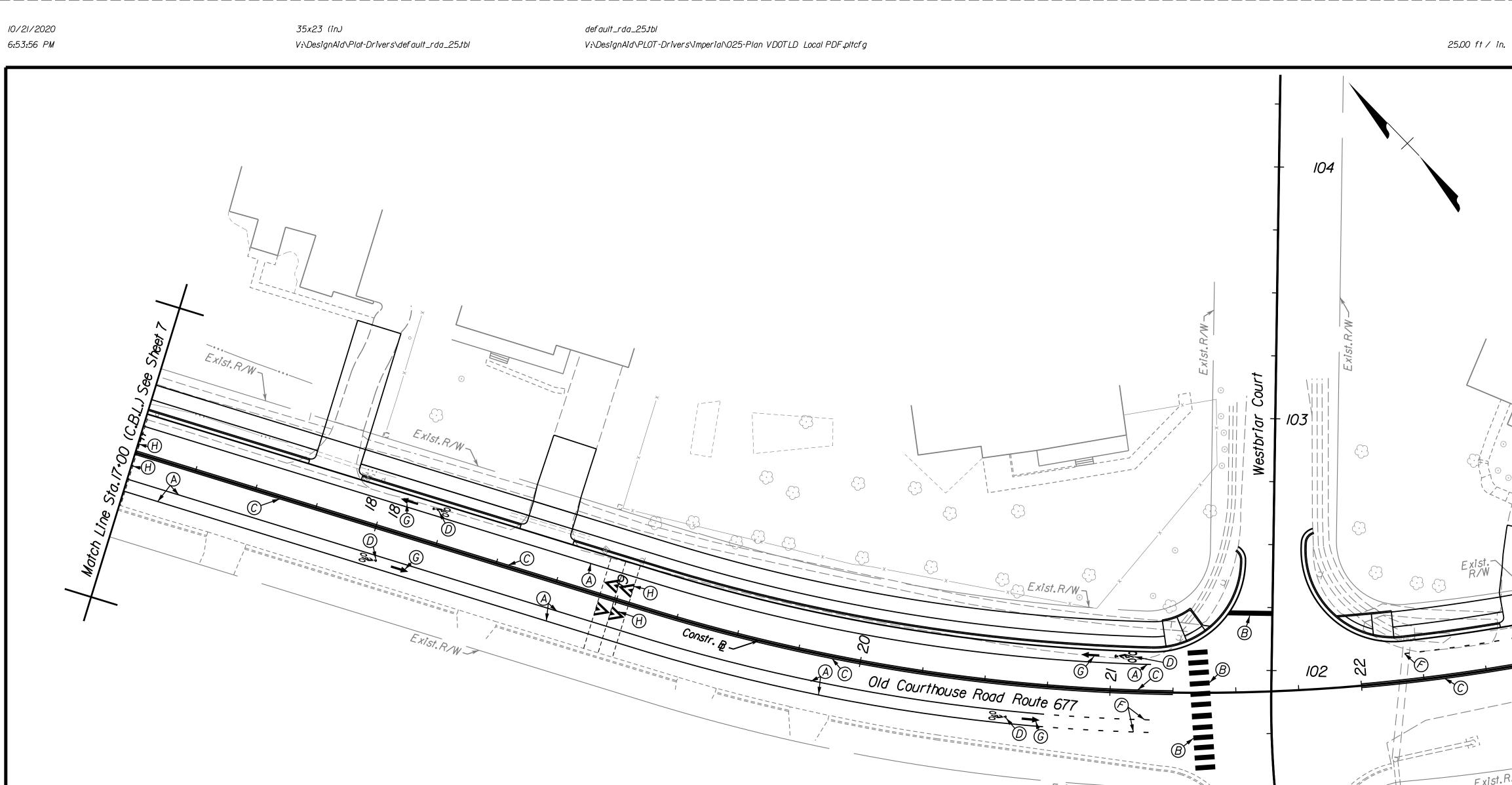
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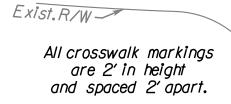




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ADAM D. WELSCHENBACH Lic. No. 044359

Rinker Design Associates, P.C. Manassas, Virginia PROFESSIONAL ENGINEER

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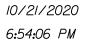
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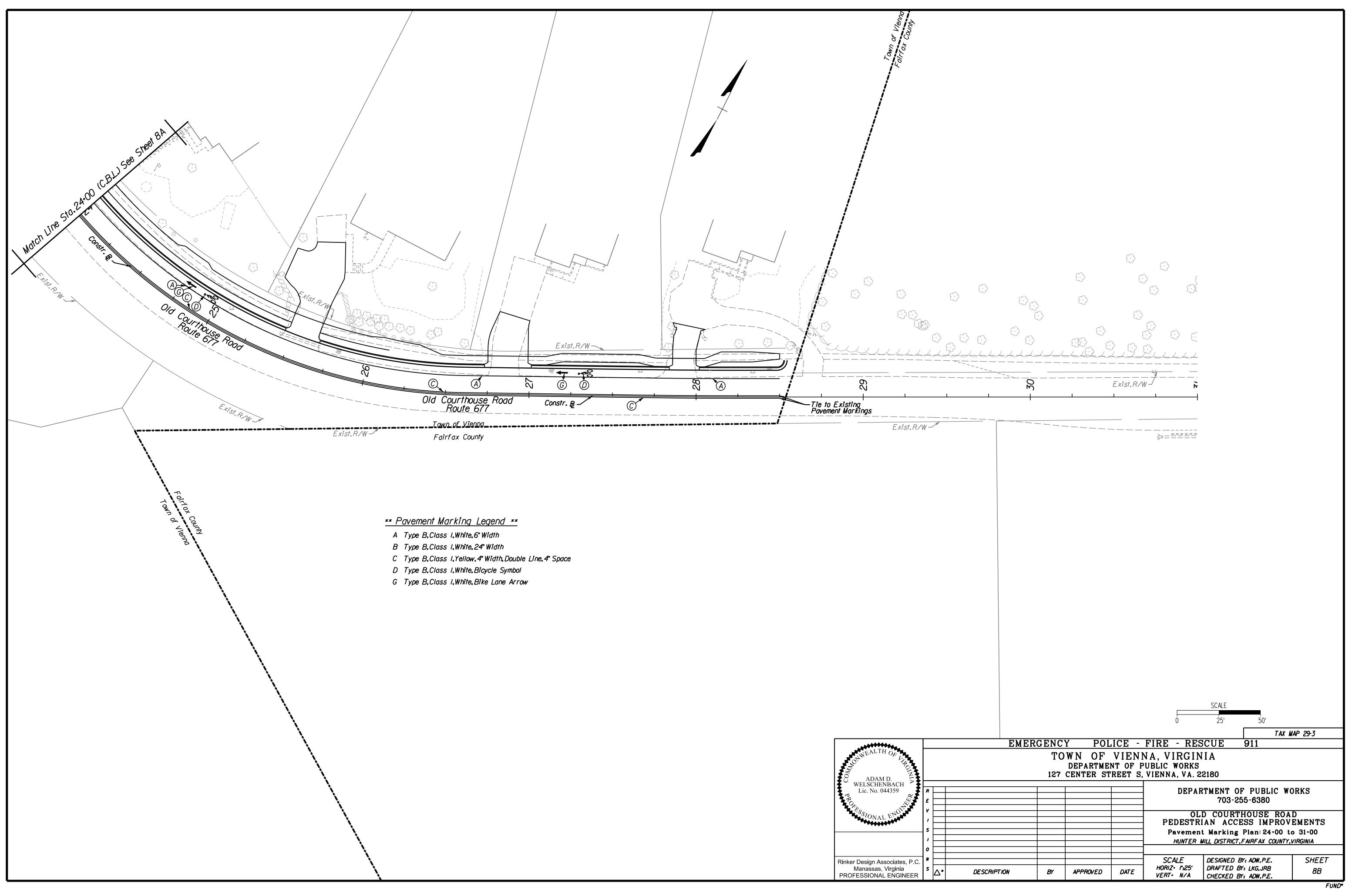
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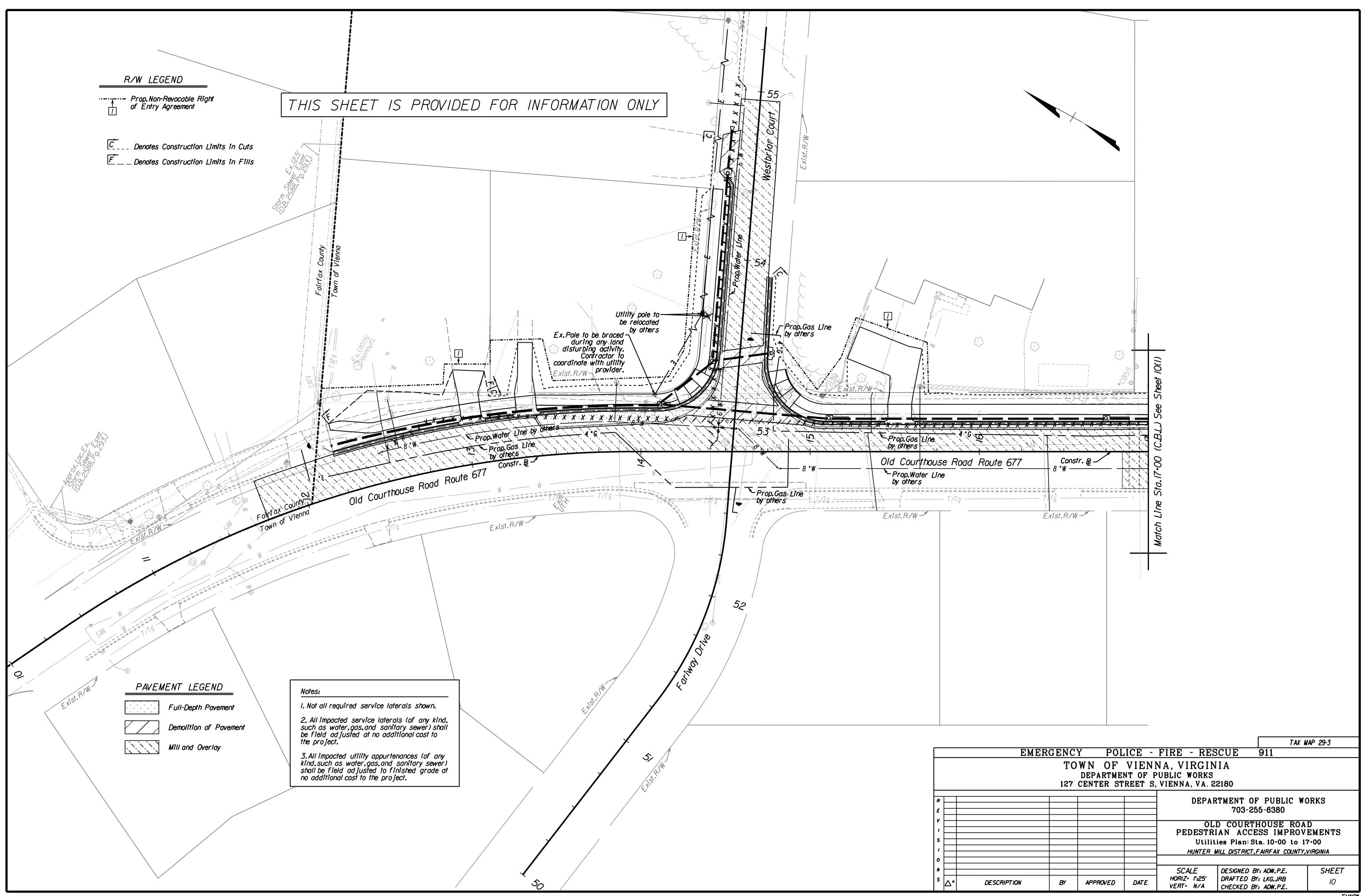
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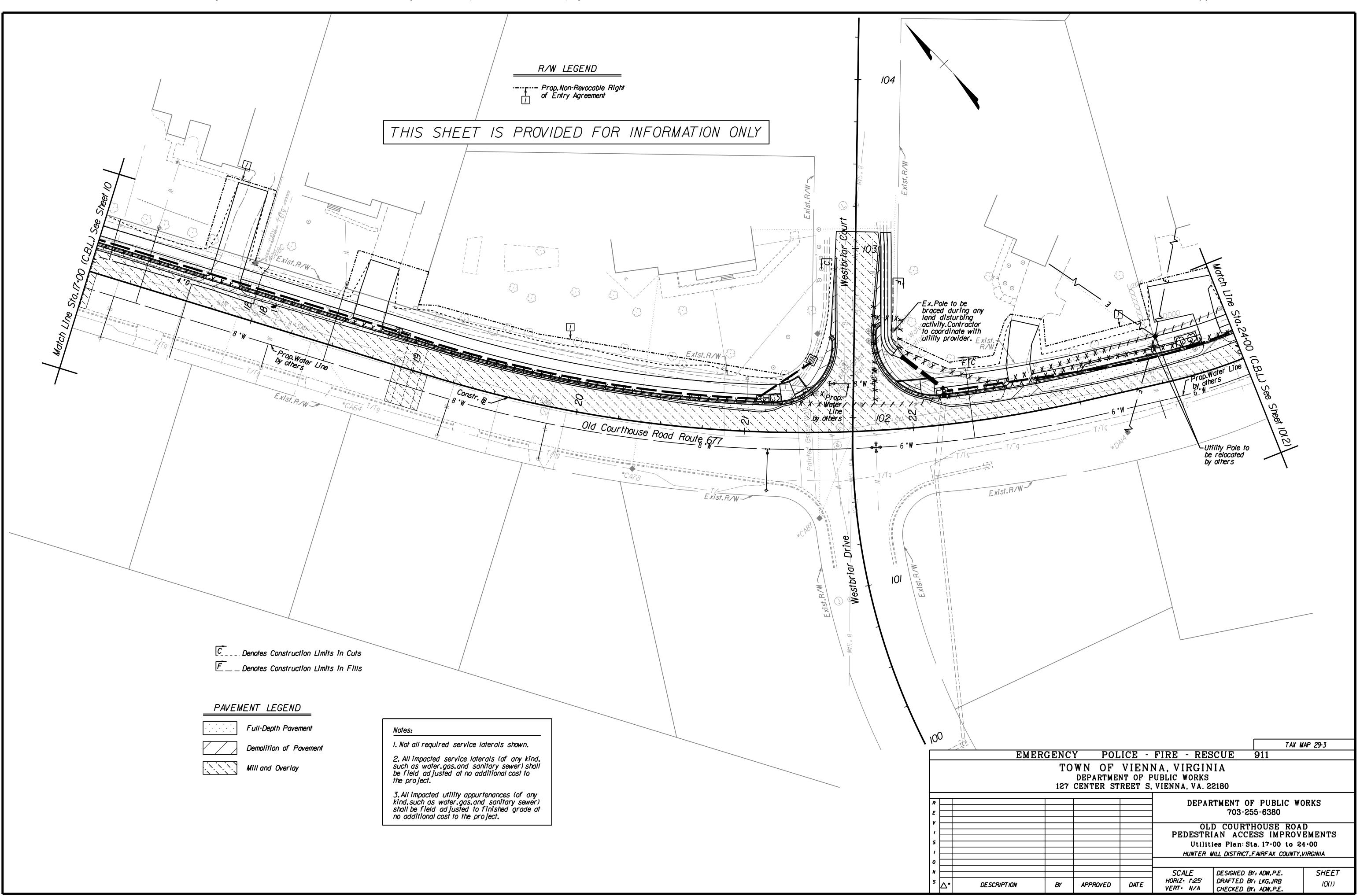


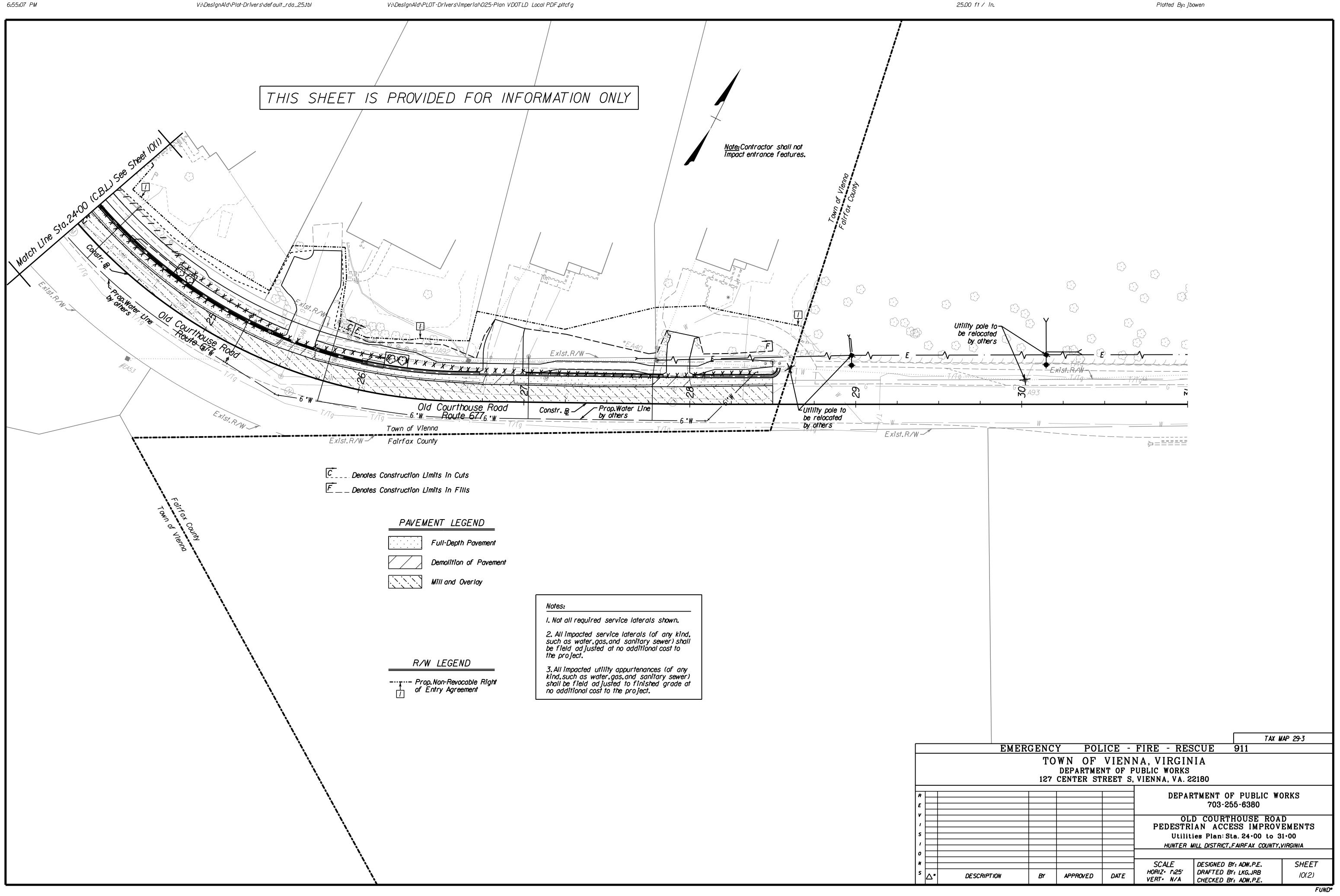




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